

March 27, 2017 Summit #16030

BD Sheridan LLC 1266 Furnace Brook Parkway Suite 300 Quincy, MA 02169 Attn: Rich and Bernie

Reference: Geotechnical Letter – Proposed Apartment Building

155 Sheridan Street, Portland, Maine

Dear Sirs;

Summit Geoengineering Services (SGS) performed a geotechnical exploration at the above referenced site on March 10, 2016. The explorations consisted of 3 borings drilled at the top, mid-height, and toe of the existing slope. Laboratory testing was performed on soil samples collected during the exploration. Additionally, a monitoring well was installed in the boring at the top of the slope. Attached to this letter are the boring logs, the laboratory testing results, and a soil cross section.

We understand that the new apartment building will be constructed as a slab-on-grade with a finish floor elevation (FFE) of 111.50 feet and a soldier pile and lagging wall will be constructed on the north end of the site to retain the cut slope. Based on the proposed layout and the soil conditions encountered in our explorations, we do not anticipate any major geotechnical concerns with the new development. Further geotechnical analyses are required for us to provide final geotechnical recommendations for the design and construction of the building foundation and the retaining wall. We will submit the final geotechnical report within two weeks.

If there are any questions, please feel free to contact me.

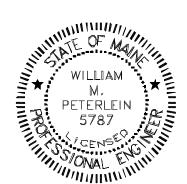
Sincerely,

Summit Geoengineering Services

William Matule.

William M. Peterlein, P.E.

President & Principal Engineer

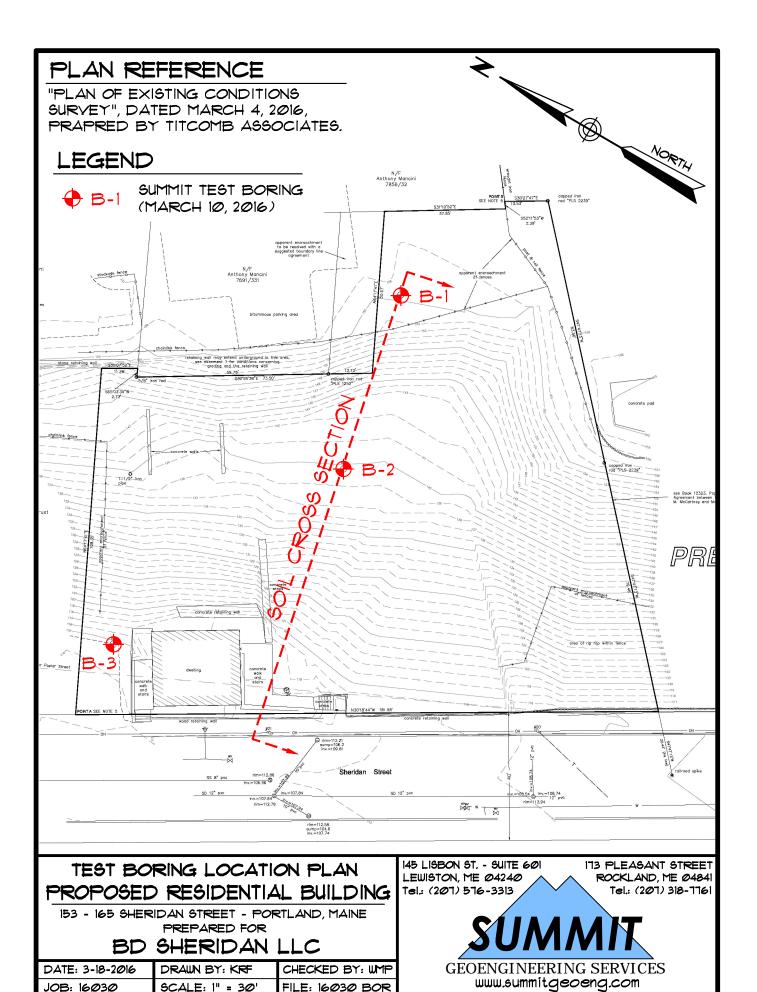






Attachments:

