

ENVIRONMENTAL • CIVIL • GEOTECHNICAL • WATER • COMPLIANCE

January 27, 2016

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Mr. Jeff Levine, AICP, Director City of Portland, Maine Planning & Urban Development Department 389 Congress Street Portland, Maine 04101-3509

Subject: Level III Site Plan Review Application for Legacy 18 Development, Inc. 4-Unit Condo Building at 5 & 9 Romasco Lane, Portland, Maine

Dear Mr. Levine:

As requested, Sevee & Maher Engineers, Inc. (SME) is enclosing one copy of a Level III Site Plan Review Application for Legacy 18 Development, Inc.'s 4-Unit Condo Building located at 5 & 9 Romasco Lane in Portland, Maine.

Legacy 18 Development, Inc. (Owners) propose to construct a 1,955-square-foot apartment building on a 3,500 square foot parcel located at 5 and 9 Romasco Lane in Portland, Maine as shown on Figure 1 – Site Location Map included in Appendix A. The building will be a four-story, four-unit condo building with dedicated on-site parking in the garage located on the first floor. The property is located within the Residential 6 (R6) Zoning District and will comply with the City's vision of multifamily dwellings at a high density within this area.

The building will include a three car garage on the first floor, two one-bedroom units on the second floor and two two-bedroom units on the third and fourth floors. The building design also includes four private roof decks for access by each unit.

The property is currently fully disturbed with an 880-square-foot paved parking area along the street frontage and an overgrown lawn area to the rear of the property. There was a 650 square-foot building removed in July of 2015. The proposed building, paved vehicle maneuvering area, walks and patios will not result in an increase in developed area on the property, but will increase the on-site impervious area to approximately 2,100 square feet, an increase of 600 square feet.

The number of dwelling units proposed requires a Level III Site Plan Development Review permit through the City.

Page 1 of 4

4 Blanchard Road, PO Box 85A, Cumberland Center, Maine 04021 • Phone 207.829.5016 • Fax 207.829.5692 • www.smemaine.com

Should questions arise or additional information be desired, please do not hesitate to contact Dan Diffin, P.E., LEED AP BD+C at 207.829.5016.

Very truly yours,

SEVEE & MAHEBENBINEERS, INC. Guy H. Cote Jr., P.E.

President

cc: Judy George David Klenicki





Yes. Life's good here.

Jeff Levine, AICP, Director Planning & Urban Development Department

## **Electronic Signature and Fee Payment Confirmation**

Notice: Your electronic signature is considered a legal signature per state law.

By digitally signing the attached document(s), you are signifying your understanding this is a legal document and your electronic signature is considered a *legal signature* per Maine state law. You are also signifying your intent on paying your fees by the opportunities below.

I, the undersigned, intend and acknowledge that no Site Plan or Historic Preservation Applications can be reviewed until payment of appropriate application fees are *paid in full* to the Inspections Office, City of Portland Maine by method noted below:

X

Within 24-48 hours, once my complete application and corresponding paperwork has been electronically delivered, I intend to **call the Inspections Office** at 207-874-8703 and speak to an administrative representative and provide a credit/debit card over the phone.

Within 24-48 hours, once my application and corresponding paperwork has been electronically delivered, I intend to **call the Inspections Office** at 207-874-8703 and speak to an administrative representative and provide a credit/debit card over the phone.

I intend to deliver a payment method through the U.S. Postal Service mail once my application paperwork has been electronically delivered.

Applicant

I have provided digital copies and sent them on:

Date:

NOTE: All electronic paperwork must be delivered to <u>buildinginspections@portlandmaine.gov</u> or by physical means i.e. a thumb drive or CD to the Inspections Office, City Hall, 3<sup>rd</sup> Floor, Room 315.

389 Congress Street \* Portland Maine 04101-3509 \* Phone: (207) 874-8703 \* Fax: (207) 874-8716 http://www.portlandmaine.gov/planning/buildinsp.asp \* E-Mail: <u>buildinginspections@portlandmaine.gov</u>



## Level III – Preliminary and Final Site Plans Development Review Application Portland, Maine

Planning and Urban Development Department Planning Division

Portland's Planning and Urban Development Department coordinates the development review process for site plan, subdivision and other applications under the City's Land Use Code. Attached is the application form for a Level III: Preliminary or Final Site Plan. Please note that Portland has delegated review from the State of Maine for reviews under the Site Location of Development Act, Chapter 500 Stormwater Permits, and Traffic Movement Permits.

#### Level III: Site Plan Development includes:

- New structures with a total floor area of 10,000 sq. ft. or more except in Industrial Zones.
- New structures with a total floor area of 20,000 sq. ft. or more in Industrial Zones.
- New temporary or permanent parking area(s) or paving of existing unpaved parking areas for more than 75 vehicles.
- Building addition(s) with a total floor area of 10,000 sq. ft. or more (cumulatively within a 3 year period) except in Industrial Zones.
- Building addition(s) with a total floor area of 20,000 sq. ft. or more in Industrial Zones.
- A change in the use of a total floor area of 20,000 sq. ft. or more in any existing building (cumulatively within a 3 year period).
- Multiple family development (3 or more dwelling units) or the addition of any additional dwelling unit if subject to subdivision review.
- Any new major or minor auto business in the B-2 or B-5 Zone, or the construction of any new major or minor auto business greater than 10,000 sq. ft. of building area in any other permitted zone.
- Correctional prerelease facilities.
- Park improvements: New structures greater than 10,000 sq. ft. and/or facilities encompassing 20,000 sq. ft. or more (excludes rehabilitation or replacement of existing facilities); new nighttime outdoor lighting of sports, athletic or recreation facilities not previously illuminated.
- Land disturbance of 3 acres or more (includes stripping, grading, grubbing, filling or excavation).

Portland's development review process and requirements are outlined in the Land Use Code (Chapter 14) which is available on our website:

Land Use Code: <u>http://me-portland.civicplus.com/DocumentCenter/Home/View/1080</u> Design Manual: <u>http://me-portland.civicplus.com/DocumentCenter/View/2355</u> Technical Manual: <u>http://me-portland.civicplus.com/DocumentCenter/View/2356</u>

Planning Division Fourth Floor, City Hall 389 Congress Street (207) 874-8719 Office Hours Monday thru Friday 8:00 a.m. – 4:30 p.m.

#### **PROPOSED DEVELOPMENT ADDRESS:**

5-9 Romasco Lane, Portland, ME

#### **PROJECT DESCRIPTION:**

A new 2,000 square foot, 4-unit condominium building with a 3 car garage on a 3,496 square foot lot.

СНАКТ/ВLOCK/LOT:	PRELIMINARY PLAN	(date)
	FINAL PLAN	(date)

CONTACT INFORMATION:		
Applicant – must be owner, Lessee or Buyer	Applicant Contact Information	
Name: David Klenicki, President	Work # (917) 608-8814	
Business Name, if applicable: Legacy 18 Development Inc.	Home#	
Address: 233 Smith Road	Cell # Fax#	
City/State :Windham, Me Zip Code: 04062	<sup>e-mail:</sup> kdsinc7@gmail.com	
<b>Owner</b> – (if different from Applicant)	Owner Contact Information	
Name:	Work #	
Address:	Home#	
City/State : Zip Code:	Cell # Fax#	
	e-mail:	
Agent/ Representative	Agent/Representative Contact information	
Name: Sevee & Maher Engineers, Inc.	Work # (207) 829-5016	
Address: 4 Blanchard Road	Cell # (207) 240-3315	
City/State : Cumberland, ME Zip Code: 04021	<sup>e-mail:</sup> dpd@smemaine.com	
Billing Information	Billing Information	
Name: Judy George	Work #	
Address: 233 Smith Road	Cell # Fax#	
City/State : Windham, Me Zip Code: 04062	e-mail:	

Engineer	Engineer Contact Information	
Name: Daniel P. Diffin, P.E. (SME)	Work # (207) 829-5016	
Address: 4 Blanchard Rd	Cell # (207) 240-3315 Fax# (207) 829-5692	
City/State : Cumberland, ME Zip Code: 04021	<sup>e-mail:</sup> dpd@smemaine.com	
Surveyor	Surveyor Contact Information	
Name: Livingston-Hughes Surveyors	Work # 207-969-9761	
Address: 88 Guinea Rd.	Cell # Fax# 207-967-4831	
City/StateKennebunkport, ME Zip Code: 04046	e-mail: emily@livingstonhughes.com	
Architect	Architect Contact Information	
Name: Klenicki Design Services, Inc.	Work #	
Address: 273 Smith Ridge Road	Cell # Fax# (	
City/State : South Salem, NY Zip Code: 10590	e-mail:	
Attorney	Attorney Contact Information	
Name:	Work #	
Address:	Cell # Fax#	
City/State : Zip Code:	e-mail:	

#### **APPLICATION FEES:**

#### Check all reviews that apply. (Payment may be made by Credit Card, Cash or Check payable to the City of Portland.)

Level III Development (check applicable reviews)	Other Reviews (check applicable reviews)		
<u>X</u> Less than 50,000 sq. ft. ( $$500.00$ )			
50,000 - 100,000 sq. ft. (\$1,000)	Traffic Movement (\$1,000)		
100,000 – 200,000 sq. ft. (\$2,000)	Stormwater Quality (\$250)		
200,000 – 300,000 sq. ft. (\$3,000)	Subdivisions (\$500 + \$25/lot)		
over \$300,00 sq. ft. (\$5,000)	# of Lots x \$25/lot =		
Parking lots over 11 spaces (\$1,000)	Site Location (\$3,000, except for		
After-the-fact Review (\$1,000.00 plus	residential projects which shall be		
applicable application fee)	\$200/lot)		
	# of Lots x \$200/lot =		
Plan Amendments (check applicable reviews)	Other		
Planning Staff Review (\$250)	Change of Use		
Planning Board Review (\$500)	Flood Plain		
	Shoreland		
The City invoices separately for the following:	Design Review		
<ul> <li>Notices (\$.75 each)</li> </ul>	Housing Replacement		
<ul> <li>Legal Ad (% of total Ad)</li> </ul>	Historic Preservation		
<ul> <li>Planning Review (\$40.00 hour)</li> </ul>			
<ul> <li>Legal Review (\$75.00 hour)</li> </ul>			
Third party review fees are assessed separately. Any outside			
reviews or analysis requested from the Applicant as part of the			
development review, are the responsibility of the Applicant and			
are separate from any application or invoice fees.			
· · · ·			
	1		

#### APPLICATION SUBMISSION:

- All site plans and written application materials must be submitted electronically on a CD or thumb drive with each plan submitted as separate files, with individual file which can be found on the Electronic Plan and Document Submittal page of the City's website at <a href="http://me-portland.civicplus.com/764/Electronic-Plan-and-Document-Submittal">http://me-portland.civicplus.com/764/Electronic-Plan-and-Document-Submittal</a>
- In addition, one (1) paper set of the plans (full size), one (1) paper set of plans (11 x 17), paper copy of written materials, and the application fee must be submitted to the Building Inspections Office to start the review process.

The application must be complete, including but not limited to the contact information, project data, application checklists, wastewater capacity, plan for fire department review, and applicant signature. The submissions shall include one (1) paper packet with folded plans containing the following materials:

- 1. One (1) full size site plans that must be folded.
- 2. One (1) copy of all written materials or as follows, unless otherwise noted:
  - a. Application form that is completed and signed.
  - b. Cover letter stating the nature of the project.
  - c. All Written Submittals (Sec. 14-525 2. (c), including evidence of right, title and interest.
- 3. A stamped standard boundary survey prepared by a registered land surveyor at a scale not less than one inch to 50 feet.
- 4. Plans and maps based upon the boundary survey and containing the information found in the attached sample plan checklist.
- 5. One (1) set of plans reduced to 11 x 17.

Please refer to the application checklist (attached) for a detailed list of submission requirements.

#### **APPLICANT SIGNATURE:**

I hereby certify that I am the Owner of record of the named property, or that the owner of record authorizes the proposed work and that I have been authorized by the owner to make this application as his/her authorized agent. I agree to conform to all applicable laws of this jurisdiction. In addition, if a permit for work described in this application is issued, I certify that the Planning Authority and Code Enforcement's authorized representative shall have the authority to enter all areas covered by this permit at any reasonable hour to enforce the provisions of the codes applicable to this permit.

This application is for a Level II Site Plan review. It is not a permit to begin construction. An approved site plan, a Performance Guarantee, Inspection Fee, Building Permit, and associated fees will be required prior to construction. Other Federal, State or local permits may be required prior to construction, which are the responsibility of the applicant to obtain.

Signature of Applicant:	Date: 1 2014
Leonatary Losacy 18 mit Tre.	
Beulgeman	

Updated: April 23, 2014

## **PROJECT DATA**

#### The following information is required where applicable, in order to complete the application.

Total Area of Site	3,496	sq. ft.
Proposed Total Disturbed Area of the Site	3,496	sq. ft.
If the proposed disturbance is greater than one acre, then the app	licant shall apply for a Maine Cons	truction General Permit
(MCGP) with DEP and a Stormwater Management Permit, Chapte	r 500, with the City of Portland.	
Impervious Surface Area		
Impervious Area (Total Existing)	1,520	sq. ft.
Impervious Area (Total Proposed)	2,095	sq. ft.
Building Ground Floor Area and Total Floor Area		
Building Footprint (Total Existing)	639	sq. ft.
Building Footprint (Total Proposed)	1,955	sq. ft.
Building Floor Area (Total Existing)	0	sq. ft.
Building Floor Area (Total Proposed)	8,020	sq. ft.
Zoning	Desidential 0	
Existing	Residential 6	(R6)
Land Use		
Existing	Single Famil/	Parking
Proposed	Multi-family a	partments
		F
Residential, If applicable		
# of Residential Units (Total Existing)	1	
# of Residential Units (Total Proposed)	4	
# of Lots (Total Proposed)	1	
# of Affordable Housing Units (Total Proposed)	0	
Proposed Bedroom Mix		
# of Efficiency Units (Total Proposed)	0	
# of One-Bedroom Units (Total Proposed)	2	
# of Two-Bedroom Units (Total Proposed)	2	
# of Three-Bedroom Units (Total Proposed)	0	
Parking Spaces		
# of Parking Spaces (Total Existing)	0	
# of Parking Spaces (Total Proposed)	3 - Garage	
# of Handicapped Spaces (Total Proposed)	0	
Disuda Daubina Conserv		
Bicycle Parking Spaces		
# OT BICYCLE Spaces (Total Existing)	0	
# OT BICYCLE Spaces (TOTAL Proposed)	2 - Garage	
Estimated Cast of Draiast		
Estimated Cost of Project		

FINAL PLAN - Level III Site Plan			
_		_	GENERAL WRITTEN SUBMISSIONS CHECKLIST
Applicant	Planner	# of	(* If applicant chooses to submit a Preliminary Plan, then the * items were
Checklist	Checklist	Copies	submitted for that phase and only updates are required)
X		1	* Completed Application form
X		1	* Application fees
X		1	* Written description of project
×		1	* Evidence of right, title and interest
Х		1	* Evidence of state and/or federal permits
×		1	* Written assessment of proposed project's specific compliance with applicable Zoning requirements
×		1	* Summary of existing and/or proposed easements, covenants, public or private rights-of-way, or other burdens on the site
X		1	* Evidence of financial and technical capacity
X		1	Construction Management Plan
Х		1	A traffic study and other applicable transportation plans in accordance with Section 1 of the technical Manual, where applicable.
Х		1	Written summary of significant natural features located on the site (Section 14- 526 (b) (a))
×		1	Stormwater management plan and stormwater calculations
X		1	Written summary of project's consistency with related city master plans
X		1	Evidence of utility capacity to serve
X		1	Written summary of solid waste generation and proposed management of solid waste
×		1	A code summary referencing NFPA 1 and all Fire Department technical standards
×		1	Where applicable, an assessment of the development's consistency with any applicable design standards contained in Section 14-526 and in City of Portland Design Manual
×		1	Manufacturer's verification that all proposed HVAC and manufacturing equipment meets applicable state and federal emissions requirements.

