GENERAL NOTES

- THE NOTES ON THESE DRAWINGS ARE NOT INTENDED TO REPLACE SPECIFICATIONS. SEE SPECIFICATIONS FOR REQUIREMENTS IN ADDITION TO GENERAL NOTES. INCONSISTENCIES BETWEEN THESE DRAWINGS AND THE SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT PRIOR TO PROCEEDING WITH THE AFFECTED PORTION OF THE WORK.
- STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH JOB SPECIFICATIONS AND 2. ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND SITE DRAWINGS, CONSULT THESE DRAWINGS FOR LOCATIONS AND DIMENSIONS OF OPENINGS, CHASES, INSERTS, REGLETS, SLEEVES, DEPRESSIONS, AND OTHER DETAILS NOT SHOWN ON STRUCTURAL DRAWINGS
- ALL DIMENSIONS, EXISTING CONDITIONS, AND AS-BUILT CONDITIONS MUST BE VERIFIED IN THE FIELD. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER BEFORE PROCEEDING WITH THE AFFECTED PART OF THE WORK.
- THE STRUCTURE IS DESIGNED TO BE SELF SUPPORTING AND STABLE ONLY AFTER THE STRUCTURAL WORK CONTAINED IN THE S- DRAWINGS IS COMPLETED. IT IS THE CONTRACTOR'S SOLE RESPONSIBILITY TO DETERMINE ERECTION PROCEDURES AND SEQUENCE TO ENSURE THE SAFETY OF THE BUILDING AND ITS COMPONENTS DURING ERECTION. THIS INCLUDES THE ADDITION OF NECESSARY SHORING. SHEETING. TEMPORARY BRACING, GUYS OR TIEDOWNS. SUCH MATERIAL SHALL REMAIN THE PROPERTY OF THE CONTRACTOR AFTER COMPLETION OF THE PROJECT.
- SECTIONS AND DETAILS SHOWN ON ANY STRUCTURAL DRAWINGS SHALL BE CONSIDERED TYPICAL FOR SIMILAR CONDITIONS AS DETERMINED BY THE STRUCTURAL ENGINEER. THE STRUCTURAL ENGINEER RESERVES THE RIGHT TO INTERPRET DETAILS TO ADDRESS OTHER PROJECT CONDITIONS.
- PROVIDE AND INSTALL NECESSARY MATERIAL TO CONNECT ELEVATOR SUPPORT BEAMS AND GUIDE RAILS. LOCATION AND SIZE OF MEMBERS AND ANY INSERTS REQUIRED SHALL BE DETERMINED BY THE ELEVATOR MANUFACTURER.
- THE CONTRACTOR SHALL SUBMIT COMPLETE SHOP DRAWINGS FOR ALL PARTS OF THE WORK, INCLUDING DESCRIPTION OF SHORING, AND CONSTRUCTION METHODS AND SEQUENCING WHERE APPLICABLE. NO PERFORMANCE OF THE WORK INCLUDING, BUT NOT LIMITED TO, DEMOLITION OF EXISTING STRUCTURE, OR FABRICATION OR ERECTION OF NEW STRUCTURAL ELEMENTS, SHALL COMMENCE WITHOUT REVIEW OF THE SHOP DRAWINGS BY THE ARCHITECT AND ENGINEER. FOR SHOP DRAWINGS AND SUBMITTALS REQUIRED, REFERENCE THE PROJECT SPECIFICATION.
- ALL APPLICABLE FEDERAL, STATE, AND MUNICIPAL REGULATIONS SHALL BE FOLLOWED, INCLUDING THE FEDERAL DEPARTMENT OF LABOR OCCUPATIONAL SAFETY AND HEALTH ACT.
- IN ACCORDANCE WITH THE MAINE UNIFORM BUILDING AND ENERGY CODE/INTERNATIONAL BUILDING CODE (2009 EDITION, SECTION 1704.1), A STATEMENT OF SPECIAL INSPECTIONS IS REQUIRED AS A CONDITION FOR PERMIT ISSUANCE BY THE LOCAL CODE OFFICIAL. THIS STATEMENT SHALL INCLUDE A COMPLETE LIST OF MATERIALS AND WORK REQUIRING SPECIAL INSPECTIONS, THE INSPECTIONS TO BE PERFORMED AND A LIST OF THE INDIVIDUALS, APPROVED AGENCIES AND FIRMS INTENDED TO BE RETAINED FOR CONDUCTING SUCH INSPECTIONS.