Boring Location Plan Boring Logs Soil Cross Section Laboratory Test Results



						S	OIL BORII	NG LOG	Boring #:	B-1 (OW)	
<i>SUMMI</i>						Project: New Apartment Building Project #:					16030
GEOENGINEERING SERVICES				Location: 153 - 165 Sheridan Street Sheet:					1 of 3		
				City, State:	Portland, Maine		Chkd by:				
Drilling Co: Northern Test Boring			Boring Elevation	:	155.0 ft. +/-	-					
Driller:				Reference: "Existing Conditions Survey, 153-165 Sheridan St." by Titcomb Dated							
Summit :	Staff:	M. Hardison, E	.l			Date started:		Date Completed:	3/10/2016		
DR	RILLING	METHOD		AMPLER				ESTIMATED GROUND	WATER DEPTH		
Vehicle:		ATV	Length:	24" SS		Date	Depth	Elevation	Re	ference	
Model:		Diedrich D-50		2"OD/1.5"	'ID	3/12/2016	11.2 ft.	143.8 ft. +/-	2 days after complet	ion of b	ooring
Method:		3" Case Wash	Hammer:	140 lb							
Hammer	Style:	Auto	Method:	ASTM D15	586						
Depth					Elev.		SAMPL	.E	Geological/	(Geological
(ft.)	No.	Pen/Rec (in)	Depth (ft)	blows/6"	(ft.)		DESCRIPT		Test Data		Stratum
	S-1	24/6	0 to 2	3				fine Sand, trace Clay,			
1_				2		rootlets and orga	anics, very loose	e, damp, ML			TOPSOIL
_				2	455						
2_				2	153.0'						
_				1							EU.
3_				1	-						FILL
4				1	1						
4_				+	1						
5					_						
J -	S-2	24/16	5 to 7	1	1	Dark tan to brow	n Silty SAND w	vood pieces in spoon			
6		2 ., 10	0 10 7	2	_	tip, very loose, h	-	rood piooco iii opooii			
I -				2		,, ,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,					
7				1	_						
_											
8					147.0'					Ш	
I _						Dense drilling en	countered at 8.	0' depth			<u></u>
9											LACIAL TILL
10_											
	S-3	24/24	10 to 12	11	1			ittle Sand and Clay,	PP >> 9,000 psf		
11_				17		hard, humid, ML					
10				27					Groundwater	ΙĦ	
12_				21							
13											
13_	S-4	24/20	13 to 15	13	_	same as above,	some Cobble ni	eres	PP >> 9,000 psf		
14	3 7	24/20	13 (0 13	13		Suric as above,	some cobbie pi	0003	11 >> 7,000 psi		
				21	_						
15				20	_						
_											
16					1						
_											
17											
Ī											
18											
	S-5	24/24	18 to 20	25				Sand and Clay, very			
19_		ļ		30	1	dense, humid, G	M				
				27							
20_		+		27							
21		+		1	-						
21_					-						
22					1						
<u> </u>		<u> </u>		+	1						
					1						
Granula	ar Soils	Cohesiv	re Soils	% Comp	osition	NOTES:	PP = Pocket Pen	etrometer, MC = Moisture	Content	Soil M	loisture Condition
Blows/ft.			Consistency	ASTM E				, PI = Plastic Index, FV =			Dry: S = 0%
0-4	V. Loose	+	V. soft			Bedrock Joints		Shear Strength, Su(r) = Re			d: S = 1 to 25%
5-10	Loose	2-4	Soft	< 5%	Trace	Shallow = 0 to 35		3	3		o: S = 26 to 50%
11-30	Compac	t 5-8	Firm	5-15%	Little	Dipping = 35 to 55	•				: S = 51 to 75%
31-50	Dense	9-15	Stiff	15-30%		Steep = 55 to 90 c	=				S = 76 to 99%
>50	V. Dense	e 16-30	V. Stiff	> 30%	With					Satur	ated: S = 100%
		>30	Hard			Boulders = diamet	er > 12 inches, C	obbles = diameter < 12 inc	ches and > 3 inches		
l						Gravel = < 3 inch	and > No 4, Sand	I = < No 4 and > No 200, S	Silt/Clay = < No 200		