Applicant	Planner	# of	SITE PLAN SUBMISSIONS CHECKLIST (* If applicant chooses to submit a Preliminary Plan, then the * items were		
Checklist	Checklist	Copies	submitted for that phase and only updates are required)		
×		1	* Boundary Survey meeting the requirements of Section 13 of the City of Portland's Technical Manual		
×		1	Final Site Plans including the following:		
×		Existing a (includin	and proposed structures, as applicable, and distance from property line glocation of proposed piers, docks or wharves if in Shoreland Zone);		
X		Existing a	and proposed structures on parcels abutting site;		
×		All street modifica	s and intersections adjacent to the site and any proposed geometric tions to those streets or intersections;		
×		Location and pede lines;	, dimensions and materials of all existing and proposed driveways, vehicle estrian access ways, and bicycle access ways, with corresponding curb		
×		Engineer	ed construction specifications and cross-sectional drawings for all driveways, paved areas, sidewalks;		
Х		Location and dimensions of all proposed loading areas including turning templates for applicable design delivery vehicles;			
х		Existing and proposed public transit infrastructure with applicable dimensions and engineering specifications;			
		Location of existing and proposed vehicle and bicycle parking spaces with			
X		applicable dimensional and engineering information;			
X		Location of all snow storage areas and/or a snow removal plan;			
Х		A traffic control plan as detailed in Section 1 of the Technical Manual;			
V		Proposed buffers and preservation measures for significant natural features,			
X		where ap	where applicable, as defined in Section 14-526(b)(1);		
Х		Location and proposed alteration to any watercourse;			
x		A delineation of wetlands boundaries prepared by a qualified professional as			
X V		detailed in Section 8 of the Technical Manual;			
Χ		Proposed buffers and preservation measures for wetlands;			
X		Existing soil conditions and location of test pits and test borings;			
		Existing vegetation to be preserved, proposed site landscaping, screening and			
X		proposed street trees, as applicable;			
		Technica	A stormwater management and drainage plan, in accordance with Section 5 of the		
		Grading	Grading plan:		
X		Ground	water protection measures:		
		Existing	Existing and proposed sewer mains and connections:		

- Continued on next page -

	Location of all existing and proposed fire hydrants and a life safety plan in
×	accordance with Section 3 of the Technical Manual;
	Location, sizing, and directional flows of all existing and proposed utilities within
X	the project site and on all abutting streets;
	Location and dimensions of off-premises public or publicly accessible
×	infrastructure immediately adjacent to the site;
	Location and size of all on site solid waste receptacles, including on site storage
×	containers for recyclable materials for any commercial or industrial property;
	Plans showing the location, ground floor area, floor plans and grade elevations for
X	all buildings;
Х	A shadow analysis as described in Section 11 of the Technical Manual, if applicable;
	A note on the plan identifying the Historic Preservation designation and a copy of
x	the Application for Certificate of Appropriateness, if applicable, as specified in
	Section Article IX, the Historic Preservation Ordinance;
	Location and dimensions of all existing and proposed HVAC and mechanical
×	equipment and all proposed screening, where applicable;
×	An exterior lighting plan in accordance with Section 12 of the Technical Manual;
	A signage plan showing the location, dimensions, height and setback of all existing
X	and proposed signs;
	Location, dimensions and ownership of easements, public or private rights of way,
	both existing and proposed.



#### PORTLAND FIRE DEPARTMENT SITE REVIEW FIRE DEPARTMENT CHECKLIST



A separate drawing[s] shall be provided as part of the site plan application for the Portland Fire Department's review.

- 1. Name, address, telephone number of applicant David Klenicki, 273 Smith Ridge Road, South Salem, NY
- 2.
- 3. Name address, telephone number of architect
  - Klenicki Design Services, 273 Smith Ridge Road, South Salem, NY (917) 608-8814
- 4. Proposed uses of any structures [NFPA and IBC classification]
- 5.
- 6. Square footage of all structures [total and per story]
  - 8020 sf and 2005 sf/floor
- 7. Elevation of all structures

102.50 first floor elevation and 144.5 roof elevation

- 8. Proposed fire protection of all structures
  - <u>As of September 16, 2010 all new construction of one and two family homes are</u> required to be sprinkled in compliance with NFPA 13D. This is required by City Code. (NFPA 101 2009 ed.)
- 9. Hydrant locations 80 feet away on Romasco Lane
- 10. Water main[s] size and location8" main in the street
- 11. Access to all structures [min. 2 sides]
  - Yes
- 12. A code summary shall be included referencing NFPA 1 and all fire department. Technical standards.

Some structures may require Fire flows using annex H of NFPA 1

# CITY OF PORTLAND, MAINE PLANNING BOARD LEVEL III SITE PLAN REVIEW APPLICATION

**Prepared for** 

# LEGACY 18 DEVELOPMENT INC. 4-UNIT CONDO BUILDING

5 & 9 Romasco Lane Portland, Maine

January 2016



ENVIRONMENTAL • CIVIL • GEOTECHNICAL • WATER • COMPLIANCE



#### **TABLE OF CONTENTS**

Section No.	Title	Page No.

Application and Submissions Checklist

PROJECT DESCRIPTION	1
FINAL PLAN – LEVEL III SITE PLAN CHECKLIST REQUIREMENTS	1
ARTICLE V SITE PLAN	2
14-139 Dimensional Requirements	2
14-526 (a) Transportation Standards	2
14-526 (a) Environmental Quality Standards	3
A. Preservation of Significant Natural Features	3
B. Landscaping and Landscape Preservation	3
C. Water Quality, Stormwater Management, and Erosion Control	3
14-526 (c) Public Infrastructure and Community Safety Standards	4
A. Consistency with City Master Plans	4
B. Public Safety and Fire Prevention	4
C. Availability and Adequate Capacity of Public Utilities	4
14-526 (d) Site Design Standards	4
A. Massing, Ventilation and Wind Impact	4
B. Shadows	4
C. Snow and Ice Loading	4
D. View Corridors	4
E. Historic Resources	5
F. Exterior Lighting	5
G. Noise and Vibration	5
H. Signage and Wayfinding	5
I. Zone Related Design Standards	5
J. Solid Waste Generation and Management	5
CONSTRUCTION MANAGEMENT PLAN	5

## LIST OF APPENDICES

APPENDIX A	FIGURE 1 – SITE LOCATION MAP
APPENDIX B	TITLE, RIGHT OR INTEREST
APPENDIX C	FINANCIAL CAPACITY
APPENDIX D	CITY OF PORTLAND GIS MAP
APPENDIX E	FEMA FLOODPLAIN MAP
APPENDIX F	STORMWATER MANAGEMENT REPORT
APPENDIX G	POST CONSTRUCTION STORMWATER MANAGEMENT PLAN
APPENDIX H	CITY WASTEWATER CAPACITY APPLICATION
APPENDIX I	PORTLAND WATER DISTRICT CAPACITY TO SERVE LETTER
APPENDIX J	CONSTRUCTION MANAGEMENT PLAN

#### **PROJECT DESCRIPTION**

Legacy 18 Development, Inc. (Owners) propose to construct a 1,955-square-foot apartment building on a 3,500 square foot parcel located at 5 and 9 Romasco Lane in Portland, Maine as shown on Figure 1 – Site Location Map included in Appendix A. The building will be a four-story, four-unit condo building with dedicated on-site parking in the garage located on the first floor. The property is located within the Residential 6 (R6) Zoning District and will comply with the City's vision of multifamily dwellings at a high density within this area.

The building will include; a three car garage on the first floor, two one-bedroom units on the second floor and two two-bedroom units on the third and fourth floors. The building design also includes four private roof decks for access by each unit.

The property is currently fully disturbed with an 880-square-foot paved parking area along the street frontage and an overgrown lawn area to the rear of the property. There was a 650 square-foot building removed in July of 2015. The proposed building, paved vehicle maneuvering area, walks and patios will not result in an increase in developed area on the property, but will increase the on-site impervious area to approximately 2,100 square feet, an increase of 600 square feet.

The number of dwelling units proposed requires a Level III Site Plan Development Review permit through the City. The following demonstrates the project's compliance with the applicable City of Portland Land Use Ordinance.

### FINAL PLAN – LEVEL III SITE PLAN CHECKLIST REQUIREMENTS

This application package has been prepared in accordance with the City of Portland Land Use Ordinance and the Level III Site Plan Development Review Application. To address the submission requirements, the Applicant has provided the following:

Right, Title and Interest – See copies of Deed included in Appendix B.

<u>State and/or Federal Permits</u> – Not required for this project.

Evidence of Financial Capacity - See letter of financing provided in Appendix C.

<u>Evidence of Technical Capacity</u> – The Owners have hired qualified professionals to assist in the design, bidding and construction administration for this project as detailed on the application form.

<u>City of Portland GIS Map</u> – See copy of map included in Appendix D.

FEMA Map – See FEMA firmette in Appendix E

#### ARTICLE V. - SITE PLAN

The following demonstrate the project's compliance with Section 14-526 Site Plan Standards.

#### 14-139 Dimensional Requirements

The site has been designed in accordance with all the dimensional requirements in section 14-139 of the City of Portland Code of Ordinances. The two lots combine to make one 3,500 square-foot lot. The proposed building will have a 1,955 square-foot footprint and the units range from 947 to 1,645 square feet. The side yard set backs are shown on drawing C-102 and total more than 10 feet. The proposed front yard is 1.5 feet, which is consistent with the average setback of the adjacent properties.

#### 14-526 (a) Transportation Standards

Romasco is directly off of Cumberland Avenue which is classified as a Major Collector. Four additional housing units would have an insignificant impact on the surrounding street systems. Romasco Lane is only 0.15 miles long and has a total travel time of less than 1 minute. The proposed development will not reduce the Level of Service below level "D" as required in in Section 14-526 of the City of Portland Code of Ordinances.

The property will be accessed through one curb cut off Romasco Lane in the same general location of the existing curb cut. The driveway will be greater than twenty feet from adjacent driveways and has a proposed width of ten feet, in accordance with Section 1.7 of the Portland Technical Manual. A sidewalk exists along the entire length of property's frontage with Romasco Lane and will be demolished and replaced by brick pavers.

The off-street parking space standards in Section 14-332.1(k) of the City of Portland Code of Ordinances require one parking space per dwelling unit over three in the (R-6)-District. The proposed development provides three parking spots in the first-floor garage, two more than required. Section 14-526 of the same document requires two bicycle parking spaces for developments with zero to ten vehicle parking spaces. Two bike hangers will be provided within the garage as shown on the Site and Utility Plan, Drawing C-101.

Section 14-526 of the City of Portland Code of Ordinances requires provisions be made for snow storage. The open space on the south west corner will provide sufficient space for snow storage, the required parking spaces are located in the garage and will not be impacted by snow storage.

### 14-526 (b) Environmental Quality Standards

## A. Preservation of Significant Natural Features

The site is currently fully developed with a paved parking area on the southern third of the property and an overgrown lawn area over the rest of the property. In addition, the property is in a highly urbanized neighborhood with little to no stands of trees, wetlands or wildlife habitat. Therefore, it is expected that there are no protected natural resources within the property footprint.

## B. Landscaping and Landscape Preservation

There is limited landscaping on the existing property. There is one large tree near the north-east corner. The project will result in the cutting of the tree and bushes in this area.

The proposed site landscaping is shown on the Landscape Plan, Drawing L-1. There will be no exterior servicing areas, dumpsters or on-site utility structures; therefore, screening is not required. There are no required landscaped islands required based on only three (3) proposed parking spaces.

Section 4 of the City of Portland Technical Manual requires that one street tree be planted per unit, unless otherwise approved and spaced thirty (30) to forty five (45) feet on center. The site has a total of sixty-three (63) feet of street frontage. No street trees are proposed at the frontage of the property, as shown on the Landscape Plan L-1. Given that there are no street trees along Romasco lane and the proximity of the building to the roadway. The owner requests a waiver from the installation of four street trees and will provide \$800 to the city's tree fund.

## C. Water Quality, Stormwater Management, and Erosion Control

The proposed development disturbs approximately 3,500 square feet (approximately 0.1 acres). According to Section 4 of the City of Portland Technical Manual any site disturbing less than one acre is exempt from complying with the Basic and General Standards, and acquiring a Stormwater Permit.

The details of the proposed stormwater management measures are included on the drawings and the Stormwater Management Report in Appendix F and the post construction stormwater management plan in Appendix G.

The proposed erosion control measures are detailed on the drawing set provided.

### 14-526 (c) Public Infrastructure and Community Safety Standards

### A. Consistency with City Master Plans

Romasco lane is not included in the Master Plan for Redevelopment of the Eastern Waterfront.

## B. Public Safety and Fire Prevention

The entrances to the building will be well-lit and visible from the street and adjacent walkways to provide natural surveillance as described in Section 3 of the City of Portland Technical Manual.

There is a fire hydrant located on the opposite sidewalk half way down Romasco Lane, within one hundred twenty (120) feet of the proposed building. The City of Portland Technical Manual requires a fire hydrant within five hundred (500) feet of all structures.