10. REFERENCE THE PROJECT SPECIFICATIONS FOR ALL TESTING REQUIREMENTS.

<u>DESIGN LOADS</u>

| 1. | BUILDING CODE: MAINE UNIFORM BUILDING AND ENERGY CODE INTERNATIONAL BUILDING CODE, 2009 EDITION INTERNATIONAL EXISTING BUILDING CODE, 2009 EDITION ASCE 7-05 MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES | | | |
|----|--|---|--|--|
| 2. | DESIGN FLOOR LIVE LOADS: PRIVATE ROOMS AND CORRIDORS SERVING THEM: PUBLIC ROOMS AND CORRIDORS SERVING THEM: DINING ROOMS AND RESTAURANTS: STAIRS AND EXITWAYS: | 40 PSF 100 PSF 100 PSF 100 PSF | | |
| 3. | DESIGN ROOF SNOW LOAD: GROUND SNOW LOAD (Pg): SNOW EXPOSURE FACTOR (Ce): SNOW LOAD IMPORTANCE FACTOR (Is): SNOW LOAD THERMAL FACTOR (Ct): FLAT ROOF SNOW LOAD (Pf): | 60 PSF 1.0 1.0 1.1 46 PSF + DRIFT | | |
| 4. | DESIGN WIND LOAD: BASIC WIND SPEED: WIND LOAD IMPORTANCE FACTOR (Iw): WIND EXPOSURE: INTERNAL PRESSURE COEFFICIENT: COMPONENTS & CLADDING PER ASCE 7-05 | 100 MPH 1.0 C ±0.18 | | |
| 5. | DESIGN SEISMIC LOADS: EQUIVALENT LATERAL FORCE PROCEDURE SEISMIC OCCUPANCY CATEGORY: SEISMIC IMPORTANCE FACTOR (Ie): MAPPED SPECTRAL RESPONSE ACCELERATIONS: Ss: S1: SEISMIC SITE CLASS: SPECTRAL RESPONSE COEFFICIENTS: Sds: Sd1: SEISMIC DESIGN CATEGORY: BASIC STRUCTURAL SYSTEM: BUILDING FRAME SYSTEM BASIC SEISMIC FORCE RESISTING SYSTEM: STEEL SYSTEMS NOT SPECIFICALLY DETAILED FOR S RESPONSE MODIFICATION FACTOR (R): | II 1.0 0.314 0.077 D 0.324 0.123 B SEISMIC RESISTANCE X: 3.0 Y: 3.0 Y: 0.000 | | |
| | SEISMIC RESPONSE COEFFICIENT (Cs): | X: 0.06 Y: 0.06 | | |

FOUNDATION NOTES (SOIL SUPPORTED)

- FOUNDATIONS HAVE BEEN DESIGNED IN ACCORDANCE WITH A REPORT ENTITLED "GEOTECHNICAL REPORT, PROPOSED APARTMENT BUILDING, 221 CONGRESS STREET, PORTLAND, MAINE," PREPARED BY SUMMIT GEOENGINEERING SERVICES, DATED 06/22/2017. THE RECOMMENDATIONS OF THE REPORT ARE PART OF THIS WORK. REFER TO THIS REPORT FOR SPECIFIC RECOMMENDATIONS.
- FOUNDATION DESIGN IS BASED ON SHALLOW SPREAD FOOTINGS BEARING ON SUITABLE UNDISTURBED NATIVE SOILS AND/OR NEW COMPACTED STRUCTURAL FILL EXTENDING TO UNDISTURBED NATIVE SOIL PER THE REQUIREMENTS OF THE GEOTECHNICAL REPORT. REFER TO 4. THIS REPORT FOR SPECIFIC BEARING RECOMMENDATIONS.
- ALLOWABLE BEARING CAPACITY 4,000 PSF EXTEND BOTTOM OF EXTERIOR FOOTINGS AT LEAST 4.0 FEET BELOW THE FINAL EXTERIOR GRADE
- FOR PROTECTION AGAINST FROST. NO FILL FOR BUILDING SUPPORT SHALL BE PLACED UNTIL UPGRADES HAVE BEEN OBSERVED AND APPROVED BY THE GEOTECHNICAL ENGINEER.
- REFERENCE THE GEOTECHNICAL REPORT FOR ALL EXCAVATION, BACKFILL, COMPACTION,
- CONSTRUCTION DEWATERING AND PERMANENT DRAINAGE REQUIREMENTS. SOILS EXPOSED AT THE BASE OF ALL SATISFACTORY FOUNDATION EXCAVATIONS SHOULD BE PROTECTED AGAINST ANY DETRIMENTAL CHANGE IN CONDITION, SUCH AS DISTURBANCE FROM RAIN OR FROST. SURFACE RUNOFF SHALL BE DRAINED AWAY FROM THE EXCAVATIONS AND NOT BE ALLOWED TO POND. FOUNDATION EXCAVATIONS SHALL BE ADEQUATELY PROTECTED FROM RAINFALL OR FREEZING CONDITIONS. GROUNDWATER SHOULD BE ANTICIPATED FOR EXCAVATIONS AND APPROPRIATE DEWATERING MEASURES SHALL BE EMPLOYED.
- EXCAVATIONS FOR BUILDING CONSTRUCTION SHALL BE IN ACCORDANCE WITH OSHA 9. REQUIREMENTS. BRACED EXCAVATIONS SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF MAINE. DO NOT UNDERMINE EXISTING FOUNDATIONS OF ANY ADJACENT STRUCTURES. REFER TO THE GEOTECHNICAL REPORT FOR ADDITIONAL AND/OR MORE SPECIFIC REQUIREMENTS.