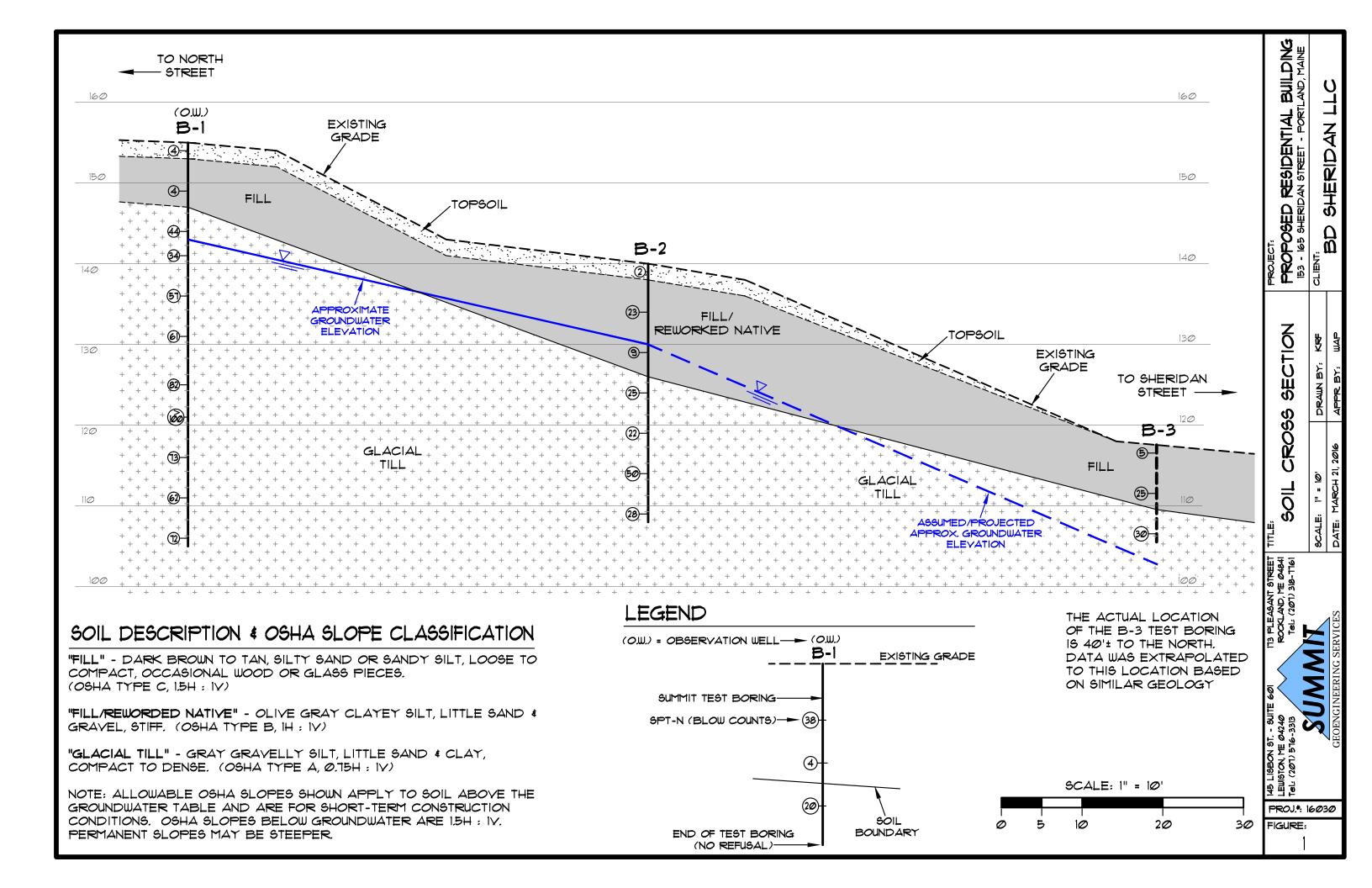
		<u> </u>				S	OIL BORII	NG LOG	Boring #:	B-1 (OW)
		<i>SUMMì</i>	I			Project:	New Apartmen	Project #:	16030	
	GEO	ENGINEERING SERV	ICES				153 - 165 Sher	•	Sheet:	2 of 3
						City, State:	Portland, Main	Chkd by:		
Drilling Co: Northern Test Boring			Boring Elevation: 155.0 ft. +/-							
Driller:					Reference: "Existing Conditions Survey, 153-165 Sheridan St." by Titcomb Dated 3/4/					
Summit		M. Hardison, E				Date started:	3/10/2016	Date Completed:	3/10/2016	
		METHOD		AMPLER			1	ESTIMATED GROUND		
Vehicle:		ATV	Length:	24" SS		Date	Depth	Elevation		ference
Model:		Diedrich D-50		2"OD/1.5"	'ID	3/12/2016	11.2 ft.	143.8 ft. +/-	2 days after comple	tion of boring
Method:		3" Case Wash		140 lb	-07					
Hammer	Style:	Auto	Method:	ASTM D15	1		CAMPI		Coological/	Coological
Depth (ft.)	No.	Pen/Rec (in)	Depth (ft)	blows/6"	Elev. (ft.)		SAMPL DESCRIPT		Geological/ Test Data	Geological Stratum
(11.)	INO.	Tenrice (III)	Deptil (it)	DIOWS/O	(11.)		DESCRIF	TON	Test Data	Stratum
23						1				
	S-6	24/28	23 to 25	13		Grav Gravelly SA	AND, little Silt, t	race Clay, very dense,		GLACIAL TILL
24				22		humid to moist,		J, J 222100)		
1 -				39		1				
25_				29		1				
1						1				
26_				ļ						
						1				
27_				1		1				
20						1				
28_				 						
29						1				
~′-	S-7	24/20	29 to 31	40		same as above,	little Clay, some	e Silt		
30				34			<i>J</i> ,			
1 -				48		1				
31_				47						
						1				
32_						1				
22				-						
33_	S-8	24/20	33 to 35	33		Gray SILT little	Sand and Clay	trace Gravel, very		
34	3-0	24/20	33 10 33	37		dense, humid to		trace Graver, very		
54_				50/4"		derise, nama to	moist, WL			
35										
_										
36_										
37_										
20				1		1				
38_	S-9	24/18	38 to 40	27		same as above				
39	3-9	24/10	30 (0 40	31		same as above				
37_				42		1				
40				43		1				
_										
41_						1				
						1				🗏
42_				<u> </u>						
40				1		1				
43_	S-10	24/24	43 to 45	15		same as above				
44	3-10	24/24	73 IU 43	30		same as above				
				32						
				40						
Granula	ar Soils	Cohesiv	re Soils	% Comp	osition	NOTES:	PP = Pocket Pen	etrometer, MC = Moistu	re Content	Soil Moisture Condition
Blows/ft.	Density	Blows/ft.	Consistency	ASTM D	2487]	LL = Liquid Limit	, PI = Plastic Index, FV	= Field Vane Test	Dry: S = 0%
	V. Loose		V. soft]		Bedrock Joints		Shear Strength, Su(r) =	Remolded Shear Strength	Humid: S = 1 to 25%
5-10	Loose	2-4	Soft	< 5% 7		Shallow = 0 to 35	-			Damp: S = 26 to 50%
	Compac		Firm	5-15%		Dipping = 35 to 5	_			Moist: S = 51 to 75%
31-50	Dense	9-15	Stiff	15-30%		Steep = 55 to 90	degrees			Wet: S = 76 to 99%
>50	V. Dense	16-30 >30	V. Stiff Hard	> 30%	With	Boulders ""-"	40 hada		! inches and > 3 inches	Saturated: S = 100%