### C. Availability and Adequate Capacity of Public Utilities

The proposed development is within two hundred (200) feet of the public sanitary collection and treatment system and is therefore required to connect. A City of Portland Wastewater Capacity Application is included as Appendix H. Water service to the building will be provided from the 8-inch water main in Romasco Lane. A Capacity to Serve letter from PWD is included as Appendix I. The electrical and communication services to the building will be connected underground from the existing utility pole on the opposite side of the street. The utility connections are detailed on drawing C-101.

### 14-526 (d) Site Design Standards

## A. Massing, Ventilation and Wind Impact

The building will be consistent with the height and mass of other new construction in the area. The HVAC equipment will be located on the roof and will not negatively affect the abutting properties.

### B. Shadows

The building will be oriented such that there are no impacts to public open space or existing vegetation.

### C. Snow and Ice Loading

Snow storage will be provided on site where available and hauled away as necessary. Ice from the building is not in danger of falling onto abutting properties as there is at least five feet between the building and abutting property lines.

### D. View Corridors

The site is outside of Downtown Vision View Corridor Protection Plan.

### E. Historic Resources

Does not apply to this application.

#### F. Exterior Lighting

Lighting on site will provided with wall packs at each entrance.

The utility pole on the west side of Romasco Lane has an existing street light to illuminate the streets and walks in front of the property.

#### G. Noise and Vibration

The mechanical equipment will be on the roof of the proposed building and will be screened from Romasco Lane as shown on the Architectural elevations and floor plans.

#### H. Signage and Wayfinding

Does not apply to this application.

### I. Zone Related Design Standards

The proposed multi-family development in the R-6 zone will be architecturally compatible with the surrounding neighborhood. The orientation and placement in relationship to the street of the building is consistent with the neighboring structures. Details of compliance can be found on the architectural drawings and renderings.

#### J. Solid Waste Generation and Management

Disposal of construction waste will be the responsibility of the selected contractor. Domestic waste will be stored in the trash bin area in the Garage as shown on the floor plan.

### **CONSTRUCTION MANAGEMENT PLAN**

A Construction Management Plan is provided in Appendix J of the application.

The proposed development will have minimal impacts on the surrounding traffic patterns. During construction there will be additional truck and construction vehicle traffic, but two-lane traffic will not be interrupted. Due to the proximity to the sidewalk on Romasco Lane, foot traffic will be redirected by signs at the nearest intersections during building construction.

The installation of water service, sanitary sewer, and utilities will require a new opening in Romasco Lane. The resulting lane closers will be coordinated with the City and controlled by the contractor.

A written Construction Management Plan will be provided by the selected contractor prior to construction.

# **APPENDIX A**

**FIGURE 1 – SITE LOCATION MAP** 



# **APPENDIX B**

TITLE, RIGHT OR INTEREST

### WARRANTY DEED KNOW ALL MEN BY THESE PRESENTS

That I, JUNE P. LANCASTER a/k/a JUNE QUIMBY a/k/a JUNE P. QUMBY of Portland, in the County of Cumberland and State of Maine, in consideration of one dollar and other valuable consideration paid by LEGACY 18 DEVELOPMENT CORPORATION, a Maine corporation with a mailing address of 233 Smith Road, Windham, Maine 04062, receipt of which I hereby acknowledge, do hereby give, grant, bargain and sell unto the said LEGACY 18 DEVELOPMENT CORPORATION of Windham, Maine, with Warranty covenants, the following:

A certain lot or parcel of land with the buildings thereon, situated in said Portland on the Easterly side of Larch Street (now called Romasco Lane), bounded and described as follows: Commencing on the Easterly side of Larch Street at the Northwesterly corner of a lot of land now or formerly owned by John Kennedy; thence running Northerly by said Larch Street thirty-one (31) feet and six (6) inches, more or less, to land now or formerly owned by John Collins; thence Northeasterly by said Collins land sixty-three (63) feet, more or less, to the Southerly line of the Church lot, so-called; thence by the Church lot twenty-one (21) feet and five (5) inches, more or less, to the land now or formerly owned by the said Kennedy; thence Westerly by said Kennedy's land to Larch Street and the point begun at.

Being the same premises conveyed to June Quimby by the deed of Eleanora Salvatore by deed dated October 25, 1974 and recorded in the Cumberland County Registry of Deeds in Book 3614, Page 0313. The said June Quimby is in fact June P. Lancaster, which name is her birth name.

Also, another certain lot or parcel of land located on the Easterly side of Larch Street (now called Romasco Lane) in said Portland, bounded and described as follows: Commencing on said side of said street at the most Westerly corner of a lot of land called the Patrick Dougher lot, and thence running on said street Northerly thirty-one (31) feet and six (6) inches to a monument; thence Northeasterly about sixty-three (63) feet to a church lot; thence on a line of said Church lot Southerly about twenty-one (21) feet and four (4) inches to the widow Butler lot, so-called; thence Southwesterly on a line of said Butler lot and line of said Dougher lot to this corner and point of beginning on said Larch Street.

Being the same premises conveyed to June P. Quimby by the deed of the City of Portland, Maine dated January 3, 1977 and recorded in the Cumberland County Registry of Deeds in Book 3959, Page 0146. The said June P. Quimby is in fact June P. Lancaster, which is her birth name.

Witness my hand and seal this twenty-first day of August, 2014

weller

June P. Lancaster By: Stephen J. Schwartz, her Received Recorded Resister of Deeds attorney in fact Aus 22,2014 03:22:34P Cumberland County

Pamela E. Lovley

STATE OF MAINE CUMBERLAND, SS. August 21, 2014

Then personally appeared by said Stephen J. Schwartz of Portland, Maine, attorney in fact for the said June P. Lancaster a/k/a June Lancaster a/k/a June P. Lancaster, and acknowledged the foregoing instrument to be his free act and deed in his said capacity and the free act and deed of said June P. Lancaster.

Before me,

Notary Public/Attorney at Law

SUSAN GAGE KNEDLER Notary Public. Maine My Commission Expires November 22 2018 Assessor's Office | 389 Congress Street | Portland, Maine 04101 | Room 115 | (207) 874-8486

City Home Departments City Council E-Services Calendar Jobs

This page contains a detailed description of the Parcel ID you selected.
New Search!

#### **Current Owner Information:**

	CBL	013 J024001
Services	Land Use Type Verify legal use with	VACANT LAND
Applications	Inspections Division	
Doing Business	Property Location	5 ROMASCO LN
Maps	<b>Owner Information</b>	LEGACY 18 DEVELOPMENT CORPORATION
Tax Relief		WINDHAM ME 04062
Tax Roll	<b>Book and Page</b>	31728/266
Q & A	Legal Description	13-J-24 ROMASCO LN 5-7
browse city		1931 SF
services a-z	Acres	0.0443
browco facto and	Comment Assessed	TT-l

browse facts and links a-z

#### **Current Assessed Valuation:**

**TAX AMOUNT** 

TAX ACCT NO.	1520	OWNER OF RECORD AS OF APRIL 2015
LAND VALUE	\$10,000.00	LEGACY 18 DEVELOPMENT CORPORATION
BUILDING VALUE	\$0.00	233 SMITH RD
NET TAXABLE - REAL ESTAT	<b>E</b> \$10,000.00	WINDHAM ME 04062

\$206.30

Best viewed at 800x600, with Internet Explorer

Any information concerning tax payments should be directed to the Treasury office at 874-8490 or <u>e-mailed</u>.



View Map

#### Sales Information:

Sale Date	Туре	Price	Book/Page
8/22/2014	LAND	\$167,000.00	31728/266
	(	New Search!	

# **APPENDIX C**

FINANCIAL CAPACITY

1/20/2016

James Whelan Saco & Biddeford Savings Institution 50 Industrial Park Rd Saco ME 04072

To whom it may concern;

Please be advised that Legacy 18 Development Corp. has applied for and received preliminary approval for construction loan financing for the construction of 4 unit condominium at 9 Romasco Lane in Portland, Maine. The loan is contingent upon final underwriting and approval by the bank's Loan Committee and the issuance of all applicable building permits.

Sincerely,

em hell

James Whelan Vice President Saco & Biddeford Savings Institution

# **APPENDIX D**

**CITY OF PORTLAND GIS MAP** 



# **APPENDIX E**

FEMA FLOODPLAIN MAP



# **APPENDIX F**

STORMWATER MANAGEMENT REPORT

CITY OF PORTLAND, MAINE PLANNING BOARD STORMWATER MANAGEMENT REPORT

**Prepared for** 

LEGACY 18 DEVELOPMENT INC. 4-UNIT CONDO BUILDING

> 5 & 9 Romasco Lane Portland, Maine

> > January 2016



ENVIRONMENTAL • CIVIL • GEOTECHNICAL • WATER • COMPLIANCE



#### TABLE OF CONTENTS

Section No.	Title	Page No.
1.0 INTRODUCTION		1
2.0 PROJECT DESCRIPTION		1
3.0 SITE WATERSHED		1
4.0 STORMWATER QUANTITY ANALYSI	S	
5.0 SUMMARY		

#### LIST OF APPENDICES

APPENDIX A NRCS SOIL REPORT APPENDIX B PRE-DEVELOPMENT HYDROCAD CALCULATIONS APPENDIX C POST-DEVELOPMENT HYDROCAD CALCULATIONS

#### LIST OF TABLES

Tab	ole No.	Title	Page No.
1		ICREASE OF IMPERVIOUS AREA	- 2
2	STORMWATE	R QUANTITY SUMMARY	

#### STORMWATER MANAGEMENT REPORT

#### 1.0 INTRODUCTION

The following outlines the stormwater management design for the proposed condominium building and associated site improvements at 5 & 9 Romasco Lane in Portland, Maine. The stormwater design prepared by Sevee & Maher Engineers, Inc. (SME) is based on the water quality and quantity objectives identified in the City Ordinances and by the City of Portland Engineer during the pre-application meeting.

#### 2.0 PROJECT DESCRIPTION

Legacy 18 Development Inc. proposes to construct a four-story, 1,955-square-foot multifamily condominium building on the existing 3,496 square foot lot. The site is currently fully developed with 880 square feet of paved parking along Romasco Lane, a recently demolished residential building and gravel and overgrown lawn on the remainder of the site. The project will include the proposed building, 143 square feet of paved vehicular access and sidewalks, plantings, utility connections and an infiltration basin to address stormwater. The project will result in no increase in developed area and an increase in impervious area of 600 square feet. The increase in impervious area is less than 1,000 square feet, therefore the project is not required to provide stormwater treatment.

#### 3.0 SITE WATERSHED

On-site soils were identified using the Natural Resources Conservation Service (NRCS) soil information for Cumberland County, Maine. A copy of the custom Soil Resource Report is included in Appendix A. The soil within the area of work consists of Hinckley gravelly sandy loam (HIB) which is classified as "excessively drained" and hydrologic soil group (HSG) A soils.

In existing conditions, the majority of the site drains to the west and then flows onto Romasco Lane. The runoff from the eastern portion of the site is then captured in an existing Catch Basin in Romasco Lane. The Catch Basin is identified as Analysis Point 1.
The northern portion of the site drains onto the abutters property and into the curb and gutter system in Romasco Lane. From there the drainage enters the combined sewer through a Catch Basin 160' to the North. This Catch Basin has been identified as Analysis Point 2.

In developed conditions, the flow to westerly abutter's yard has been removed. The building roof drains and the areas to the east of the building will flow into a shallow infiltration basin in the backyard. The overflow from the infiltration basin during larger storms will be captured in a field inlet at the north corner of the building. From there the runoff will be conveyed into the existing catch basin in the street through a closed storm drain. The southern portion of the site will sheet flow to Romasco Lane, similar to existing conditions.

Pre-development and post-development stormwater management plans identify the on-site drainage patterns before and after development (See Drawings D-100 and D-101) and are included in the plan set. Appendices B and C provide pre- and post-development calculations using TR-20 methodologies prepared with the HydroCAD computer stormwater modeling system by Applied Microcomputer Systems of Chocorua, New Hampshire.

## 4.0 STORMWATER QUALITY ANALYSIS

Based on the City Standards Projects with a net increase of impervious area less than 1,000 sf are not required to provide stormwater treatment. However, an infiltration basin is proposed for this project to capture roof runoff at the rear of the building.

#### TABLE 1

#### PROPOSED INCREASE OF IMPERVIOUS AREA

	Existing (sf)	Post Development (sf)	Difference (sf)
Total Impervious			
Building	639	1,955	1,316
Pavement	865	143	-722
Open Space	1,992	1,398	594

\\nserver\CFS\Klenicki Design\5 Romasco Lane\City of Portland\Appendix F - Stormwater Management Report.doc Sevee & Maher Engineers, Inc. January 20, 2016

### 5.0 STORMWATER QUANTITY ANALYSIS

Stormwater quantity is managed to the maximum extent practicable through minimizing the amount of impervious area on the site and through the proposed infiltration basin on the east portion of the property. Table 2 below demonstrates peak flow rates from the subwatershed areas at the two analysis points shown on Drawings D-100 and D-101.

#### STORMWATER QUANTITY SUMMARY

	2-yr S	Storm	10-yr \$	Storm	25-yr Storm		
AP	Pre- Post- (cfs) (cfs)		Pre- Post- (cfs) (cfs)		Pre- Post- (cfs) (cfs)		
1	0.27	0.11	0.55	0.55	0.68	0.70	
2	0.37	0.36	1.29	1.17	1.80	1.80	

The post-development flows have been analyzed for the 2-year, 10-year and 25-year storms using HydroCAD (see Appendices C and D). The infiltration qualities of the soil for the infiltration basin were conservatively estimated at 1.42 inches/hour, based on the National Resources Conservation Service's characterization of the soils on site (Hinckley gravelly sandy loam).

## 6.0 MAINTENANCE AND INSPECTION

The owner or operator of a BMP shall hire a qualified post-construction stormwater inspector to at least annually, inspect the BMPs, including but not limited to any parking areas, catch basins, drainage swales, detention basins and ponds, pipes and related structures, in accordance with all municipal and state inspection, cleaning and maintenance requirements of the approved post-construction stormwater management plan.

If the BMP requires maintenance, repair or replacement to function as intended by the approved post-construction stormwater management plan, the owner or operator of the BMP shall take corrective action(s) to address the deficiency or deficiencies as soon as possible after the deficiency is discovered and shall provide a record of the deficiency and corrective action(s) to the department of public services ("DPS") in the annual report.