<u>METAL DECK</u>

THE METAL ROOF AND FLOOR DECK SHALL BE FORMED OF STEEL SHEETS CONFORMING TO THE FOLLOWING STANDARDS:

A. FLOOR DECKING: ASTM A1008, GRADE C, D OR ASTM A653, STRUCTURAL QUALITY, GRADE 40 OR HIGHER

B. ROOF DECKING: ASTM A1008, GRADE C, D OR ASTM A653, STRUCTURAL QUALITY, GRADE 33 OR HIGHER

- FLOOR AND ROOF DECK SHALL BE AS NOTED ON THE DRAWINGS (OR EQUIVALENT).
- FOR DECK ATTACHMENTS, PENETRATIONS AND ACCESSORIES REFER TO SPECIFICATIONS.

CONCRETE NOTES

- CONCRETE WORK SHALL CONFORM TO "ACI MANUAL OF C LATEST EDITION. THIS PUBLICATION IS AVAILABLE THROUG CONCRETE INSTITUTE (248) 848-3800.
- CONCRETE FOUNDATIONS SHALL HAVE A MINIMUM 28-DAY STRENGTH OF 3,500 PSI. CONCRETE SLABS SHALL HAVE A COMPRESSIVE STRENGTH OF 3,000 PSI, U.N.O. EXTERIOR S HAVE A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 5,00 CONCRETE MIX PERFORMANCE DATA INCLUDING AIR CON RATIO, AGGREGATE SIZE, SLUMP, ETC. HAS BEEN INCLUDE SPECIFICATIONS. SEE THE SPECIFICATIONS FOR ADDITION
- CONCRETE SHALL NOT BE PLACED IN WATER OR ON FROZE 3. PROVIDE PVC SLEEVES WHERE PIPES PASS THROUGH EXT 4. SLABS.
- REINFORCING BARS SHALL CONFORM TO ASTM A615 GRAD 5. AND SHALL BE DETAILED, FABRICATED AND PLACED IN ACC LATEST EDITION.
- WELDED WIRE FABRIC SHALL CONFORM TO ASTM A-185 ANI SHEETS.
- FIBER REINFORCEMENT SHALL BE TYPE III SYNTHETIC VIR POLYPROPYLENE FIBERS CONFORMING TO ASTM C1116.
- MINIMUM CONCRETE PROTECTIVE COVERING FOR REINFOR NOTED OTHERWISE, SHALL BE AS FOLLOWS: SURFACES CAST AGAINST AND PERMANENTLY IN CON
- B. FORMED SURFACES IN CONTACT WITH EARTH OR EXPO #5 BARS, 5/8" DIAMETER WIRE AND SMALLER, 1.5" #6 THROUGH #11 BARS, 2.0" C. SURFACES NOT IN CONTACT WITH EARTH OR EXPOSE WALLS, SLABS, JOISTS #11 BARS AND SMALLER, 1.0" BEAMS, GIRDERS, AND COLUMNS; ALL REINFORCEMENT,
- REINFORCEMENT SHALL BE CONTINUOUS AROUND CORNE INTERSECTIONS. PROVIDE LAPPED BARS AT NECESSARY S BARS AT DISCONTINUOUS ENDS. PROVIDE TENSION LAP SI SCHEDULE THIS DRAWING, FOR ALL REINFORCING UNLESS PLAN.
- WELDING OF REINFORCEMENT IS NOT PERMITTED 10.
- 11. FOR ALL OPENINGS IN CONCRETE WALLS AND SLABS, PRO REINFORCING AROUND OPENING AS SHOWN ON THE CONT TYPICAL DETAILS. NO PENETRATIONS SHALL BE MADE THE WITHOUT WRITTEN PERMISSION FROM ENGINEER.
- CONSTRUCTION JOINTS SHOWN ON DRAWINGS ARE MAND 12. ADDITIONS, OR CHANGES SHALL NOT BE MADE EXCEPT WI WRITTEN REQUEST TOGETHER WITH DRAWINGS OF THE P LOCATIONS FOR APPROVAL OF THE STRUCTURAL ENGINE CONSTRUCTION JOINTS ARE NOT SHOWN, OR WHEN ALTE PROPOSED, DRAWINGS SHOWING LOCATION OF CONSTRU JOINTS AND CONCRETE PLACING SEQUENCE SHALL BE SU STRUCTURAL ENGINEER FOR REVIEW PRIOR TO PREPARA REINFORCEMENT SHOP DRAWINGS. CONCRETE SHALL BE HORIZONTAL CONSTRUCTION JOINTS EXCEPT WHERE SHO VERTICAL CONSTRUCTION JOINTS AND STOPS IN CONCRE BEAMS SHALL BE MADE AT MIDSPAN OR AT POINTS OF MIN NOTED OTHERWISE.
- SPACING OF CONSTRUCTION JOINTS, UNLESS NOTED OTHE 13. FOLLOWS: A. FOOTINGS AND WALLS MAX LENGTH 40'-0" OR 15'-0" FROM ANY CORNER** B. SLABS ON GRADE SEE FOUNDATION PLAN