		/\\				S	OIL BORII	NG LOG	Boring #:	B-1 (OW)
SUMMIX						Project:	16030			
	GEOENGINEERING SERVICES					153 - 165 Sher		Sheet: Chkd by:	3 of 3	
			City, State:							
Drilling (Co:	Northern Test	Boring			Boring Elevation		155.0 ft. +/-		
Driller: Summit	Ctoff.	Mike Nadeau M. Hardison, E	- 1			Reference: Date started:	"Existing Co 3/10/2016	onditions Survey, 153- Date Completed:	165 Sheridan St." by Tit 3/10/2016	comb Dated 3/4/16
		METHOD		AMPLER		Date started:	3/10/2010	ESTIMATED GROUND		
Vehicle:	ILLING	ATV	Length:	24" SS		Date	Depth	Elevation		ference
Model:		Diedrich D-50		2"OD/1.5"	'ID	3/12/2016	11.2 ft.	143.8 ft. +/-	2 days after comple	
Method:		3" Case Wash		140 lb						
Hammer	Style:	Auto	Method:	ASTM D15						
Depth	<u>.</u>	D (D (1)	D 11 (61)		Elev.		SAMPL		Geological/	Geological
(ft.)	No.	Pen/Rec (in)	Depth (ft)	blows/6"	(ft.)		DESCRIP	ION	Test Data	Stratum
46										GLACIAL TILL
47_										
48_	C 11	24/24	40 to 50	ΕZ	1	Croy CILT 1941	Cand and Ole	traca Craval		
49	S-11	24/24	48 to 50	56 38	1	dense, humid to	-	trace Gravel, very		
- - ا				34	1	donso, nama to	o.o., IVIL			
50				44	105.0'					
_						End of Boring at	50.0', no refus	al <u></u>		
51_					1					
52						Monitoring Well	Installed:			
						2" PVC Well Scre		to 31.5'		
53						2" PVC Riser Pip	e from 31.5' to	ground surface		
54_										
55										
_										
56_										
57_										
58										
_										
59_										
60										
61_					1					
					4					
62_					-					
63					1					
_					1					
64_					4					
65					ł					
00_					1					
66_]					
					1					
67_					1					
					†					
Granula	r Soils	Cohesiv	ve Soils	% Comp	osition	NOTES:	PP = Pocket Per	etrometer, MC = Moistu	re Content	Soil Moisture Condition
Blows/ft.	Density	Blows/ft.	Consistency	ASTM D	2487		LL = Liquid Limit	, PI = Plastic Index, FV	= Field Vane Test	Dry: S = 0%
	V. Loose		V. soft		_	Bedrock Joints		Shear Strength, Su(r) =	Remolded Shear Strength	Humid: S = 1 to 25%
5-10	Loose	2-4	Soft	< 5% T		Shallow = 0 to 35	•			Damp: S = 26 to 50%
11-30 31-50	Compaci Dense	5-8 9-15	Firm Stiff	5-15% 15-30%		Dipping = 35 to 55 Steep = 55 to 90 o	_			Moist: $S = 51 \text{ to } 75\%$ Wet: $S = 76 \text{ to } 99\%$
	V. Dense		V. Stiff	> 30%		2.00p = 00 to 70 t	9.000			Saturated: S = 100%
		>30	Hard			Boulders = diamet	ter > 12 inches, (Cobbles = diameter < 12	inches and > 3 inches	
						Gravel = < 3 inch	and > No 4, San	$d = \langle No 4 \text{ and } \rangle No 200$	D, Silt/Clay = < No 200	