The owner or operator of a BMP or a qualified post-construction stormwater inspector hired by that person, shall, on or by June 30 of each year, provide a completed and signed certification to DPS in a form provided by DPS, certifying that the person has inspected the BMP(s) and that they are adequately maintained and functioning as intended by the approved post-construction stormwater management plan, or that they require maintenance or repair, including the record of the deficiency and corrective action(s) taken.

## 7.0 SUMMARY

The stormwater management for the 5 & 9 Romasco Lane project will have no adverse impact to the downstream drainage or abutting properties and additional storage and infiltration is provided to decrease flows to the City's combined sewer during rainfall events.

**APPENDIX A** 

NRCS SOIL REPORT



United States Department of Agriculture

NATURAL NATURAL

Resources Conservation Service A product of the National Cooperative Soil Survey, a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local participants Custom Soil Resource Report for Cumberland County and Part of Oxford County, Maine



## Preface

Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (http:// offices.sc.egov.usda.gov/locator/app?agency=nrcs) or your NRCS State Soil Scientist (http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/? cid=nrcs142p2\_053951).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

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# Contents

Preface	2
How Soil Surveys Are Made	5
Soil Map	7
Soil Map	8
Legend	9
Map Unit Legend	10
Map Unit Descriptions	10
Cumberland County and Part of Oxford County, Maine	12
HIB—Hinckley gravelly sandy loam, 3 to 8 percent slopes	12
References	13

# **How Soil Surveys Are Made**

Soil surveys are made to provide information about the soils and miscellaneous areas in a specific area. They include a description of the soils and miscellaneous areas and their location on the landscape and tables that show soil properties and limitations affecting various uses. Soil scientists observed the steepness, length, and shape of the slopes; the general pattern of drainage; the kinds of crops and native plants; and the kinds of bedrock. They observed and described many soil profiles. A soil profile is the sequence of natural layers, or horizons, in a soil. The profile extends from the surface down into the unconsolidated material in which the soil formed or from the surface down to bedrock. The unconsolidated material is devoid of roots and other living organisms and has not been changed by other biological activity.

Currently, soils are mapped according to the boundaries of major land resource areas (MLRAs). MLRAs are geographically associated land resource units that share common characteristics related to physiography, geology, climate, water resources, soils, biological resources, and land uses (USDA, 2006). Soil survey areas typically consist of parts of one or more MLRA.

The soils and miscellaneous areas in a survey area occur in an orderly pattern that is related to the geology, landforms, relief, climate, and natural vegetation of the area. Each kind of soil and miscellaneous area is associated with a particular kind of landform or with a segment of the landform. By observing the soils and miscellaneous areas in the survey area and relating their position to specific segments of the landform, a soil scientist develops a concept, or model, of how they were formed. Thus, during mapping, this model enables the soil scientist to predict with a considerable degree of accuracy the kind of soil or miscellaneous area at a specific location on the landscape.

Commonly, individual soils on the landscape merge into one another as their characteristics gradually change. To construct an accurate soil map, however, soil scientists must determine the boundaries between the soils. They can observe only a limited number of soil profiles. Nevertheless, these observations, supplemented by an understanding of the soil-vegetation-landscape relationship, are sufficient to verify predictions of the kinds of soil in an area and to determine the boundaries.

Soil scientists recorded the characteristics of the soil profiles that they studied. They noted soil color, texture, size and shape of soil aggregates, kind and amount of rock fragments, distribution of plant roots, reaction, and other features that enable them to identify soils. After describing the soils in the survey area and determining their properties, the soil scientists assigned the soils to taxonomic classes (units). Taxonomic classes are concepts. Each taxonomic class has a set of soil characteristics with precisely defined limits. The classes are used as a basis for comparison to classify soils systematically. Soil taxonomy, the system of taxonomic classification used in the United States, is based mainly on the kind and character of soil properties and the arrangement of horizons within the profile. After the soil scientists classified and named the soils in the survey area, they compared the

individual soils with similar soils in the same taxonomic class in other areas so that they could confirm data and assemble additional data based on experience and research.

The objective of soil mapping is not to delineate pure map unit components; the objective is to separate the landscape into landforms or landform segments that have similar use and management requirements. Each map unit is defined by a unique combination of soil components and/or miscellaneous areas in predictable proportions. Some components may be highly contrasting to the other components of the map unit. The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The delineation of such landforms and landform segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, onsite investigation is needed to define and locate the soils and miscellaneous areas.

Soil scientists make many field observations in the process of producing a soil map. The frequency of observation is dependent upon several factors, including scale of mapping, intensity of mapping, design of map units, complexity of the landscape, and experience of the soil scientist. Observations are made to test and refine the soillandscape model and predictions and to verify the classification of the soils at specific locations. Once the soil-landscape model is refined, a significantly smaller number of measurements of individual soil properties are made and recorded. These measurements may include field measurements, such as those for color, depth to bedrock, and texture, and laboratory measurements, such as those for content of sand, silt, clay, salt, and other components. Properties of each soil typically vary from one point to another across the landscape.

Observations for map unit components are aggregated to develop ranges of characteristics for the components. The aggregated values are presented. Direct measurements do not exist for every property presented for every map unit component. Values for some properties are estimated from combinations of other properties.

While a soil survey is in progress, samples of some of the soils in the area generally are collected for laboratory analyses and for engineering tests. Soil scientists interpret the data from these analyses and tests as well as the field-observed characteristics and the soil properties to determine the expected behavior of the soils under different uses. Interpretations for all of the soils are field tested through observation of the soils in different uses and under different levels of management. Some interpretations are modified to fit local conditions, and some new interpretations are developed to meet local needs. Data are assembled from other sources, such as research information, production records, and field experience of specialists. For example, data on crop yields under defined levels of management are assembled from farm records and from field or plot experiments on the same kinds of soil.

Predictions about soil behavior are based not only on soil properties but also on such variables as climate and biological activity. Soil conditions are predictable over long periods of time, but they are not predictable from year to year. For example, soil scientists can predict with a fairly high degree of accuracy that a given soil will have a high water table within certain depths in most years, but they cannot predict that a high water table will always be at a specific level in the soil on a specific date.

After soil scientists located and identified the significant natural bodies of soil in the survey area, they drew the boundaries of these bodies on aerial photographs and identified each as a specific map unit. Aerial photographs show trees, buildings, fields, roads, and rivers, all of which help in locating boundaries accurately.

# Soil Map

The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.



	MAP L	EGEND	)	MAP INFORMATION		
Area of In	terest (AOI)	100	Spoil Area	The soil surveys that comprise your AOI were mapped at 1:24,000.		
	Area of Interest (AOI)		Stony Spot			
Soils		ň	Very Stony Spot	Warning: Soil Map may not be valid at this scale.		
	Soil Map Unit Polygons	60 (1)	Wet Spot	Enlargement of mans beyond the scale of manning can cause		
~	Soil Map Unit Lines	A N	Other	misunderstanding of the detail of mapping and accuracy of soil line		
	Soil Map Unit Points		Special Line Features	placement. The maps do not show the small areas of contrasting		
Special	Special Point Features			Sons that could have been shown at a more detailed scale.		
అ	(b) Blowout		Streams and Canals	Please rely on the bar scale on each map sheet for map		
	Borrow Pit	Transport	tation	measurements.		
英	Clay Spot	++++	Rails	Source of Man: Natural Becources Concentration Service		
$\diamond$	Closed Depression	~	Interstate Highways	Web Soil Survey URL: http://websoilsurvey.nrcs.usda.gov		
X	Gravel Pit	~	US Routes	Coordinate System: Web Mercator (EPSG:3857)		
0 0 0	Gravelly Spot	~	Major Roads	Maps from the Web Soil Survey are based on the Web Mercator		
0	Landfill		Local Roads	projection, which preserves direction and shape but distorts		
A	Lava Flow	Backgrou	ind	distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate		
ماند ماند	Marsh or swamp	Duckgrou	Aerial Photography	calculations of distance or area are required.		
\$	Mine or Quarry			This product is apparented from the LISDA NDCS sortified data as of		
0	Miscellaneous Water			the version date(s) listed below.		
ŏ	Perennial Water					
Š	Rock Outcrop			Soil Survey Area: Cumberland County and Part of Oxford County, Maine		
Ť	Saline Spot			Survey Area Data: Version 9, Sep 13, 2014		
T •_•	Sandy Spot			Soil man units are labeled (as space allows) for man scalos 1:50 000		
°° •	Severely Froded Spot			or larger.		
÷	Sinkhole					
×				Date(s) aerial images were photographed: Jul 31, 2013—Aug 11, 2013		
\$						
ø	Soaic Spot			The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting		
				of map unit boundaries may be evident.		

## Map Unit Legend

Cumberland County and Part of Oxford County, Maine (ME005)							
Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI				
НІВ	Hinckley gravelly sandy loam, 3 to 8 percent slopes	1.9	100.0%				
Totals for Area of Interest	•	1.9	100.0%				

## **Map Unit Descriptions**

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas. An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An association is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

## **Cumberland County and Part of Oxford County, Maine**

#### HIB—Hinckley gravelly sandy loam, 3 to 8 percent slopes

#### Map Unit Setting

National map unit symbol: blhp Elevation: 10 to 2,000 feet Mean annual precipitation: 30 to 48 inches Mean annual air temperature: 37 to 46 degrees F Frost-free period: 90 to 160 days Farmland classification: Farmland of statewide importance

#### **Map Unit Composition**

*Hinckley and similar soils:* 85 percent *Estimates are based on observations, descriptions, and transects of the mapunit.* 

#### **Description of Hinckley**

#### Setting

Landform: Outwash terraces Landform position (two-dimensional): Toeslope Landform position (three-dimensional): Tread Down-slope shape: Linear Across-slope shape: Linear Parent material: Sandy-skeletal glaciofluvial deposits derived from granite and gneiss

#### **Typical profile**

Oe - 0 to 1 inches: moderately decomposed plant material

H1 - 1 to 8 inches: gravelly sandy loam

H2 - 8 to 11 inches: gravelly sandy loam

H3 - 11 to 25 inches: gravelly loamy sand

H4 - 25 to 65 inches: very gravelly sand

#### **Properties and qualities**

Slope: 3 to 8 percent
Depth to restrictive feature: More than 80 inches
Natural drainage class: Excessively drained
Capacity of the most limiting layer to transmit water (Ksat): Moderately high to very high (1.42 to 14.17 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Available water storage in profile: Very low (about 2.6 inches)

#### Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 3s Hydrologic Soil Group: A

# References

American Association of State Highway and Transportation Officials (AASHTO). 2004. Standard specifications for transportation materials and methods of sampling and testing. 24th edition.

American Society for Testing and Materials (ASTM). 2005. Standard classification of soils for engineering purposes. ASTM Standard D2487-00.

Cowardin, L.M., V. Carter, F.C. Golet, and E.T. LaRoe. 1979. Classification of wetlands and deep-water habitats of the United States. U.S. Fish and Wildlife Service FWS/OBS-79/31.

Federal Register. July 13, 1994. Changes in hydric soils of the United States.

Federal Register. September 18, 2002. Hydric soils of the United States.

Hurt, G.W., and L.M. Vasilas, editors. Version 6.0, 2006. Field indicators of hydric soils in the United States.

National Research Council. 1995. Wetlands: Characteristics and boundaries.

Soil Survey Division Staff. 1993. Soil survey manual. Soil Conservation Service. U.S. Department of Agriculture Handbook 18. http://www.nrcs.usda.gov/wps/portal/nrcs/ detail/national/soils/?cid=nrcs142p2 054262

Soil Survey Staff. 1999. Soil taxonomy: A basic system of soil classification for making and interpreting soil surveys. 2nd edition. Natural Resources Conservation Service, U.S. Department of Agriculture Handbook 436. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/?cid=nrcs142p2 053577

Soil Survey Staff. 2010. Keys to soil taxonomy. 11th edition. U.S. Department of Agriculture, Natural Resources Conservation Service. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/?cid=nrcs142p2 053580

Tiner, R.W., Jr. 1985. Wetlands of Delaware. U.S. Fish and Wildlife Service and Delaware Department of Natural Resources and Environmental Control, Wetlands Section.

United States Army Corps of Engineers, Environmental Laboratory. 1987. Corps of Engineers wetlands delineation manual. Waterways Experiment Station Technical Report Y-87-1.

United States Department of Agriculture, Natural Resources Conservation Service. National forestry manual. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/ home/?cid=nrcs142p2\_053374

United States Department of Agriculture, Natural Resources Conservation Service. National range and pasture handbook. http://www.nrcs.usda.gov/wps/portal/nrcs/ detail/national/landuse/rangepasture/?cid=stelprdb1043084

United States Department of Agriculture, Natural Resources Conservation Service. National soil survey handbook, title 430-VI. http://www.nrcs.usda.gov/wps/portal/ nrcs/detail/soils/scientists/?cid=nrcs142p2 054242

United States Department of Agriculture, Natural Resources Conservation Service. 2006. Land resource regions and major land resource areas of the United States, the Caribbean, and the Pacific Basin. U.S. Department of Agriculture Handbook 296. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/? cid=nrcs142p2\_053624

United States Department of Agriculture, Soil Conservation Service. 1961. Land capability classification. U.S. Department of Agriculture Handbook 210. http://www.nrcs.usda.gov/Internet/FSE\_DOCUMENTS/nrcs142p2\_052290.pdf

**APPENDIX B** 

PRE-DEVELOPMENT HYDROCAD CALCULATIONS



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## Area Listing (all nodes)

Area	CN	Description
(acres)		(subcatchment-numbers)
0.463	39	>75% Grass cover, Good, HSG A (1S, 2S)
0.357	98	Impervious (2S)
0.116	98	Paved parking, HSG A (1S)
0.936	69	TOTAL AREA

## Soil Listing (all nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
0.579	HSG A	1S, 2S
0.000	HSG B	
0.000	HSG C	
0.000	HSG D	
0.357	Other	2S
0.936		TOTAL AREA

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HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers			
 0.463	0.000	0.000	0.000	0.000	0.463	>75% Grass cover, Good	1S, 2S			
0.000	0.000	0.000	0.000	0.357	0.357	Impervious	2S			
0.116	0.000	0.000	0.000	0.000	0.116	Paved parking	1S			
0.579	0.000	0.000	0.000	0.357	0.936	TOTAL AREA				