** EXCEED ONLY WHERE INTERMEDIATE CONTRACTION JO MINIMUM OF 72 HOURS SHALL ELAPSE BETWEEN ADJACEI PLACEMENTS.

- ANCHOR RODS SHALL BE HEADED RODS CONFORMING TO 14. KSI WELDABLE STEEL, UNLESS NOTED OTHERWISE ON DRA THAT ARE TO BE IN CONTACT WITH PRESSURE TREATED LUMBER SHALL GALVANIZED.
- 15. ALL GROUT BENEATH BASE PLATES & BEARING PLATES SH NON-SHRINK GROUT BY U.S. GROUT CORP.
- SLAB THICKNESSES INDICATED ON THE DRAWINGS ARE MIN 16. SUFFICIENT CONCRETE TO ACCOUNT FOR STRUCTURE DE FLUCTUATIONS, AND TO OBTAIN THE SPECIFIED SLAB ELEV FLATNESS AND LEVELNESS INDICATED.
- INSTALLATION OF REINFORCEMENT SHALL BE COMPLETED 17 PRIOR TO THE SCHEDULED CONCRETE PLACEMENT. NOTIF STRUCTURAL ENGINEER OF COMPLETION AT LEAST 24 HO SCHEDULED COMPLETION OF THE INSTALLATION OF REINF
- 18. ALL ITEMS TO BE EMBEDDED INTO CONCRETE SHALL BE IN PLACEMENT OF CONCRETE. PROVIDE ADDITIONAL REINFO TEMPLATES AS REQUIRED TO ENSURE THE CORRECT POS "WET SETTING" OF EMBEDMENTS INTO CONCRETE IS STRI EMBEDMENTS INCLUDE, BUT NOT BY LIMITATION, REINFOR DOWELS, EMBEDDED PLATES, ANCHOR RODS, ANCHOR INS TRANSFER PLATES, DIAMOND DOWELS, AND SHELF BULK H

STRUCTURAL STEEL NOTES

- STRUCTURAL STEEL FABRICATION, ERECTION, AND CONNE CONFORM TO AISC "SPECIFICATION FOR THE DESIGN FABR ERECTION OF STRUCTURAL STEEL" LATEST EDITION, AND STANDARD PRACTICE", LATEST EDITION.
- STRUCTURAL STEEL: STEEL PLATES, SHAPES, AND BARS, (UNLESS NOTED OTHER WISE (U.N.O.). STRUCTURAL STEEL ON THE DRAWINGS FOR WIDE-FLANGE SECTIONS: ASTM AS 50 WITH SPECIAL REQUIREMENTS PER AISC TECHNICAL BU MARCH, 1997)
- STRUCTURAL TUBING: CONFORM TO ASTM A500 GRADE B4 CONNECTION DESIGN FOR THIS PROJECT IS THE RESPONS FABRICATOR. CONNECTION CALCULATIONS, SIGNED AND S PROFESSIONAL ENGINEER LICENSED IN THE STATE OF MA SUBMITTED WITH THE SHOP DRAWINGS FOR THIS PROJECT
- SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS. FIELD CONNECTIONS SHALL BE BOLTED USING ASTM A325N BOLTS (U.N.O.) EXCEPT WHERE SLIP CRITICAL CONNECTIO NOTED BY A325 (SC) ON THE DRAWINGS. PROVIDE SLIP CR CONNECTIONS AT ALL MOMENT CONNECTIONS, BRACED F ANGLES AND AS OTHERWISE NOTED. USE A490 BOLTS WHI
- WHERE WELDING IS INDICATED, ALL WELDING SHALL CONF D1.1-LATEST EDITION. ELECTRODES SHALL CONFORM TO A WITH PROPER ROD TO PRODUCE OPTIMUM WELD (LOW HYL
- SEE CONCRETE NOTES AND DRAWINGS FOR ANCHOR BOL PROVIDE 3/8" MINIMUM STIFFENER PLATES EACH SIDE OF E 8.
- FRAMING OVER COLUMNS AND AT BEAMS SUPPORTING CO PROVIDE 1/4" THICK LEVELING PLATE UNDER ALL COLUMN OTHERWISE NOTED. LEVELING PLATES SHALL BE SET AND
- ERECTING COLUMNS. PROVIDE ALL MISCELLANEOUS ANGLES, PLATES, ANCHOR BLOTS ETC., SHOWN ON 10. ARCHITECTURAL DRAWINGS FOR SUPPORT OF BLOCKING, PARAPETS, FINISHES, ETC. COORDINATE WITH MISCELLANEOUS METAL FABRICATOR TO ENSURE COMPLETE COVERAGE OF ALL ITEMS.
- PROVIDE L 4 x 4 x 1/4 SLAB SUPPORT ANGLE AS REQUIRED AT COLUMNS WHERE 11. STRUCTURAL MEMBERS DO NOT FRAME IN AT ALL FOUR SIDES.