		/				S	OIL BORII	NG LOG	Boring #:	B-2
	,	<i>SUMM</i>	T			Project:	Project: New Apartment Building Project:			16030
	GEC	DENGINEERING SERV	VICES				153 - 165 Sher	•	Sheet:	1 of 2
						City, State:	Portland, Main	е	Chkd by:	
Drilling Co: Northern Test Boring				Boring Elevation	Boring Elevation: 139.5 ft. +/-					
Driller: Mike Nadeau			Reference:		onditions Survey, 153-16		comb Dated 3/4/16			
Summit		M. Hardison, E				Date started:	3/10/2016	Date Completed:	3/10/2016	
		METHOD		AMPLER			1	ESTIMATED GROUND V		
Vehicle:		ATV	Length:	24" SS	up.	Date	Depth	Elevation		ference
Model: Method:		Diedrich D-50 2.25" ID H.S.A.		2"OD/1.5' 140 lb	טו	3/10/2016	20.0 ft.	119.5 ft. +/-	observed on sample	es
Hammer		Auto	Method:	ASTM D15	586					
Depth	otyle.	Auto	wethou.	NOTHI DIC	Elev.		SAMPL	F	Geological/	Geological
(ft.)	No.	Pen/Rec (in)	Depth (ft)	blows/6"	(ft.)		DESCRIPT		Test Data	Stratum
	S-1	24/20	0 to 2	2	` ,	Black SILT, freq	uent roots, root	tlets, and organics, soft,		
1				1		damp, ML		, , , ,		TOPSOIL
1				1						
2_				1	137.5'					
										FILL (DELVODICED
3_					-					FILL/REWORKED
4					1	1				NATIVE
4_					1	1				
5										
-	S-2	24/10	5 to 7	9		Olive gray Claye	y SILT, little Sa	nd and fine to medium	PP = 4,000 psf to	
6_				12		Gravel, humid, s			6,000 psf	
				11		1				
7_				10		1				
8_										
9										
10										
_	S-3	26/16	10 to 12	3		same as above			PP = 3,000 psf	
11_				5						
10				4						
12_				5						
13										
14					125.5'					
_										
15_				_						GLACIAL TILL
1,	S-4	24/12	15 to 17	8			,	trace Gravel, very		
16_				13 12		dense, humid to	moist, ivil			
17				9	1	1				
''-				,	1	1				
18						1				
						1				
19_						1				
20					-	1				
20_	S-5	24/24	20 to 22	8		same as above,	wet		<u> </u>	
21	3 3	24/24	20 10 22	11		same as above,	WCt			
				11	1	1			PP = 5,000 psf	
22_				13					to 9,000 psf	
						1				
Granula		Cohesiv		% Comp		NOTES:		netrometer, MC = Moisture		Soil Moisture Condition
Blows/ft.			Consistency	ASTM D	2487	Podrock loints	•	t, PI = Plastic Index, FV =		Dry: $S = 0\%$ Humid: $S = 1 \text{ to } 25\%$
0-4 5-10	V. Loose	e <2 2-4	V. soft Soft	< 5%]	Frace	Bedrock Joints Shallow = 0 to 35		Shear Strength, $Su(r) = Re$	emolueu Shear Strength	Damp: S = 26 to 50%
	Compac		Firm	5-15%		Dipping = 35 to 55	=			Moist: $S = 51 \text{ to } 75\%$
31-50	Dense	9-15	Stiff	15-30%		Steep = 55 to 90	_			Wet: $S = 76 \text{ to } 99\%$
>50	V. Dens		V. Stiff	> 30%	With	1				Saturated: S = 100%
		>30	Hard					Cobbles = diameter < 12 in		
						Gravel = < 3 inch	and > No 4, San	$d = \langle No 4 \text{ and } \rangle No 200,$	Silt/Clay = < No 200	