## Ground Covers (all nodes)

<b>EXISTING Klenicki- 5-9 Romasco L</b> Prepared by Sevee and Maher Engineer	<b>ane</b> s, Inc.	Туре	III 24-hr	2-Year Rain Printed	fall=3.00" 1/20/2016
HydroCAD® 10.00 s/n 01260 © 2012 HydroCA	D Software Solutions	LLC			Page 5
Time span=0.00- Runoff by S Reach routing by Stor-Ind+Tr	30.00 hrs, dt=0.01 h SCS TR-20 method, ans method - Ponc	nrs, 3001 j UH=SCS d routing b	points y Stor-Inc	d method	
Subcatchment1S: South	Runoff Area=6,978 T	8 sf   72.48 ⁻c=5.0 min	% Impervi CN=82	ious Runoff D Runoff=0.27 c	epth=1.38" fs_0.018 af
Subcatchment 2S: North	Runoff Area=33,810 Flow Length=260' T	0 sf 46.04 <sup>-</sup> c=6.6 min	% Impervi CN=66	ious Runoff D Runoff=0.37 c	epth=0.54" fs_0.035 af
Pond CB-1:			F	Inflow=0.27 c Primary=0.27 c	fs  0.018 af fs  0.018 af
Pond CB-2:			F	Inflow=0.37 c Primary=0.37 c	fs  0.035 af fs  0.035 af
Total Runoff Area = 0.936 a	ac Runoff Volume 49.44% Pervious =	= 0.054 a 0.463 ac	f Avera 50.56%	ge Runoff De ⁄₀ Impervious	epth = 0.69" = 0.473 ac

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#### Summary for Subcatchment 1S: South

Runoff = 0.27 cfs @ 12.08 hrs, Volume= 0.018 af, Depth= 1.38"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 2-Year Rainfall=3.00"

A	rea (sf)	CN	Description				
	1,920	39	>75% Gras	s cover, Go	ood, HSG A		
	5,058	98	Paved park	ing, HSG A			
	6,978	82	Weighted Average				
	1,920		27.52% Per	vious Area			
	5,058		72.48% Impervious Area				
То	Longth	Slope	Volocity	Canacity	Description		
	Lengin	Siope		Capacity	Description		
<u>(min)</u>	(feet)	(ft/ft	) (ft/sec)	(CfS)			
5.0					Direct Entry, 5 MIN MINIMUM		
					•		

#### Summary for Subcatchment 2S: North

Runoff = 0.37 cfs @ 12.12 hrs, Volume= 0.035 af, Depth= 0.54"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 2-Year Rainfall=3.00"

	A	rea (sf)	CN	Description			
		18,245	39	>75% Gras	s cover, Go	ood, HSG A	
*		15,565	98	Impervious			
		33,810	66	Weighted A	verage		
		18,245		53.96% Pei	vious Area		
		15,565		46.04% Imp	pervious Ar	ea	
	ŢĊ	Length	Slope	Velocity	Capacity	Description	
	(min)	(teet)	(ft/ft)	(ft/sec)	(cts)		
	5.7	54	0.0250	0.16		Sheet Flow, A-B	
						Grass: Short n= 0.150 P2= 3.00"	
	0.9	206	0.0339	3.74		Shallow Concentrated Flow, B-C	
						Paved Kv= 20.3 fps	
	6.6	260	Total				

## Summary for Pond CB-1:

Inflow Are	ea =	0.160 ac, 72.48% Imper	rvious, Inflow D	epth = 1.3	8" for 2-Year event
Inflow	=	0.27 cfs @ 12.08 hrs, \	√olume=	0.018 af	
Primary	=	0.27 cfs @ 12.08 hrs, \	Volume=	0.018 af,	Atten= 0%, Lag= 0.0 min

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## Summary for Pond CB-2:

Inflow Ar	ea =	0.776 ac, 4	46.04% Impervious,	Inflow Depth = (	).54" for 2-Year event	
Inflow	=	0.37 cfs @	12.12 hrs, Volume	e 0.035 a	f	
Primary	=	0.37 cfs @	12.12 hrs, Volume	e 0.035 a	f, Atten= 0%, Lag= 0.0 m	nin

EXISTING Klenicki- 5-9 Romasco L Prepared by Sevee and Maher Engineer	<b>ane</b> rs, Inc.	Туре	III 24-hr	<i>10-Year Rain</i> Printed	fall <b>=4</b> .70" 1/20/2016
<u>HydroCAD® 10.00 s/n 01260 © 2012 HydroCA</u>	AD Software Solution	ns LLC			Page 8
Time span=0.00 Runoff by Stor-Ind+Tr Reach routing by Stor-Ind+Tr	-30.00 hrs, dt=0.01 SCS TR-20 metho ans method - Po	l hrs, 300 d, UH=S0 nd routing	1 points CS g by Stor-Ir	nd method	
Subcatchment1S: South	Runoff Area=6,9	978 sf 72 Tc=5.0 m	.48% Imper iin CN=82	vious Runoff D Runoff=0.55 cl	epth=2.81" fs_0.038 af
Subcatchment 2S: North	Runoff Area=33,8 Flow Length=260'	310 sf 46 Tc=6.6 m	.04% Imper iin CN=66	vious Runoff D Runoff=1.29 ct	epth=1.53" fs_0.099 af
Pond CB-1:				Inflow=0.55 c Primary=0.55 c	fs  0.038 af fs  0.038 af
Pond CB-2:				Inflow=1.29 c Primary=1.29 c	fs  0.099 af fs  0.099 af
Total Runoff Area = 0.936 a	ac Runoff Volum 49.44% Pervious	ne = 0.13 = 0.463 a	6 af Aver ac 50.56	age Runoff De % Impervious	epth = 1.75" = 0.473 ac

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#### Summary for Subcatchment 1S: South

Runoff = 0.55 cfs @ 12.07 hrs, Volume= 0.038 af, Depth= 2.81"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10-Year Rainfall=4.70"

A	rea (sf)	CN	Description				
	1,920	39	>75% Gras	s cover, Go	bod, HSG A		
	5,058	98	Paved park	ing, HSG A			
	6,978	82	Weighted A	verage			
	1,920		27.52% Pervious Area				
	5,058		72.48% Imp	pervious Ar	ea		
_				<b>.</b>			
Тс	Length	Slope	e Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	) (ft/sec)	(cfs)			
5.0					Direct Entry, 5 MIN MINIMUM		

#### Summary for Subcatchment 2S: North

Runoff = 1.29 cfs @ 12.10 hrs, Volume= 0.099 af, Depth= 1.53"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10-Year Rainfall=4.70"

	A	rea (sf)	CN	Description			
		18,245	39	>75% Gras	s cover, Go	ood, HSG A	
*		15,565	98	Impervious			
		33,810	66	Weighted A	verage		
		18,245		53.96% Pei	vious Area		
	15,565 46.04% Impervious Area						
	ŢĊ	Length	Slope	Velocity	Capacity	Description	
	(min)	(teet)	(ft/ft)	) (ft/sec)	(cts)		
	5.7	54	0.0250	0.16		Sheet Flow, A-B	
						Grass: Short n= 0.150 P2= 3.00"	
	0.9	206	0.0339	3.74		Shallow Concentrated Flow, B-C	
						Paved Kv= 20.3 fps	
	6.6	260	Total				

## Summary for Pond CB-1:

Inflow A	rea =	0.160 ac, 72.48% Imp	ervious, Inflow	Depth = $2.81''$	for 10-Year event
Inflow	=	0.55 cfs @ 12.07 hrs,	Volume=	0.038 af	
Primary	=	0.55 cfs @ 12.07 hrs,	Volume=	0.038 af, Att	en= 0%, Lag= 0.0 min

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## Summary for Pond CB-2:

Inflow A	rea =	0.776 ac, 46.	04% Impervious,	Inflow Depth = 1	.53" for 10-	Year event
Inflow	=	1.29 cfs @ 12	2.10 hrs, Volume	e 0.099 at	f	
Primary	=	1.29 cfs @ 12	2.10 hrs, Volume	e 0.099 at	f, Atten= 0%,	Lag= 0.0 min

<b>EXISTING Klenicki- 5-9 Romasco L</b> Prepared by Sevee and Maher Engineer	<b>ane</b> rs, Inc.	Туре	III 24-hr	25-Year Rainf Printed	<i>all=5.50"</i> 1/20/2016
HydroCAD® 10.00 s/n 01260 © 2012 HydroCA	D Software Solution	ns LLC			Page 11
Time span=0.00- Runoff by S Reach routing by Stor-Ind+Tr	30.00 hrs, dt=0.01 SCS TR-20 method ans method - Por	hrs, 300 d, UH=S0 nd routing	1 points CS g by Stor-Ir	nd method	
Subcatchment1S: South	Runoff Area=6,9	78 sf 72 Tc=5.0 m	.48% Imper iin CN=82	vious Runoff De Runoff=0.68 cf	epth=3.53" s_0.047 af
Subcatchment 2S: North	Runoff Area=33,8 Flow Length=260'	10 sf 46 Tc=6.6 m	.04% Imper iin CN=66	vious Runoff De Runoff=1.80 cf	epth=2.08" s_0.134 af
Pond CB-1:				Inflow=0.68 cf Primary=0.68 cf	s  0.047 af s  0.047 af
Pond CB-2:				Inflow=1.80 cf Primary=1.80 cf	s  0.134 af s  0.134 af
Total Runoff Area = 0.936 a	ac Runoff Volum 49.44% Pervious	ie = 0.18 <sup>,</sup> = 0.463 a	1 af Aver ac 50.56	age Runoff De % Impervious	pth = 2.33" = 0.473 ac

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#### Summary for Subcatchment 1S: South

Runoff = 0.68 cfs @ 12.07 hrs, Volume= 0.047 af, Depth= 3.53"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 25-Year Rainfall=5.50"

A	rea (sf)	CN	Description			
	1,920	39	>75% Gras	s cover, Go	ood, HSG A	
	5,058	98	Paved park	ing, HSG A	l l l l l l l l l l l l l l l l l l l	
	6,978	82	Weighted A	verage		
	1,920		27.52% Pervious Area			
	5,058		72.48% Imp	pervious Ar	ea	
-			N / I · · ·	0		
IC	Length	Slope	e Velocity	Capacity	Description	
<u>(min)</u>	(feet)	(ft/ft	) (ft/sec)	(cfs)		
5.0					Direct Entry, 5 MIN MINIMUM	
					-	

#### Summary for Subcatchment 2S: North

Runoff	=	1.80 cfs @	12.10 hrs,	Volume=	0.134 af,	Depth= 2.08"
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Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 25-Year Rainfall=5.50"

	A	rea (sf)	CN	Description			
		18,245	39	>75% Gras	s cover, Go	ood, HSG A	
*		15,565	98	Impervious			
		33,810	66	Weighted A	verage		
	18,245 53.96% Pervious Area						
	15,565 46.04% Impervious Area						
	Tc (min)	Length (feet)	Slope (ft/ft)	velocity (ft/sec)	Capacity (cfs)	Description	
	5.7	54	0.0250	0.16		Sheet Flow, A-B	
	0.9	206	0.0339	3.74		Grass: Short n= 0.150 P2= 3.00" Shallow Concentrated Flow, B-C Paved Kv= 20.3 fps	
	6.6	260	Total				

## Summary for Pond CB-1:

Inflow Are	ea =	0.160 ac, 72.48% Imper	vious, Inflow Dept	th = 3.5	53" for 25-Year event
Inflow	=	0.68 cfs @ 12.07 hrs, V	/olume= 0	.047 af	
Primary	=	0.68 cfs @_ 12.07 hrs, ∖	/olume= 0	.047 af,	Atten= 0%, Lag= 0.0 min

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## Summary for Pond CB-2:

Inflow A	rea =	0.776 ac, 4	16.04% Impe	ervious,	Inflow Depth =	2.0	)8" for 25-	Year event
Inflow	=	1.80 cfs @	12.10 hrs,	Volume	= 0.134	af		
Primary	=	1.80 cfs @	12.10 hrs,	Volume	= 0.134	af,	Atten= 0%,	Lag= 0.0 min

**APPENDIX C** 

**POST-DEVELOPMENT HYDROCAD CALCULATIONS** 



## PROPOSED Klenicki- 5-9 Romasco Lane

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## Area Listing (all nodes)

Area	CN	Description
(acres)		(subcatchment-numbers)
0.449	39	>75% Grass cover, Good, HSG A (1S, 2S, 3S)
0.408	98	Impervious (2S, 3S)
0.021	98	Paved parking, HSG A (1S)
0.058	98	Unconnected roofs, HSG C (1S)
0.936	70	TOTAL AREA

## Soil Listing (all nodes)

Area	Soil	Subcatchment		
(acres)	Group	Numbers		
0.471	HSG A	1S, 2S, 3S		
0.000	HSG B			
0.058	HSG C	1S		
0.000	HSG D			
0.408	Other	2S, 3S		
0.936		TOTAL AREA		
Prepared by Sev	/ee and N	laher Engine	ers, Inc.	
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HSG-A (acres)	HSG-B (acres)	HSG-C (acres)	HSG-D (acres)	Other (acres)	Total (acres)	Ground Cover	Subcatchment Numbers
 0.449	0.000	0.000	0.000	0.000	0.449	>75% Grass cover, Good	1S, 2S,
							3S
0.000	0.000	0.000	0.000	0.408	0.408	Impervious	2S, 3S
0.021	0.000	0.000	0.000	0.000	0.021	Paved parking	1S
0.000	0.000	0.058	0.000	0.000	0.058	Unconnected roofs	1S
0.471	0.000	0.058	0.000	0.408	0.936	TOTAL AREA	

# Ground Covers (all nodes)

PROPOSED Klenicki- 5-9 Romas	co Lane Type III 24-hr 2-Year Rainfall=3.00"
HydroCAD® 10.00 s/n 01260 © 2012 Hydro	DCAD Software Solutions LLC Printed 1/20/2010
Time span=0.	00-30.00 hrs, dt=0.01 hrs, 3001 points
Reach routing by Stor-Ind-	+Trans method - Pond routing by Stor-Ind method
0.7	
Subcatchment1S: East	Runoff Area=4,737 st 72.83% Impervious Runoff Depth=1.38" Tc=5.0 min CN=82 Runoff=0.18 cfs 0.012 af
Subcatchment2S: North	Runoff Area=33,034 sf 47.12% Impervious Runoff Depth=0.58" Flow Length=306' Tc=10.2 min CN=67 Runoff=0.36 cfs 0.037 af
Subcatchment3S: South	Runoff Area=3,002 sf   72.92% Impervious   Runoff Depth=1.38" Tc=5.0 min   CN=82   Runoff=0.11 cfs   0.008 af
Reach 1R: n=0.013	Avg. Flow Depth=0.02' Max Vel=2.88 fps Inflow=0.07 cfs 0.005 af L=69.0' S=0.0957 '/' Capacity=45.86 cfs Outflow=0.07 cfs 0.005 af
Pond CB-1	Inflow=0.12 cfs. 0.013 af
	Primary=0.12 cfs 0.013 af
Pond CB-2:	Inflow=0.36 cfs 0.037 af
	Primary=0.36 cfs 0.037 af
Pond P1: Infiltration Basin	Peak Elev=101.65' Storage=211 cf Inflow=0.18 cfs 0.012 af
Discarded=0.0	00 cfs 0.004 af Primary=0.07 cfs 0.005 af Outflow=0.07 cfs 0.009 af
Total Runoff Area = 0.93	36 ac Runoff Volume = 0.057 af Average Runoff Depth = 0.74" 47.99% Pervious = 0.449 ac 52.01% Impervious = 0.487 ac