| | <u>MASON</u> | <u>NRY NOTES</u> | <u>ABBREVIATIONS</u> | |
|---|---------------|---|-----------------------------|--|
| ONCRETE PRACTICE", | 1. | ALL MASONRY CONSTRUCTION SHALL CONFORM TO ACI 530.1-LATEST. | A.B. | ANCHOR BOLT |
| SH THE AMERICAN | 2. | ALL CONCRETE MASONRY UNITS WALL BE ASTM C90 GRADE N, TYPE I STANDARD WEIGHT BLOCKS INCLUDING STRETCHERS AND CORNER BLOCKS. | ABV ACI | ABOVE AMERICAN CONCRETE INSTITUTE |
| COMPRESSIVE | | MINIMUM PRISM STRENGTH OF BLOCK SHALL BE F'M = 1500 PSI IN 28 DAYS. | ACT ADDI | ACOUSTICAL CEILING TILE |
| MINIMUM 28-DAY SLAB-ON-GRADE SHALL | 3. 1 | MORTAR SHALL CONFORM TO ASTM SPECIFICATION C270, TYPE M OR S | AESS | ARCHITECTURAL EXPOSED STRUCTURAL STEEL |
| 00 PSI. ADDITIONAL | 4. 5. | REINFORCING FOR BOND BEAMS, LINTEL BLOCKS AND VERTICAL WALL | A.F.F ALT | ABOVE FINISH FLOOR ALTERNATE |
| ED IN THE PROJECT | _ | REINFORCING SHALL BE BILLET STEEL CONFORMING TO ASTM A615, GRADE 60 | ALUM | ALUMINUM |
| IAL REQUIREMENTS. | 6. | HORIZONTAL JOINT REINFORCING SHALL BE DUR-O-WAL TRUSS DESIGN, STANDARD CLASS MILL GALVANIZED WITH 3/16" DIAMETER SIDE RODS AND 9 | APA APPROX | AMERICAN PLYWOOD ASSOCIATION APPROXIMATE |
| EN GROUND. TERIOR CONCRETE, OR | | GAUGE CROSS TIES, U.N.O. REINFORCING SHALL BE PLACED IN MASONRY WALLS AT EVERY SECOND BLOCK COURSE. | A.R. ARCH | ANCHOR ROD ARCHITECT OR ARCHITECTURAL |
| DE 60 DEFORMED BARS CORDANCE WITH ACI 315, | 7. | CONCRETE MASONRY UNITS SHALL BE LAID IN RUNNING BOND UNLESS OTHERWISE NOTED. PROVIDE FULL MORTAR COVERAGE ON ALL WEBS AND FACE SHELLS. PROVIDE CORNER BLOCKS AND END BLOCKS TO FINISH ALL 90 | BAL B.C.X. BD | BALANCE BOTTOM CHORD EXTENSION BOARD |
| ID BE PROVIDED IN FLAT | 8. | DEGREE CORNERS AND WALL OPENINGS. PROVIDE LINTELS AT WALL PENETRATIONS AS SHOWN IN THE LINTEL | B.F. BLDG | BRACED FRAME BUILDING |
| GIN HOMOPOLYMER | 9 | SCHEDULE. STANDARD I AP I ENGTH OF GRADE 60 MASONRY REINFORCING BARS SHALL BE | BLKG BM | BEOCKING BEAM |
| RCEMENT, UNLESS | 0. | 48 BAR DIAMETERS FOR BARS #5 AND SMALLER. PROVIDE MECHANICAL SPLICES RATED FOR 125% THE BAR YIELD STRENGTH FOR BARS #6 AND | BIT B.O. BOT | BITUMINOUS BOTTOM OF/ BY OTHERS BOTTOM |
| TACT WITH EARTH, 3.0" OSED TO WEATHER | 10. | CELLS TO BE GROUTED SHALL BE 2-CELL BLOCK. ALIGN CELLS TO MAINTAIN A CLEAR UNOBSTRUCTED, CONTINUOUS VERTICAL CHASE. CELLS MUST BE KEPT | B.P. B.PL BRG | BEAM POCKET BASE PLATE BEARING |
| D TO WEATHER | | CLEAN OF PROTRUSIONS OR FINS OF MORTAR. FILL CELLS OF MASONRY UNITS AND WALL CAVITIES WHERE INDICATED WITH 2500 PSI GROUT. MAXIMUM GROUT LIFT WITHOUT CLEAN-OUTS SHALL BE 4'-0". HIGH LIFT GROUTING SHALL | B.S. BSMT BTWN | BOTH SIDES BASEMENT BETWEEN |
| 1.5" | | CONFORM TO CODE REQUIREMENTS WITH A MINIMUM CEMENT CONTENT OF 8 SACKS PER CUBIC YARD. SUPPORT ALL VERTICAL BARS IN CENTER OF GROUTED CELLS WITH VERTICAL BAR POSITIONER. | C/C C | CENTER TO CENTER CHANNEL |
| RS AND AT | 11. | FIELD PENETRATIONS THROUGH BLOCK WALLS SHALL NOT BE MADE THROUGH | CFMF C I P | COLD FORM METAL FRAMING CAST IN PLACE |
| PLICES OR HOOKED | | BOND BEAMS, LINTELS OR GROUTED CELLS. | C.J. | CONTRACTION/CONST. JOINT |