		<u> </u>				S	OIL BORII	NG LOG	Boring #:	B-2		
		SUMMI	I X			Project: New Apartment Building			Project #:	16030		
	G	EOENGINEERING SER	RVICES			Location:	153 - 165 Sher	•	Sheet:	2 of 2		
						City, State:	Portland, Main	е	Chkd by:			
Drilling (Orilling Co: Northern Test Boring					Boring Elevation: 139.5 ft. +/-						
Driller:				Reference:	tcomb Dated 3/4/16							
Summit		M. Hardison, E				Date started:	3/10/2016	Date Completed:	3/10/2016			
		METHOD		AMPLER			1	ESTIMATED GROUND				
Vehicle:		ATV	Length:	24" SS		Date	Depth	Elevation		ference		
Model: Method:		Diedrich D-50 2.25" ID H.S.A.		2"OD/1.5" 140 lb	'ID	3/10/2016	20.0 ft.	119.5 ft. +/-	observed on sample	es		
Hammer		Auto	Method:	ASTM D15	586							
Depth	Otylo.	Auto	wethou.	NOTHI DIC	Elev.		SAMPL	F	Geological/	Geological		
(ft.)	No.	Pen/Rec (in)	Depth (ft)	blows/6"	(ft.)		DESCRIP		Test Data	Stratum		
(-)		,	31.1. (3)		(- /							
23												
						Dense drilling st	arting at 23'			GLACIAL TILL		
24_												
25												
25_	S-6	24/24	25 to 27	23		Gray SILT 1841a	Sand and Clav	trace Gravel, very				
26	3-0	24/24	20 10 27	23		dense, humid to	-	, nace Graver, very				
				27	1	asiiso, ridinid to	oist, IVIL					
27				28								
28_		1		ļ								
		-										
29_		-										
30		1										
	S-7	24/24	30 to 32	20		same as above						
31				23								
_				25								
32_				27	107.5'							
						End of Boring at	32.0', no refus	al				
33_												
24												
34_												
35												
_												
36												
37_												
20		1										
38_		1										
39					1							
l -]							
40_												
		<u> </u>										
41_		-										
42		-										
42_		1										
43					1							
_]							
44_												
Granula		Cohesiv		% Comp		NOTES:		netrometer, MC = Moistur		Soil Moisture Condition		
Blows/ft. 0-4			Consistency	ASTM D	12487	Podrock loints	•	t, PI = Plastic Index, FV =		Dry: S = 0%		
0-4 5-10	V. Loose		V. soft Soft	< 5% 1	Frace	Bedrock Joints Shallow = 0 to 35		Shear Strength, $Su(r) = F$	vernoided Strength	Humid: $S = 1 \text{ to } 25\%$ Damp: $S = 26 \text{ to } 50\%$		
	Compa		Firm	5-15%		Dipping = 35 to 5	•			Moist: S = 51 to 75%		
31-50	Dense		Stiff	15-30%		Steep = 55 to 90	-			Wet: S = 76 to 99%		
	V. Dens		V. Stiff	> 30%			-			Saturated: S = 100%		
		>30	Hard					Cobbles = diameter < 12				
						Gravel = < 3 inch and > No 4, Sand = < No 4 and >No 200, Silt/Clay = < No 200						