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### Summary for Subcatchment 1S: East

Runoff = 0.18 cfs @ 12.08 hrs, Volume= 0.012 af, Depth= 1.38"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 2-Year Rainfall=3.00"

A	rea (sf)	CN	Description					
	2,518	98	Unconnecte	ed roofs, HS	SG C			
	1,287	39	>75% Gras	s cover, Go	bod, HSG A			
	932	98	Paved park	ing, HSG A	A			
	4,737	82	Weighted A	verage				
	1,287		27.17% Pervious Area					
	3,450		72.83% Impervious Area					
	2,518		72.99% Un	connected				
Tc	Length	Slope	e Velocity	Capacity	Description			
<u>(min)</u>	(feet)	(ft/ft	) (ft/sec)	(cfs)				
5.0					Direct Entry, 5 minute min			
					-			

# Summary for Subcatchment 2S: North

Runoff = 0.36 cfs @ 12.17 hrs, Volume= 0.037 af, Depth= 0.58"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 2-Year Rainfall=3.00"

	A	rea (sf)	CN I	Description						
		17,469	39 :	>75% Gras	s cover, Go	ood, HSG A				
*		15,565	98 I	mpervious						
		33,034	67	Weighted Average						
		17,469	į	52.88% Pervious Area						
		15,565	565 47.12% Impervious Area							
	_									
	TC	Length	Slope	Velocity	Capacity	Description				
	(min)	(feet)	(ft/ft)	(ft/sec)	(cts)					
	9.3	100	0.0250	0.18		Sheet Flow, A-B				
						Grass: Short n= 0.150 P2= 3.00"				
	0.9	206	0.0339	3.74		Shallow Concentrated Flow, B-C				
						Paved Kv= 20.3 fps				
	10.2	306	Total							

### Summary for Subcatchment 3S: South

Runoff = 0.11 cfs @ 12.08 hrs, Volume= 0.008 af, Depth= 1.38"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 2-Year Rainfall=3.00"

 Type III 24-hr
 2-Year Rainfall=3.00"

 Printed
 1/20/2016

 C
 Page 7

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A	rea (sf)	CN	Description						
	813	39	39 >75% Grass cover, Good, HSG A						
*	2,189	98	Impervious						
	3,002	82	Weighted A	verage					
	813		27.08% Per	vious Area					
	2,189		72.92% Imp	ervious Are	ea				
Tc (min)	Length (feet)	Slope (ft/ft	e Velocity ) (ft/sec)	Capacity (cfs)	Description	I			
5.0					Direct Ent	ry, 5 minute m	in		
				•	· -				
	Summary for Reach 1R:								
Inflow Ar	ea = =	0.109	9 ac, 72.83% cfs @ 12.32	% Impervio 2 hrs. Volu	us, Inflow D ime=	epth = 0.55" 0.005 af	for 2-Year event		
Outflow	=	0.07	ofs @ 12.3	3 hrs, Volu	ime=	0.005 af, Atte	en= 0%, Lag= 0.7 min		
Routing by Stor-Ind+Trans method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Max. Velocity= 2.88 fps, Min. Travel Time= 0.4 min Avg. Velocity = 1.68 fps, Avg. Travel Time= 0.7 min									
Peak Storage= 2 cf @ 12.32 hrs Average Depth at Peak Storage= 0.02' Bank-Full Depth= 1.00' Flow Area= 2.0 sf, Capacity= 45.86 cfs									
1.00' x <sup>-</sup> Side Slor	1.00' deej	p chan	nel, n= 0.01	3 Asphalt,	smooth				

Side Slope Z-value = 1.0 '/' Top Width = 3.00' Length = 69.0' Slope = 0.0957 '/' Inlet Invert = 101.60', Outlet Invert = 95.00'



# Summary for Pond CB-1:

Inflow /	Area	=	0.178 ac, 7	72.86% Impervious,	Inflow Depth = 0.	87" for 2-Year event
Inflow	:	=	0.12 cfs @	12.31 hrs, Volume	e= 0.013 af	
Primar	y :	=	0.12 cfs @	12.31 hrs, Volume	e= 0.013 af,	Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

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# Summary for Pond CB-2:

Inflow /	Area =	0.758	ac, 47.12% In	npervious,	Inflow Depth =	0.5	8" for 2-Y	ear event
Inflow	=	0.36 cf	fs @ 12.17 hr	s, Volume	= 0.037	af		
Primar	y =	0.36 cf	fs @12.17 hr	s, Volume	= 0.037	af,	Atten= 0%,	Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

### **Summary for Pond P1: Infiltration Basin**

Inflow Area	a =	0.109 ac, 7	2.83% Imp	ervious, Ir	nflow Depth =	1.38"	for 2-Ye	ar event	
Inflow	=	0.18 cfs @	12.08 hrs,	Volume=	0.012	af			
Outflow	=	0.07 cfs @	12.32 hrs,	Volume=	0.009	af, Atte	n= 60%,	Lag= 14.6 m	າin
Discarded	=	0.00 cfs @	12.32 hrs,	Volume=	0.004	af		-	
Primary	=	0.07 cfs @	12.32 hrs,	Volume=	0.005	af			

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 101.65' @ 12.32 hrs Surf.Area= 331 sf Storage= 211 cf

Plug-Flow detention time= 249.6 min calculated for 0.009 af (73% of inflow) Center-of-Mass det. time= 156.3 min ( 994.8 - 838.5 )

Volume	Inve	ert Avail.Sto	rage Sto	rage Description	
#1	100.0	0' 3	45 cf Cu	stom Stage Data (P	Prismatic)Listed below (Recalc)
Elevatio	on et)	Surf.Area (sq-ft)	Inc.Stor (cubic-fee	e Cum.Store t) (cubic-feet)	
100.0	00	5		0 0	
101.0	00	121	6	3 63	
101.3	30	219	5	1 114	
102.0	00	440	23	1 345	
Device	Routing	Invert	Outlet De	vices	
#1 Discarded #2 Primary		d 101.30' 101.60'	1.420 in/ Conducti Excluded 2.0' long Head (fee Coef. (Er	hr Exfiltration over vity to Groundwater Surface area = 219 x 0.5' breadth Bro et) 0.20 0.40 0.60 iglish) 2.80 2.92 3	<b>Surface area above 101.30'</b> Elevation = 0.00' 9 sf <b>ad-Crested Rectangular Weir</b> 0.80 1.00 .08 3.30 3.32

**Discarded OutFlow** Max=0.00 cfs @ 12.32 hrs HW=101.65' (Free Discharge) **1=Exfiltration** (Controls 0.00 cfs)

Primary OutFlow Max=0.07 cfs @ 12.32 hrs HW=101.65' (Free Discharge) -2=Broad-Crested Rectangular Weir (Weir Controls 0.07 cfs @ 0.65 fps)

PROPOSED Klenicki- 5-9 Romas Prepared by Sevee and Maher Engine HydroCAD® 10.00 s/n 01260 © 2012 Hydro	<b>co Lane</b> eers, Inc. oCAD Software Solutions L	Type III 24-hr LC	10-Year Rainfall=4.70" Printed 1/20/2016 Page 9					
Time span=0.00-30.00 hrs, dt=0.01 hrs, 3001 points Runoff by SCS TR-20 method, UH=SCS Reach routing by Stor-Ind+Trans method . Pond routing by Stor-Ind method								
Subcatchment1S: East	Runoff Area=4,737	sf 72.83% Impei	vious Runoff Depth=2.81"					
	Tc	=5.0 min CN=82	Runoff=0.37 cfs 0.025 af					
Subcatchment2S: North	Runoff Area=33,034	sf 47.12% Impei	vious Runoff Depth=1.60"					
	Flow Length=306' Tc=	10.2 min CN=67	Runoff=1.17 cfs 0.101 af					
Subcatchment3S: South	Runoff Area=3,002	sf 72.92% Impei	vious Runoff Depth=2.81"					
	Tc	=5.0 min CN=82	Runoff=0.24 cfs 0.016 af					
<b>Reach 1R:</b> n=0.013	Avg. Flow Depth=0.06'	Max Vel=5.19 fp:	s Inflow=0.34 cfs 0.018 af					
	L=69.0' S=0.0957 '/' Ca	pacity=45.86 cfs	Outflow=0.34 cfs 0.018 af					
Pond CB-1:			Inflow=0.56 cfs 0.034 af Primary=0.56 cfs 0.034 af					
Pond CB-2:			Inflow=1.17 cfs 0.101 af Primary=1.17 cfs 0.101 af					
Pond P1: Infiltration Basin	Peak Elev=101.7	6' Storage=246 c	f Inflow=0.37 cfs 0.025 af					
Discarded=0.0	0 cfs 0.004 af Primary=0	).34 cfs  0.018 af	Outflow=0.35 cfs 0.022 af					
Total Runoff Area = 0.93	6 ac Runoff Volume =	= 0.143 af Avei	rage Runoff Depth = 1.83"					
	47.99% Pervious = 0	0.449 ac 52.01	I% Impervious = 0.487 ac					

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### Summary for Subcatchment 1S: East

Runoff = 0.37 cfs @ 12.07 hrs, Volume= 0.025 af, Depth= 2.81"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10-Year Rainfall=4.70"

A	rea (sf)	CN	Description					
	2,518	98	Unconnecte	ed roofs, HS	SGC			
	1,287	39	>75% Gras	s cover, Go	bod, HSG A			
	932	98	Paved park	ing, HSG A	A			
	4,737	82	Weighted A	verage				
	1,287		27.17% Pervious Area					
	3,450		72.83% Imp	pervious Are	ea			
	2,518		72.99% Un	connected				
Tc	Length	Slope	e Velocity	Capacity	Description			
(min)	(feet)	(ft/ft	) (ft/sec)	(cfs)				
5.0					Direct Entry, 5 minute min			
					-			

# Summary for Subcatchment 2S: North

Runoff = 1.17 cfs @ 12.15 hrs, Volume= 0.101 af, Depth= 1.60"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10-Year Rainfall=4.70"

	A	rea (sf)	CN I	Description			
		17,469	39 :	>75% Gras	s cover, Go	ood, HSG A	
*		15,565	98 I	mpervious			
		33,034	67	Neighted A	verage		
	17,469 52.88% Pervious Area						
	15,565 47.12% Impervious Ar					ea	
	_						
	TC	Length	Slope	Velocity	Capacity	Description	
	(min)	(feet)	(ft/ft)	(ft/sec)	(cts)		
	9.3	100	0.0250	0.18		Sheet Flow, A-B	
						Grass: Short n= 0.150 P2= 3.00"	
	0.9	206	0.0339	3.74		Shallow Concentrated Flow, B-C	
						Paved Kv= 20.3 fps	
	10.2	306	Total				

### Summary for Subcatchment 3S: South

Runoff = 0.24 cfs @ 12.07 hrs, Volume= 0.016 af, Depth= 2.81"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 10-Year Rainfall=4.70"

Type III 24-hr 10-Year Rainfall=4.70" Printed 1/20/2016 LC Page 11

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	Are	ea (sf)	CN	Description					
		813	39	>75% Gras	s cover, Go	ood, HSG A			
*		2,189	98	Impervious					
		3,002 813 2,189	82	2 Weighted Average 27.08% Pervious Area 72.92% Impervious Area					
(m	Tc nin)	Length (feet)	Slope (ft/ft	· Velocity ) (ft/sec)	Capacity (cfs)	Description			
Į	5.0					Direct Entry,	, 5 minute m	nin	
	Summary for Reach 1R:								
Inflov Inflov	w Are w	ea = =	0.109 0.34 c	) ac, 72.839 sfs @12.1	% Impervio 0 hrs, Volu	us, Inflow Dep ıme=	oth = 1.95" ).018 af	for 10-Year	revent

Outflow = 0.34 cfs (0.12, 11 hrs), Volume= 0.018 af, Atten= 0%, Lag= 0.4 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Max. Velocity= 5.19 fps, Min. Travel Time= 0.2 min Avg. Velocity = 1.80 fps, Avg. Travel Time= 0.6 min

Peak Storage= 5 cf @ 12.11 hrs Average Depth at Peak Storage= 0.06' Bank-Full Depth= 1.00' Flow Area= 2.0 sf, Capacity= 45.86 cfs

1.00' x 1.00' deep channel, n= 0.013 Asphalt, smooth Side Slope Z-value= 1.0 '/' Top Width= 3.00' Length= 69.0' Slope= 0.0957 '/' Inlet Invert= 101.60', Outlet Invert= 95.00'



#### Summary for Pond CB-1:

Inflow A	rea =	0.178 ac, 72.86% Impervious, Inflow	/ Depth = 2.29"	for 10-Year event
Inflow	=	0.56 cfs @ 12.09 hrs, Volume=	0.034 af	
Primary		0.56 cfs @ 12.09 hrs, Volume=	0.034 af, Atte	en= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

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# Summary for Pond CB-2:

Inflow A	Area =	0.758 ac, 4	7.12% Impervious,	Inflow Depth = 1.	60" for 10-Year event
Inflow	=	1.17 cfs @	12.15 hrs, Volume	e= 0.101 af	
Primary	y =	1.17 cfs @	12.15 hrs, Volume	e 0.101 af,	Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

# **Summary for Pond P1: Infiltration Basin**

Inflow Area	a =	0.109 ac, 7	2.83% Imp	ervious, Inflow De	epth = 2.8	1" for 10-`	Year event
Inflow	=	0.37 cfs @	12.07 hrs,	Volume=	0.025 af		
Outflow	=	0.35 cfs @	12.10 hrs,	Volume=	0.022 af,	Atten= 7%,	Lag= 1.8 min
Discarded	=	0.00 cfs @	12.10 hrs,	Volume=	0.004 af		-
Primary	=	0.34 cfs @	12.10 hrs,	Volume=	0.018 af		