| S OTHERWISE SHOWN ON | <u>LINTEL</u> | <u>S</u> | ∉ CLG | CENTERLINE CEILING |
| | 1. | THE FOLLOWING LINTELS SHALL BE USED FOR MASONRY OPENINGS, U.N.O. ON | CLR | CLEAR |
| VIDE SUPPLEMENTAL | | DRAWINGS: | COL | COLUMN |
| RACT DOCUMENTS ROUGH FOOTINGS | | MASONRY OPENING LINTEL SIZE | CONC CONN | CONCRETE |
| | | 0P TO 3'-0" L 3 1/2 x 3 1/2 x 5/16 3'-1" TO 4'-6" L 4 x 3 1/2 x 5/16 (LLV) | CONST | CONSTRUCTION |
| ATORY. OMISSIONS, TH THE SUBMITTAL OF A | | 4'-7" TO 6'-0" L 5 x 3 1/2 x 5/16 (LLV) 6'-1" TO 8'-0" L 6 x 3 1/2 x 5/16 (LLV) | CONT CONTR | CONTRACTOR |
| ROPOSED JOINT | 2 | PROVIDE ONE ANGLE FOR EACH 4" WALL THICKNESS. FOR 6" WALL THICKNESS. | COORD | COORDINATE |
| ER. WHERE RNATE LOCATIONS ARE | L . | PROVIDE WIT OR BUILT-UP SECTION WITH PROPERTIES EQUAL TO OR GREATER | | |
| ICTION AND CONTROL | 3 | THAN 1 ½ TIMES THE ANGLES PROPERTIES FOR A 4" WALL THICKNESS. PROVIDE 8" OF BEARING AT EACH END OF ALL LINTELS. | a DBL | PENNY DOUBLE |
| TION OF THE | <i>4.</i> | ALL EXTERIOR LINTELS SHALL BE HOT-DIPPED GALVANIZED. | DIA OR ∅ DIA G | DIAMETER |
| PLACED WITHOUT WWN OR NOTED. | TIMBEF | RNOTES | DIM | DIMENSION |
| TE BEAMS/ GRADE | | | DL DN | DEAD LOAD DOWN |
| IIMUM SHEAR, UNLESS | 1. | ALL TIMBER FRAMING SHALL BE IN ACCORDANCE WITH THE ATTC TIMBER CONSTRUCTION MANUAL- LATEST EDITION, AND THE AF & PA NATIONAL DESIGN | DO/do | DITTO/DO OVER |
| ERWISE SHALL BE AS | | SPECIFICATION FOR WOOD CONSTRUCTION (NDS) LATEST EDITION. | DP DTL(S) | DRILLED PIER OR DEEP DETAIL(S) |
| | 2. | INDIVIDUAL TIMBER FRAMING MEMBERS SHALL BE VISUALLY GRADED. MINIMUM GRADE NO1/NO2 SPRUCE-PINE-FIR KILN DRIED TO 19% MAXIMUM MOISTURE CONTENT UNLESS OTHERWISE INDICATED ON THE DRAWINGS. | DWG(S) DWL(S) | DRAWING(S) DOWEL(S) |
| | 3. | ENGINEERED WOOD PRODUCTS SHALL BE AS SPECIFIED ON THE DRAWINGS. REFER TO MANUFACTURER'S LITERATURE FOR PROPER HANDLING AND INSTALLATION GUIDELINES, MANUFACTURER AND PRODUCT SHALL BE: | (E) OR EXIST EA. E.E. | EXISTING EACH EACH END |
| DINTS ARE PROVIDED. NT CONCRETE | | I-LEVEL: I-JOIST (T.II) PARALLAM (PSL) MICROLAM (LVL) | E.F. E.J. | EACH FACE EXPANSION JOINT |
| | | | EL FL EV | ELEVATION ELEVATOR |
| ASTM F1554, GRADE 36 | 1 | BOISE: VERSALAM (LVL) | ELEC | ELECTRICAL |
| AWINGS. ANCHOR RODS | 4. | EXPOSURE, OR WHERE SHOWN ON THE DRAWINGS. TIMBER SHALL BE SOUTHERN | EMBED ENGR | EMBEDMEN I ENGINEER |
| BE HOT-DIPPED | | YELLOW PINE TREATED WITH CCA OR ACQ TO 0.4 #/CF IN ACCORDANCE WITH AWPA C-18. ACZA IS STRICTLY PROHIBITED. | E.O.P. E O B | EDGE OF DECK |
| IALL BE "5-STAR" 5000-PSI | 5. | ALL ROOF AND WALL SHEATHING SHALL BE APA PERFORMANCE-RATED. SHEATHING | E.O.S. | EDGE OF SLAB |
| | | SHALL BE NAILED TO THE FRAMING AS FOLLOWS, U.N.O.: | EQ EQ SP | EQUAL EQUALLY SPACED |
| NIMUMS. PROVIDE FLECTION, SUBGRADE | | A. ROOFS: 8d NAILS AT 6" O.C. AT SUPPORTED PANEL EDGES AND 12" O.C. AT | EQUIP | EQUIPMENT |
| VATION AT THE | | INTERMEDIATE SUPPORTS. B. WALLS: 8d NAILS AT 6" O.C. AT SUPPORTED PANEL EDGES AND 12" O.C. AT | E.S. E.W. | EACH SIDE EACH WAY |
|) AT I FAST 24 HOURS | | INTERMEDIATE SUPPORTS. | E.W.B. | EACH WAY BOTTOM |