		\sim	_			S	OIL BORII	NG LOG	Boring #:	B-3
	,	SUMMI	<u>X</u>			Project:	New Apartmer	nt Building	Project #:	16030
	GEO	DENGINEERING SERV	TICES			Location:	153 - 165 Sher		Sheet:	1 of 1
						City, State:	Portland, Main		Chkd by:	
	Drilling Co: Northern Test Boring			Boring Elevation: 114.5 ft. +/-						
Driller:				Reference:		onditions Survey, 153-1		tcomb Dated 3/4/16		
Summit		M. Hardison, E		AMBLES		Date started:	3/10/2016	Date Completed:	3/10/2016	
	ILLING	METHOD		AMPLER		Dete	Da H.	ESTIMATED GROUND		f
Vehicle: Model:		ATV Diedrich D-50	Length:	24" SS 2"OD/1.5"	חוי	Date 3/10/2016	Depth	Elevation	none encountered	ference
Method:		2.25" ID H.S.A.		2 OD/1.5	טו	3/10/2010	-	-	none encountered	
Hammer		Auto	Method:	ASTM D15	586					
Depth					Elev.		SAMPL	.E	Geological/	Geological
(ft.)	No.	Pen/Rec (in)	Depth (ft)	blows/6"	(ft.)		DESCRIP	TION	Test Data	Stratum
	S-1	24/10	0 to 2	3		Dark brown - bla	ack Silty SAND,	few glass pieces, little		
1_				2		to trace Clay, ro	otlets, moist, lo	ose, SM		FILL
_		<u> </u>		3						
2_				3						
3				 						
4										
-										
5_										
	S-2	24/1	5 to 7	7		no recovery, Sai	ndy SILT in tip			
6_		-		15						
7				10 10						
7_				10						
8					106.5'					
_					. 55.5					
9										GLACIAL TILL
10_										
	S-3	24/24	10 to 12	10			T, little Gravel a	and Sand, humid,	PP > 9,000 psf	
11_				15		very stiff, ML				
12				15 18	102.5'					
'2_				10	102.5	End of Explorati	on at 12.0 no r	efusal		
13						Ziid oi Zipiorati	0.1 41 12.07 110 1	0.404.		
_										
14_										
15_										
14		1								
16_				<u> </u>						
17										
· · · -										
18										
19_		1		ļ						
20		-		-						
20_		-								
21				1						
22		1								
_										
Granula		Cohesiv		% Comp		NOTES:		netrometer, MC = Moisture		Soil Moisture Condition
Blows/ft.	-		Consistency	ASTM D	2487		-	t, PI = Plastic Index, FV =		Dry: S = 0%
	V. Loose		V. soft	, 50/ 3	Franc	Bedrock Joints		Shear Strength, $Su(r) = R$	emolded Shear Strength	Humid: S = 1 to 25%
5-10 11-30	Loose Compac		Soft Firm	< 5% 1 5-15%		Shallow = 0 to 35 Dipping = 35 to 5	•			Damp: S = 26 to 50% Moist: S = 51 to 75%
31-50	Dense		Stiff	15-30%		Steep = 55 to 90	-			Wet: S = 76 to 99%
	V. Dens		V. Stiff	> 30%		33.070				Saturated: S = 100%
		>30	Hard			Boulders = diame	ter > 12 inches, (Cobbles = diameter < 12 i	nches and > 3 inches	
				1		Gravel = < 3 inch	and > No 4, San	d = < No 4 and >No 200,	Silt/Clay = < No 200	