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 101.76' @ 12.10 hrs Surf.Area= 363 sf Storage= 246 cf

Plug-Flow detention time= 132.3 min calculated for 0.022 af (87% of inflow) Center-of-Mass det. time= 72.8 min ( 890.8 - 817.9 )

Volume	Inver	t Avail.Stor	rage Storage	Description	
#1	100.00	)' 34	15 cf Custom	Stage Data (Pr	<b>ismatic)</b> Listed below (Recalc)
Elevatio (fee	on S et)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
100.0	00	5	0	0	
101.0	00	121	63	63	
101.3	30	219	51	114	
102.0	00	440	231	345	
Device	Routing	Invert	Outlet Device	S	
#1 #2	Discarded Primary	101.30' 101.60'	1.420 in/hr Ex Conductivity t Excluded Sur 2.0' long x 0 Head (feet) 0 Coef (English	<b>xfiltration over</b> o Groundwater I face area = 219 <b>.5' breadth Bro</b> .20 0.40 0.60	Surface area above 101.30' Elevation = 0.00' sf ad-Crested Rectangular Weir 0.80 1.00 08 3 30 3 32
				1) 2.00 2.02 0.	00 0.00 0.02

**Discarded OutFlow** Max=0.00 cfs @ 12.10 hrs HW=101.75' (Free Discharge) **1=Exfiltration** (Controls 0.00 cfs)

Primary OutFlow Max=0.34 cfs @ 12.10 hrs HW=101.75' (Free Discharge) -2=Broad-Crested Rectangular Weir (Weir Controls 0.34 cfs @ 1.10 fps)

PROPOSED Klenicki- 5-9 Romas	<b>:o Lane</b>	Type III 24-hr	25-Year Rainfall=5.50"
Prepared by Sevee and Maher Engine	ers, Inc.		Printed 1/20/2016
HydroCAD® 10.00 s/n 01260 © 2012 Hydro	CAD Software Solutions I		Page 13
Time span=0.	)0-30.00 hrs, dt=0.01 hr	rs, 3001 points	nd method
Runoff b	y SCS TR-20 method, l	UH=SCS	
Reach routing by Stor-Ind+	Trans method - Pond	routing by Stor-I	
Subcatchment1S: East	Runoff Area=4,737	sf 72.83% Imper	rvious Runoff Depth=3.53"
	To	c=5.0 min CN=82	Runoff=0.46 cfs 0.032 af
Subcatchment 2S: North	Runoff Area=33,034	sf 47.12% Imper	rvious Runoff Depth=2.16"
	Flow Length=306' Tc=	=10.2 min CN=67	′ Runoff=1.62 cfs 0.136 af
Subcatchment3S: South	Runoff Area=3,002	sf 72.92% Imper	rvious Runoff Depth=3.53"
	Tc	c=5.0 min CN=82	Runoff=0.29 cfs 0.020 af
<b>Reach 1R:</b> n=0.013	Avg. Flow Depth=0.07'	Max Vel=5.64 fp	s Inflow=0.43 cfs 0.024 af
	L=69.0' S=0.0957 '/' Ca	apacity=45.86 cfs	Outflow=0.43 cfs 0.024 af
Pond CB-1:			Inflow=0.71 cfs 0.044 af Primary=0.71 cfs 0.044 af
Pond CB-2:			Inflow=1.62 cfs 0.136 af Primary=1.62 cfs 0.136 af
Pond P1: Infiltration Basin	Peak Elev=101.7	78' Storage=256 c	f Inflow=0.46 cfs 0.032 af
Discarded=0.0	1 cfs_0.005 af_Primary=	0.43 cfs  0.024 af	Outflow=0.44 cfs 0.029 af
Total Runoff Area = 0.93	6 ac Runoff Volume =	= 0.189 af Avei	rage Runoff Depth = 2.42"
	47.99% Pervious = (	0.449 ac 52.0 <sup>7</sup>	1% Impervious = 0.487 ac

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### Summary for Subcatchment 1S: East

Runoff = 0.46 cfs @ 12.07 hrs, Volume= 0.032 af, Depth= 3.53"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 25-Year Rainfall=5.50"

A	rea (sf)	CN	Description				
	2,518	98	Unconnecte	ed roofs, HS	SG C		
	1,287	39	∋ >75% Grass cover, Good, HSG A				
	932	98	Paved park	ing, HSG A	N		
	4,737	82	Weighted A	verage			
	1,287 27.17% Pervious Area						
	3,450 72.83% Impervious Are			pervious Are	ea		
	2,518		72.99% Un	connected			
Tc (min)	Length (feet)	Slope (ft/ft	e Velocity ) (ft/sec)	Capacity (cfs)	Description		
5.0					Direct Entry, 5 minute min		

# Summary for Subcatchment 2S: North

Runoff = 1.62 cfs @ 12.15 hrs, Volume= 0.136 af, Depth= 2.16"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 25-Year Rainfall=5.50"

	A	rea (sf)	CN I	Description			
		17,469	39 :	>75% Gras	s cover, Go	ood, HSG A	
*		15,565	98 I	mpervious			
		33,034	67	Neighted A	verage		
	17,469 52.88% Pervious Area						
	15,565 47.12% Impervious Ar					ea	
	_						
	TC	Length	Slope	Velocity	Capacity	Description	
	(min)	(feet)	(ft/ft)	(ft/sec)	(cts)		
	9.3	100	0.0250	0.18		Sheet Flow, A-B	
						Grass: Short n= 0.150 P2= 3.00"	
	0.9	206	0.0339	3.74		Shallow Concentrated Flow, B-C	
						Paved Kv= 20.3 fps	
	10.2	306	Total				

### Summary for Subcatchment 3S: South

Runoff = 0.29 cfs @ 12.07 hrs, Volume= 0.020 af, Depth= 3.53"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Type III 24-hr 25-Year Rainfall=5.50"

 Type III 24-hr
 25-Year Rainfall=5.50"

 Printed
 1/20/2016

 LC
 Page 15

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A	rea (sf)	CN E	escription						
	813	39 >	75% Gras	s cover, Go	ood, HSG A				
*	2,189	<u>98 lı</u>	mpervious						
	3,002	82 V	Veighted A	verage					
	813	2	7.08% Per	vious Area					
	2,189	7	2.92% Imp	ervious Ar	ea				
Та	Lanath	Clana	Valasity	Consister	Description				
IC (min)	(foot)			Capacity	Description				
	(leet)	(1711)	(II/Sec)	(CIS)	Dine of Easter	· Funita da unita			
5.0	5.0 Direct Entry, 5 minute min								
	Current and fair Datash 4D								
				Summa	ry for Reac	n 1 <b>k</b> :			
Inflow Ar	ea = =	0.109 0.43 cf	ac, 72.83%	6 Impervio	us, Inflow De	pth = 2.66"  for 25-Year event 0.024 af			
Outflow	=	0.40 cf	s @ 12.1	1 hrs, Volu	ime=	0.024 af, Atten= 0%, Lag= 0.3 min			
			U	,		, , , <b>, , , , , , , , , , , , , , , , </b>			
Routing Max. Vel Avg. Vel	Routing by Stor-Ind+Trans method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Max. Velocity= 5.64 fps,  Min. Travel Time= 0.2 min Avg. Velocity = 1.88 fps.  Avg. Travel Time= 0.6 min								
-	-	•	-						

Peak Storage= 5 cf @ 12.10 hrs Average Depth at Peak Storage= 0.07' Bank-Full Depth= 1.00' Flow Area= 2.0 sf, Capacity= 45.86 cfs

1.00' x 1.00' deep channel, n= 0.013 Asphalt, smooth Side Slope Z-value= 1.0 '/' Top Width= 3.00' Length= 69.0' Slope= 0.0957 '/' Inlet Invert= 101.60', Outlet Invert= 95.00'



# Summary for Pond CB-1:

Inflow Ar	rea =	0.178 ac, 72.86% Impervious, Inflov	v Depth = 3.00"	for 25-Year event
Inflow	=	0.71 cfs @ 12.09 hrs, Volume=	0.044 af	
Primary	=	0.71 cfs @ 12.09 hrs, Volume=	0.044 af, Atte	en= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

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# Summary for Pond CB-2:

Inflow Ar	ea =	0.758 ac, 47.12% Impervious, Inflov	v Depth = 2.16" for 25-Year ever	nt
Inflow	=	1.62 cfs @ 12.15 hrs, Volume=	0.136 af	
Primary	=	1.62 cfs @   12.15 hrs,  Volume=	0.136 af, Atten= 0%, Lag= 0.0	min

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs

# **Summary for Pond P1: Infiltration Basin**

Inflow Area	a =	0.109 ac, 7	2.83% Imp	ervious, Inflo	w Depth =	3.53" for	25-Year event
Inflow	=	0.46 cfs @	12.07 hrs,	Volume=	0.032	af	
Outflow	=	0.44 cfs @	12.10 hrs,	Volume=	0.029 a	af, Atten=	6%, Lag= 1.7 min
Discarded	=	0.01 cfs @	12.10 hrs,	Volume=	0.005	af	-
Primary	=	0.43 cfs @	12.10 hrs,	Volume=	0.024 a	af	

Routing by Stor-Ind method, Time Span= 0.00-30.00 hrs, dt= 0.01 hrs Peak Elev= 101.78' @ 12.10 hrs Surf.Area= 371 sf Storage= 256 cf

Plug-Flow detention time= 111.0 min calculated for 0.029 af (89% of inflow) Center-of-Mass det. time= 60.4 min (871.9 - 811.5)

Volume	Inve	rt Avail.Sto	rage Sto	rage Description	
#1	100.00	)' 34	45 cf <b>Cu</b>	stom Stage Data (F	Prismatic)Listed below (Recalc)
Elevatio (fee	on S et)	Surf.Area (sq-ft)	Inc.Sto (cubic-fee	re Cum.Store et) (cubic-feet)	
100.0	00	5		0 0	
101.0	00	121	6	63 63	
101.3	30	219	Ę	51 114	
102.0	00	440	23	31 345	
Device	Routing	Invert	Outlet D	evices	
#1 #2	Discardeo Primary	101.30' 101.60'	1.420 in/ Conduct Excluded 2.0' long Head (fe Coef. (E	<pre>/hr Exfiltration over ivity to Groundwater d Surface area = 219 f x 0.5' breadth Bro et) 0.20 0.40 0.60 nglish) 2.80 2.92 3</pre>	r Surface area above 101.30' Elevation = 0.00' 9 sf Dad-Crested Rectangular Weir 0.80 1.00 3.08 3.30 3.32

**Discarded OutFlow** Max=0.01 cfs @ 12.10 hrs HW=101.78' (Free Discharge) **1=Exfiltration** (Controls 0.01 cfs)

Primary OutFlow Max=0.43 cfs @ 12.10 hrs HW=101.78' (Free Discharge) -2=Broad-Crested Rectangular Weir (Weir Controls 0.43 cfs @ 1.19 fps)

# **APPENDIX G**

# POST CONSTRUCTION STORMWATER MANAGEMENT PLAN

# POST-CONSTRUCTION STORMWATER MANAGEMENT PLAN

**Prepared for** 

# LEGACY 18 DEVELOPMENT INC. 4-UNIT CONDO BUILDING

5 & 9 ROMASCO LANE Portland, Maine

# January 2016







# TABLE OF CONTENTS

Section No.	Title	Page No.
1.0 SITE DESCRIPTION		1
2.0 CONTACTS		1
3.0 POST CONSTRUCTION S OBJECTIVES	STORMWATER MANAGEMENT PLA	N OVERVIEW AND
<ul> <li>3.1 Site Management  </li> <li>3.2 Inspections</li> <li>3.3 Routine Maintenan</li> <li>3.4 Annual Report</li> </ul>	Practices	

### POST-CONSTRUCTION STORMWATER MANAGEMENT PLAN LEGACY 18 DEVELOPMENT INC. 4-UNIT CONDO BUILDING

### **1.0 SITE DESCRIPTION**

David Klenicki (Owner) proposes to construct a 1,955-square-foot condo building on a 3,500 square foot parcel located at 5&9 Romasco Lane in Portland, Maine. The building will be a four-story, four-unit condo building with dedicated on-site parking in the garage located on the first floor. The property is located within the Residential 6 (R6) Zoning District and will comply with the City's vision of multifamily dwellings at a high density within this area.

The Owner intends to construct a four-story property to include; a garage on the first floor, two one-bedroom units on the second floor and two two-bedroom units on the other two floors.

The property is currently fully disturbed with an 880-square-foot paved parking area along the lane frontage and an overgrown lawn area to the rear of the property. There was a 650 square-foot building removed in July of 2015. The proposed building, paved vehicle maneuvering area, walks and patios will not result in an increase in developed area on the property, but will increase the on-site impervious area to approximately 2,100 square feet, an increase of 600 square feet.

### 2.0 CONTACTS

Facility:	4-Unit Condo Building 5 & 9 Romasco Lane Portland, Maine 04101
Owner Representative:	David Klenicki, President Legacy 18 Development Inc. 233 Smith Road Windham, ME 04062 Telephone: (917) 608-8814

Consultant/Designer:

Sevee & Maher Engineers 4 Blanchard Road Cumberland, Maine 04021 Telephone: 207-829-5016 Daniel P. Diffin, P.E. dpd@smemaine.com

# 3.0 POST CONSTRUCTION STORMWATER MANAGEMENT PLAN OVERVIEW AND OBJECTIVES

The Post Construction Stormwater Management Plan (PCSWMP) is an important component of the overall stormwater management system for the site. PCSWMP addresses various maintenance activities that should occur <u>after construction</u> and site stabilization. Proper implementation of the SWP can minimize pollutant generation and transport and maintain the stormwater treatment system to ensure proper operation. This PCSWMP includes three primary components:

- 1. Site Management Practices
- 2. Inspections
- 3. Routine Maintenance and Corrective Actions

# 3.1 Site Management Practices

Site management practices are aimed at reducing pollutants by minimizing use of certain materials, using alternative materials, or removing pollutants prior to discharge to the stormwater treatment system. These practices shall include:

- a. Use slow release sulfur or plastic coated ureaform fertilizers (e.g., Nutralene).
- b. Do not fertilize vegetated swales once vegetation is established.
- c. Minimize use of pesticides by using a sound integrated pest management (IPM) approach to monitor and control the actual pests present.
- d. Collect and remove autumn leaves to minimize transport to the stormwater treatment system.
- e. Minimize use of de-icing materials and sand.