| TY ARCHITECT AND | 6. | FLOOR SHEATHING SHALL BE 3/4", APA RATED TONGUE AND GROOVE PANELS. GLUE AND NAIL TO ELOOR ERAMING WITH 8d RING SHANK NAILS AT 6" O.C. AT SUPPORTED | EXP ANCHOR | EXPANSION ANCHOR |
| URS PRIOR TO THE FORCEMENT. | | PANEL EDGES AND 12" O.C. AT INTERMEDIATE SUPPORTS. HUBER ADVANTECH | EXP FXT | EXPANSION EXTERIOR |
| ISTALLED PRIOR TO | | TONGUE AND GROOVE PANELS MAY BE SUBSTITUTED ONLY WITH WRITTEN PERMISSION FROM THE ARCHITECT. | | |
| RCEMENT AND/OR ITIONS OF EMBEDMENTS. | 7. | ALL BUILT-UP BEAMS AND COLUMNS SHALL BE NAILED AS FOLLOWS (FASTENING IN | F.D. | FLOOR DRAIN |
| CTLY PROHIBITED. | | EACH PLY): | FDN FIN. FL. | FOUNDATION FINISH FLOOR |
| SERTS, SLEEVES, LOAD | | UNIFORMLY LOADED BEAMS: | F.F. | FINISH FLOOR/ FAR FACE |
| IEADS. | | BEAM DEPTH <16 - 2 ROWS OF 160 NAILS AT 12 'O.C., STAGGERED BEAM DEPTH >=16" - 3 ROWS OF 160 NAILS AT 12" O.C. STAGGERED | FLG FLR | FLOOR |
| | | NOTE: SIDE LOADED BEAMS REQUIRE ADDITIONAL FASTENING. SEE DETAILS. | F.F.E. | FINISH FLOOR ELEVATION |
| ECTION DESIGN SHALL | | COLUMNS: | F.O. | FACE OF |
| RICATIONS, AND THE "CODE OF | | 2-10d NAILS AT 6" O.C. | FRMG F.S. | FRAMING FAR SIDE |
| | 8. | FASTENING NOT SPECIFIED SHALL CONFORM WITH IBC 2009 TABLE 2304.9.1. NAIL FASTENERS SHALL MEET THE REQUIREMENTS OF ASTM F1667. UNLESS NOTED | F.T. | FOOT OR FEET |
| CONFORM TO ASTM A36 SHAPES DESIGNATED | | OTHERWISE, NAILS REFERENCED ON DRAWINGS ARE TO BE COMMON NAILS WITH DIMENSIONS AS FOLLOWS. | GA | GAGE/GAUGE |
| 992 (ASTM A572 GRADE II I FTIN #3 DATED | | 64: 2" LONG BY 0 112" DIAMETER SHANK WITH 0 266" DIAMETER HEAD | GALV | GALVANIZED |
| | | 8d: 2 1/2" LONG BY 0.131" DIAMETER SHANK WITH 0.281" DIAMETER HEAD | G.B. | GRADE BEAM |
| 6 KSI. | | 10d: 3" LONG BY 0.148" DIAMETER SHANK WITH 0.312" DIAMETER HEAD 12d: 3 1/4" LONG BY 0.148" DIAMETER SHANK WITH 0.312" DIAMETER HEAD | G.C. | GENERAL CONTRACTOR |
| SIBILITY OF THE SEALED BY A | | 16d: 3 1/2" LONG BY 0.162" DIAMETER SHANK WITH 0.344" DIAMETER HEAD | GWB | GYPSUM WALL BOARD |
| INE SHALL BE | | 200: 4" LONG BY 0.192" DIAME LER SHANK WITH 0.406" DIAMETER HEAD 30d: 4 1/2" LONG BY 0.207" DIAMETER SHANK WITH 0.438" DIAMETER HEAD | H.D. | HOLDOWN |
| 1. SEE THE PROJECT | 9. | ALL TIMBER CONNECTION HARDWARE (JOIST HANGERS, POST BASES, SHEARWALL | H.D. GALV HK | HOT DIPPED GALVANIZED HOOK |
| N HIGH STRENGTH | | HOLDOWNS, ETC) SHALL BE AS INDICATED ON THE DRAWINGS AND MANUFACTURED | HORIZ | HORIZONTAL |
| INS ARE REQUIRED AND ITICAL (SC) | | GALVANIZED G-90 (U.N.O.). CONNECTION HARDWARE USED IN CONJUNCTION WITH | HVAC | HEATING VENTILATION & COOLING |
| RAMES, RÉLIEVING | | PRESERVATIVE TREATMENT SHALL BE GALVANIZED G185 (ZMAX) USE FASTENERS AND HANGERS OF SAME MATERIAL & COATING REFER TO MANUFACTURED'S | HSS | HOLLOW STRUCTURAL SHAPE |
| ERE INDICATED. FORM TO AWS | | LITERATURE FOR PROPER HANDLING AND INSTALLATION GUIDELINES. | | |
| WS A5.1 E70XX SERIES | 10. | FASTENERS USED IN CONJUNCTION WITH PT LUMBER, BUT NOT AT TIMBER | | |
| DROGEN) | | CONNECTION HARDWARE REFERENCED IN NOTE ABOVE, SHALL BE POST HOT-DIPPED GALVANIZED (ASTM A153). | | |
| BEAM WEB AT BEAMS | | | | |
| DLUMNS ABOVE. | | | | |
| BASE PLATES UNLESS GROUTED PRIOR TO | | | | |
| | | | | |