GRAIN SIZE ANALYSIS - ASTM D422

PROJECT NAME: New Apartment Building PROJECT NUMBER: 16030

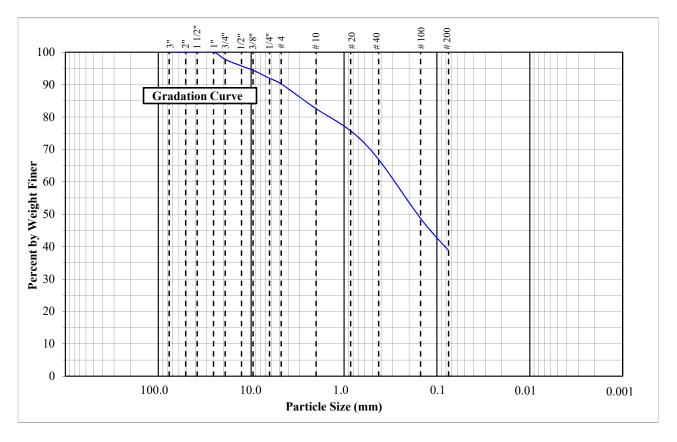
CLIENT: BD Sheridan, LLC SAMPLE NUMBER: B-2, S-2 & S-3

SOURCE: Boring B-2, 5'-12' DESCRIPTION: SAND, some Silt & Clay, little Gravel, SM-SC

DATE: 3/25/2016 TECHNICIAN: Erika Stewart, E.I.

DATA

PARTICLI	E SIZE mm	% BY WT FINER
76.20	(3 in)	100.0
50.80	(2 in)	100.0
38.10	(1-1/2 in)	100.0
25.40	(1 in)	100.0
19.05	(3/4 in)	97.8
12.70	(1/2 in)	95.7
9.53	(3/8 in)	94.4
6.35	(1/4 in)	92.0
4.75	(No. 4)	90.2
2.00	(No. 10)	82.6
0.85	(No. 20)	75.7
0.43	(No. 40)	66.9
0.15	(No. 100)	48.7
0.075	(No. 200)	38.8



REMARKS: Moisture Content = 11.4%



ATTERBERG LIMIT TEST - ASTM D4318

Method "A" (Multi-point)

PROJECT NAME: New Apartment Building PROJECT NUMBER: 16030

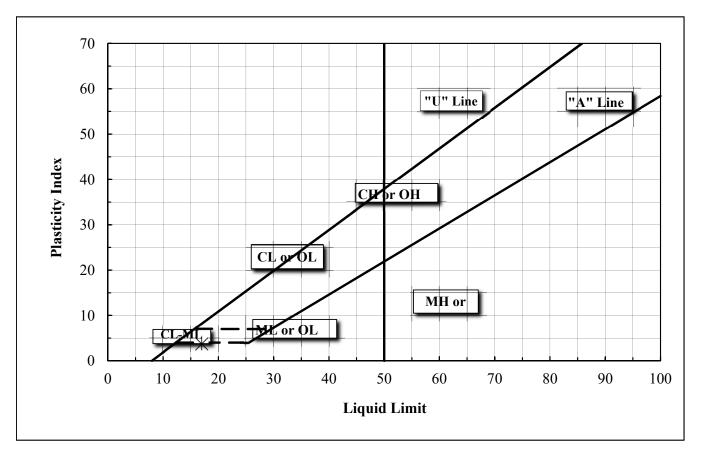
CLIENT: BD Sheridan, LLC SAMPLE NUMBER: B-2, S-2 & S-3

SOURCE: Fill / Reworked Native DEPTH: 5'-12'

DATE: 3/28/2016 TECHNICIAN: Erika Stewart, E.I.

DATA

Source	Depth	LL	PL	PI	Classification
B-2	5'-12'	17	13	4	Gray SAND, some Silt & Clay, little Gravel, SM-SC



Notes:

Sample was screened on the #40 sieve to remove gravel and med-coarse sand before performing Atterberg Limit test.