- f. Routine sweeping of parking areas and driveways.
- g. Fertilizers, pesticides and other hazardous materials should be stored in enclosed areas to avoid exposure to precipitation.
- h. Material handling should be conducted to minimize risk of spillage and release to the stormwater treatment system.

### 3.2 Inspections

A series of routine inspections by the Owner or their agent shall be completed to allow for the early identification of potential problems, and to guide routine maintenance activities. Inspections shall be carried out in accordance with the Site Inspection Schedule (Table 1). Dates and observations shall be recorded for each inspection on the attached 'Inspection Log'.

In addition to the routine inspections, an inspection by a qualified post-construction stormwater inspector to inspect the BMPs is required on a minimum annual basis.

### 3.3 Routine Maintenance and Corrective Actions

Routine maintenance activities are designed to ensure proper function of the stormwater management system and minimize pollutant transport from the site. Routine maintenance activities must be completed according to the schedule (Table 1) provided in this plan. This schedule is the <u>minimum</u> amount of maintenance required, and maintenance that is more frequent may be needed when indicated by the inspections. Corrective actions (supplemental maintenance activities or repairs) should be completed as soon as possible, but no more than 7 days, after the inspection identifying the problem. Each maintenance activity will be recorded on the attached 'Maintenance and Repair Log'. Records of the deficiencies and corrective actions shall be included in the annual report.

During construction, the Sitework Contractor shall be responsible for cleaning and maintaining stormwater components on the schedule outlined in Table 1.

Following completion of construction, the Owner will be responsible for cleaning and maintaining stormwater components on the schedule outlined in Table 1.

Place removed sediments in an area of low erosion potential, either on-site or off-site, and seed with erosion control seed mix.

The following describes specific stormwater facilities maintenance requirements and minimum schedule of inspection and maintenance.

- 1. Open swales and ditches need to be inspected in the spring and fall, or after a major rainfall event, to assure that debris or sediments do not reduce the effectiveness of the system. Debris needs to be removed at that time. Sign of erosion or blockage shall be immediately repaired to assure a vigorous growth of vegetation for the stability of the structure and proper functioning. Swales that show newly formed channels or gullies will be immediately repaired by reseeding/sodding of bare spots, removal of trash, leaves and/or accumulated sediments, and the control of woody or other undesirable vegetation.
- Vegetated ditches should be mowed at least once during the growing season.
   Larger brush or trees must not be allowed to become established in the channel.
   Any areas where the vegetation fails will be subject to erosion and should be repaired and revegetated.
- 3. If sediment in culverts or piped drainage systems exceeds 20 percent of the diameter of the pipe, it should be removed. This may be accomplished by hydraulic flushing or other mechanical means; however, care should be taken to not flush the sediments into the infiltration basin as it will reduce the pond's capacity and hasten the time when it must be cleaned. Storm pipes should be inspected on an annual basis.

- 4. Paved surfaces shall be swept or vacuumed at least annually in the spring to remove winter sand and periodically during the year on an as-needed basis to minimize the transportation of sediment during rainfall events.
- 5. Sediments within the infiltration basin shall be removed and the basin bottom repaired. Any areas around the infiltration basin found to have erosion should be corrected as necessary. Any bare areas should be seeded or sodded, as necessary. Inspect the area around the basin semi-annually for eroding soil and other sediment sources. Repair eroding areas using appropriate erosion control BMPs immediately. Control sediment sources, such as stockpiles of winter sand, by removing them from the basin's drainage area or surrounding them with sediment control BMP's. Prohibit vehicle access to all filtration areas, and limit pedestrian access into the basin. Heavy equipment used to maintain or rehabilitate the basins should work from the basin's perimeter.

# 3.4 Annual Report

The Owner or a qualified post-construction stormwater inspector shall provide a completed and signed certification to the department of public services (DPS) in a form provided by DPS certifying that the person has inspected the BMPs and that they are adequately maintained and functioning as required by this Plan, or that they require maintenance or repair, including the record of the deficiencies and corrective actions taken. The Owner will be required to pay a filing fee established by the DPS.

#### TABLE 1

#### LEGACY 18 DEVELOPMENT INC. 4-UNIT CONDO BUILDING

	Spring	Fall or Yearly	After a Major Storm	Every 2-5 Years
Vegetated Areas				
Inspect all slopes and embankments	Х	Х	Х	
Replant bare areas or areas with sparse growth	Х	Х	Х	
Armor areas with rill erosion with an appropriate lining or divert the erosive flows to on-site areas able to withstand concentrated flows.	х	х	х	
Driveways and Parking Surfaces				
Clear accumulated winter sand in parking lots and along roadways	Х			
Sweep pavement to remove sediment	Х			
Infiltration Basins				
Inspect soil filter to see that collected water drains within 72 hours.	Х	Х	Х	
Rototill top 3" soil, or remove and replace the top 3" of soil with clean soil to the proper specification, when the bed fails to drain dry within 72 hours.				х
Remove accumulated sediment, dead portions of plants, excessive growth, and weeds.		х		
Mow grass-covered filter bed no shorter than 6", at a frequency of no more than 2 times per growing season to maintain a high-grass meadow. Do not fertilize unless absolutely needed.	x	х		

The maintenance needs for most vegetative and stabilization measures may be found in the Maine Erosion and Sediment Control BMPs manual as published in 2003 (or latest version) and/or the Maine Stormwater Best Management Practices Manual.

#### LEGACY 18 DEVELOPMENT INC.

#### **4-UNIT CONDO BUILDING INSPECTION LOG**

Date	Device/Area Inspected	Inspected By	Observations, Deficiencies & Recommended Corrective Actions

#### LEGACY 18 DEVELOPMENT INC.

#### 4-UNIT CONDO BUILDING MAINTENANCE LOG

Date	Device or Area Maintained/ Repaired	Maintenance and/or Repair Completed By	Maintenance Completed/Corrective Actions Taken

# **APPENDIX H**

# **CITY WASTEWATER CAPACITY APPLICATION**

# **CITY OF PORTLAND WASTEWATER CAPACITY APPLICATION**

Department of Public Services, 55 Portland Street, Portland, Maine 04101-2991



Mr. Frank J. Brancely, Senior Engineering Technician, Phone #: (207) 874-8832, Fax #: (207) 874-8852, E-mail:fjb@portlandmaine.gov

Date: 10/21/2015

#### 1. Please, Submit Utility, Site, and Locus Plans.

Site Address:	5&9 Romasco Lane, Portland, ME			
		C	hart Block Lot Number: G10NE/16D/	10
Proposed Use:	Multi-Family Housing			
Previous Use:	Parking	2	Commercial (see part 4 below)	
Existing Sanitary I	Flows: 0GPD	080	Industrial (complete part 5 below)	
Existing Process F	lows: 0GPD	ate	Governmental	
Description and lo	ocation of City sewer that is to	e O	Residential	X
receive the propo	osed building sewer lateral.	Sit	Other <i>(specify)</i>	
18" line located in	a Saint Lawrence St. south of the lot.			

#### (Clearly, indicate the proposed connections, on the submitted plans)

#### 2. Please, Submit Contact Information.

City Planner's Name:	Ph	one:			
Owner/Developer Name:	Legacy 18 Devel	Legacy 18 Development Inc.			
Owner/Developer Address:	233 Smith Road	233 Smith Road, Windham, Me 04062			
Phone: 917-608-8814	Fax:	Fax: E-mail: kdsinc7@gmail.com			
Engineering Consultant Name:	Daniel P. D	Daniel P. Diffin, P.E.			
Engineering Consultant Address:	4 Blanchard	4 Blanchard Rd, Cumberland, ME 04021			
Phone: 207-829-5016	Fax:	E-mail:	dpd@smemaine.com		
(Note: Consultants	and Developers shoul prior to Planning	ld allow +/- 15 days, fo Board Review)	or capacity status,		

#### 3. Please, Submit Domestic Wastewater Design Flow Calculations.

Estimated Domestic Wastewater Flow Generated:	
Peaking Factor/ Peak Times:6	
Specify the source of design guidelines: (i.e"Handboo	ok of Subsurface Wastewater Disposal in Maine,
"Plumbers and Pipe Fitters Calculation Manual," P	Portland Water District Records, $\underline{ imes}$ Other (specify
TR-16	

# (Note: Please submit calculations showing the derivation of your design flows, either on the following page, in the space provided, or attached, as a separate sheet)

4. Please, Submit External Grease Interceptor Calculations.	
Total Drainage Fixture Unit (DFU) Values:	
Size of External Grease Interceptor:	
Retention Time:	
Peaking Factor/ Peak Times:	

(Note: In determining your restaurant process water flows, and the size of your external grease interceptor, please use The Uniform Plumbing Code. Note: In determining the retention time, sixty (60) minutes is the minimum retention time. Note: Please submit detailed calculations showing the derivation of your restaurant process water design flows, and please submit detailed calculations showing the derivation of the size of your external grease interceptor, either in the space provided below, or attached, as a separate sheet)

#### **5. Please, Submit Industrial Process Wastewater Flow Calculations** Estimated Industrial Process Wastewater Flows Generated:

Do you currently hold Federal or State discharge permits? Is the process wastewater termed categorical under CFR 40? OSHA Standard Industrial Code (SIC): Peaking Factor/Peak Process Times:

0	GPD	
Yes	No	×
Yes	No	X
http://www.osha.gov/c	shstats/sics	ser.html

(Note: On the submitted plans, please show where the building's domestic sanitary sewer laterals, as well as the building's industrialcommercial process wastewater sewer laterals exits the facility. Also, show where these building sewer laterals enter the city's sewer. Finally, show the location of the wet wells, control manholes, or other access points; and, the locations of filters, strainers, or grease traps)

> (Note: Please submit detailed calculations showing the derivation of your design flows, either in the space provided below, or attached, as a separate sheet)

Notes, Comments or Calculation

Number of Bedrooms: 6 Daily Water Demand: 6 x 90 gallons/ day= 540 gallons/day Peak Flow: 540 gallons/day x 6 = 3,240 gallons/ day 2.25 gal/min

# **APPENDIX I**

# PORTLAND WATER DISTRICT CAPACITY TO SERVE LETTER



# **Portland Water District**

FROM SEBAGO LAKE TO CASCO BAY

November 19, 2015

Sevee & Maher Engineers, Inc. 4 Blanchard Road, P.O. Box 85A Cumberland, ME 04021

Attn:Daniel DiffinRe:5 and 9 Romasco Lane - Portland<br/>Ability to Serve with PWD Water

Dear Mr. Diffin:

The Portland Water District has received your request for an Ability to Serve Determination for the noted site submitted on October 22, 2015. Based on the information provided, we can confirm that the District will be able to serve the proposed project as further described in this letter.

#### Conditions of Service

The following conditions of service apply:

- It is the District's understanding that a four unit residential building with a NFPA 13R life safety sprinkler system is proposed at this location. The each of the two parcels are currently served with a <sup>3</sup>/<sub>4</sub>-inch domestic water service; the size of these services is undersized for the proposed use. Each service must be terminated by shutting the corporation valve and cutting the pipe from the water main.
- New fire and domestic services may be installed from the water main in Romasco Lane. The services should enter through the properties frontage at least 10-feet from side property lines. Please note that only one meter and one bill will be associated to each domestic service line. This one master meter must be located in a common space that all tenants could gain access to if necessary.
- Water District approval of water infrastructure plans will be required for the project prior to construction. As your project progresses, we advise that you submit any preliminary design plans to MEANS for review of the water main and water service line configuration. We will work with you to ensure that the design meets our current standards.

#### Existing Site Service

According to District records, the project site does currently have existing water service. Two 3/4-inch diameter copper water service lines, located as shown on the attached water service

PO - 5 & 9 Romasco Lane - Ability to Serve Determination - 2015

 $( \mathfrak{B} )$ 

cards, provide water service to this site. Please refer to the "Conditions of Service" section of this letter for requirements related to the use of these services.

#### Water System Characteristics

According to District records, there is an 8-inch diameter cast iron water main on the west side of Romasco Lane and a public fire hydrant located 75 feet from the site. Recent flow data is not available in this area. The most recent static pressure reading was 72 psi on 8/14/2015.

#### Public Fire Protection

This project will most likely not include the installation of new public hydrants to be accepted into the District water system. It is your responsibility to contact the Portland Fire Department to ensure that this project is adequately served by existing and/or proposed hydrants.

#### Domestic Water Needs

The data noted above indicates there should be adequate pressure and volume of water to serve the domestic water needs of your proposed project.

#### Private Fire Protection Water Needs

It is anticipated that this project will require water service to provide private fire protection to the site. Please note that the District does not guarantee any quantity of water or pressure through a fire protection service. Please share these results with your sprinkler system designer so that they can design the fire protection system to best fit the noted conditions. If the data is out of date or insufficient for their needs, please contact MEANS to request a hydrant flow test and we will work with you to get more complete data.

If the District can be of further assistance in this matter, please let us know.

Sincerely. Portland Water District Navu

Glissen Havu, E.I. Design Engineer



# **APPENDIX J**

**CONSTRUCTION MANAGEMENT PLAN** 

### CONSTRUCTION MANAGEMENT PLAN LEGACY 18 DEVELOPMENT INC. 4-UNIT CONDO BUILDING

### **BUILDING CONSRUCTION AND SITE DEVELOPMENT**

Development of the 4-unit condo building at 5 & 9 Romasco Lane (Project) will begin with the installation of erosion control measures including the stabilized construction entrance and sediment barriers. Erosion control measures will be inspected and maintained throughout the project. Materials required for construction will generally be stored off-site, however if necessary, on-site storage will be on the northeast end of the property, away from Romasco Lane.

Construction will begin with the demolition of the existing paved parking area and site grading. Waste materials from the demolition will be disposed of by the contractor. Pouring of the foundation and building construction will follow the demolition of the parking area. The utilities will then be connected to the building from the existing infrastructure.

Near the end of building construction, exterior site improvements will be completed. These site improvements include grading, installing walkways, constructing the infiltration basin, paving the maneuvering area and landscaping. Restoration of Romasco Lane pavement and sidewalk will be completed near the end of building construction to avoid damage from construction equipment on new pavement, curbing and sidewalk pavers.

### TRAFFIC CONTROL

Certain components of the Project will affect traffic on Romasco Lane, primarily during utility tieins. The owner and contractor will coordinate road closures with the City of Portland. The construction of this Project is not anticipated to significantly increase traffic volumes of Romasco Lane. Pedestrian traffic will be directed to use the sidewalk on the opposite side of Romasco Lane to avoid the immediate area of the Project with appropriate signage and barriers.