ABBREVIATIONS

I.L

IN

| | INSIDE DIAMETER |
|----------------|--|
| =0 | INFORMATION |
| | INSIDE FACE INCH |
| T T | INTERIOR |
| 1 | JOINT KIPS (1K=10001 BS) |
| | ANGLE LENGTH |
| (S) | POUND(S) LIVE LOAD |
| BB | LONG LEGS BACK TO BACK |
| H | LONG LEG HORIZ |
| V | LONG LEG VERT |
| VC(S) | LOCATION(S) OR LOCATE |
| L | LAMINATED STRAND LUMBER LIGHT |
| S | TENSION LAP SPLICE LENGTH |
| WT | LIGHTWEIGHT |
| L | LEVEL OR LAMINATE VENEER LUMBER |
| NCH | MACHINE |
| ACH RM | MACHINE ROOM |
| AS | MASONRY |
| ATL | MATERIAL |
| AX | MAXIMUM |
| E.P. MITE | MECHANICAL MECHANICAL/ELECTRICAL/PLUMBING MANUEACTURER |
| N | MINIMUM |
| SC | MISCELLANEOUS |
| | MICRO-LAM |
| O. | MASONRY OPENING |
| TL | METAL NORTH |
| .C | NOT IN CONTRACT |
|) OR # | NUMBER |
| OM | NOMINAL |
| S | NORTH-SOUTH |
| S. | NEAR SIDE |
| T.S. | NOT TO SCALE |
| C. | ON CENTER |
| D. | OUTSIDE DIAMETER |
| H. PNG | OPPOSITE HAND OPFNING |
| P | OPPOSITE |
| A F | POWDER ACTUATED EASTENER |
| С. | PILE CAP |
| С. | PORTLAND CONCRETE ASSOCIATION |
| N | PENETRATION |
| RP | PERPENDICULAR |
| CS | PLATE PLACES POLINDS BED LINEAD FOOT |
| r F | POUNDS PER LINEAR FOOT POUNDS PER SQUARE FOOT POUNDS PER SQUARE INCH |
| REFAB RELIM | PREFABRICATION PRELIMINARY |
| Г. | PRESSURE TREATED |
| ′С | POLYVINYL CHLORIDE |
| Y | QUANTITY RADIUS |
| OR REF | REFER TO REFERENCE |
| D. | ROOF DRAIN |
| | REINFORCE(ING)(D)(MENT) REQUIRED |
| EQMNTS | REQUIREMENT(S) |
| D. | ROUGH OPENING |
| 0 C. | SLIP CRITICAL |
| HED | SCHEDULE |
| CT | SECTION |
| = | SOUABE FOOT |
| IT M | SHEET SIMILAR |
| H | SHORT LEG HORIZONTAL |
| V | SHORT LEG VERTICAL |
| 0 | SOUTH SPACE AT |
| , | SPACE(S) |
| PECS | SPECIFICATIONS |
| < | SHEAR KEY |
| | SHEAR LUG |
| 5. | STAINLESS STEEL |
| ELT | SHORT SLOT |
| D | STANDARD |
| L RUCT | STEEL STRUCTURAL STIEFENED |
| IFF | STIFFENER |
| N. | SHEARWALL |
| M | SYMMETRICAL |
| _ | TOP |
| B | TOP AND BOTTOM |
| C.F. | TOP CHORD EXTENSION |
| к | TOTAL LOAD |
| І. | TIE JOIST |
| D. OR T/ | TOP OF |
| D.S. T/STL | TOP OF STEEL etc. |
| ANS | TRANSVERSE |
| P | TYPICAL |
| N.O. | UNLESS NOTED OTHERWISE |
| RT | VERTICAL |
| .F. | VERIFY IN FIELD WITH |
| 70 | WITHOUT |
| D | WIDTH OR WOOD |
| _ | WIDE ELANGE |

WORK POINT

WELDED WIRE FABRIC

WEIGHT

WP

WT

W.W.F.

шн V V

~ ш

5-







Hay 221