HABITEC 2000®

, EXP. : JUNE 30, 2004

Les Habitations techniques Ltée
Technical Habitation Ltd
255, rue du Parc, Ville St-Joseph de Beauce

STATE APPROVAL STAMP

P.F.S. CORPORATION
401, Market Street
Bloomsburg, Pennsylvania
USA 17815

THIRD PARTY INSPECTION AGENCY MAINE

TEMPORARY CELLING FOR TRANSPORTATION FOR FIRST FLOOR

USE GROUP		: R3
CONSTRUCTION TYPE		: 5B
FLOOR AREA	BASEMENT FLOOR	: 1330 SQ. FT.
	FIRST FLOOR	: 1330 SQ. FT.
	SECOND FLOOR	: 1330 SQ. FT.
	THIRD FLOOR	: 1400 SQ. FT.
VOLUME OF INCLOSED SPACE		: 10 640 CU. FT. (APP 1) , 10640 CU. FT. (APP #2), 11200 CU. FT. (APP #3)
STORY ABOVE FOUNDATION		: 3 STORIES
BUILDING HEIGHT ABOVE FOUNDATION		: 39'-8 1/4"
DESIGN OCCUPENCY LOAD PER FLOOR		: 6 FOR APP #1 : 6 FOR APP #2 : 7 FOR APP #3 (1 x 200 S.Q.)
FIRE ALARM SYSTEM		: SMOKE DETECTORS
FIRE SUPPRESSION SYSTEM		: N/A
OTHER		: N/A
DESIGN LIVE LOAD PSF	WALL	: 25
	FLOOR	: NON SLEEPING 40
		: SLEEPING 30
	ROOF	: 52.5psf. (SNOW LOAD 60psf. AND WIND LOAD 90mph.)
	CORRIDOR	: N/A
	STAIRS	: 40
	BALCONIES	: N/A
	PORCH	: N/A
TYPE OF HEATING SYSTEM		: HOT WATER BASEBOARD
VENTING SYSTEMS		: RANGE HOOD AND BATH FAN TO BE EXHAUSTED : TO EXTERIOR

EXTERIOR ENVELOPE THERMAL PERFORMANCE	INSTALLED ALL MATERIALS	INSTALLED INSULATION ONLY
WALL	: SEE MECCHECK	
FLOOR	: SEE MECCHECK	
FLOOR CANTILEVER	: SEE MECCHECK	
ROOF AND CEILING	: SEE MECCHECK	
DOORS	: SEE MECCHECK	
THERMOS WINDOW	: SEE MECCHECK	
FOUNDATION	: ON SITE : BY CLIENT	

NOTE: THE BUILDING SHALL BE SET BACK GREATER THAN 5' FOR A NON-RATED EXTERIOR WALL AND THE BUILDING SHALL NOT BE LOCATED WITHIN THE FIRE LIMITS.

LABEL LOCATION DATA PLATE +STATE LABEL+PFS LABELS+WARRANTY SEALS:		
	FIRST FLOOR #1	UNDER KITCHEN SINK
	SECOND FLOOR #2	UNDER KITCHEN SINK (APP #2)
	SECOND FLOOR #2	UNDER KITCHEN SINK (APP #3)

NOTE: THIS COVER SHEET AND APPROVAL DOES NOT INCLUDE SITE CONSTRUCTION

MAINE CODES
1993 BOCA NATIONAL BUILDING CODE WITH AMENDMENTS
1993 BOCA NATIONAL PLUMBING CODE
1993 CABO MODEL ENERGY CODE
1999 NATIONAL ELECTRIC CODE
1997 NFPA 31 (MECHANICAL) – EFFECTIVE 4/1/01
2000 NFPA 101 (LIFE SAFETY CODE)

R.A. OR P.E. APPROVAL STAMP

SITE LOCATION
...
PORTLAND

DRAWING INDEX

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3	FRONT ELEVATION	SEPTEMBER 2002	
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7	FIRST FLOOR PLAN	SEPTEMBER 2002	
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16A TO 16I	BEAM EACH FLOOR + LVL CALCUL	SEPTEMBER 2002	
17A TO 17D	TRUSS DETAILS	SEPTEMBER 2002	
18A, 18B, 18C	PLUMBING EACH FLOOR	SEPTEMBER 2002	
19	PLUMBING PROFILE	SEPTEMBER 2002	
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21	ELECTRICAL (PANEL BOX)	SEPTEMBER 2002	
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25A TO 25G	WINDOWS AND DOORS DETAILS	SEPTEMBER 2002	
26A, 26B	DETAILS	SEPTEMBER 2002	
27	CROSS SECTION (MARRIED WALL)	SEPTEMBER 2002	
28	GRAB BARS DETAILS	SEPTEMBER 2002	
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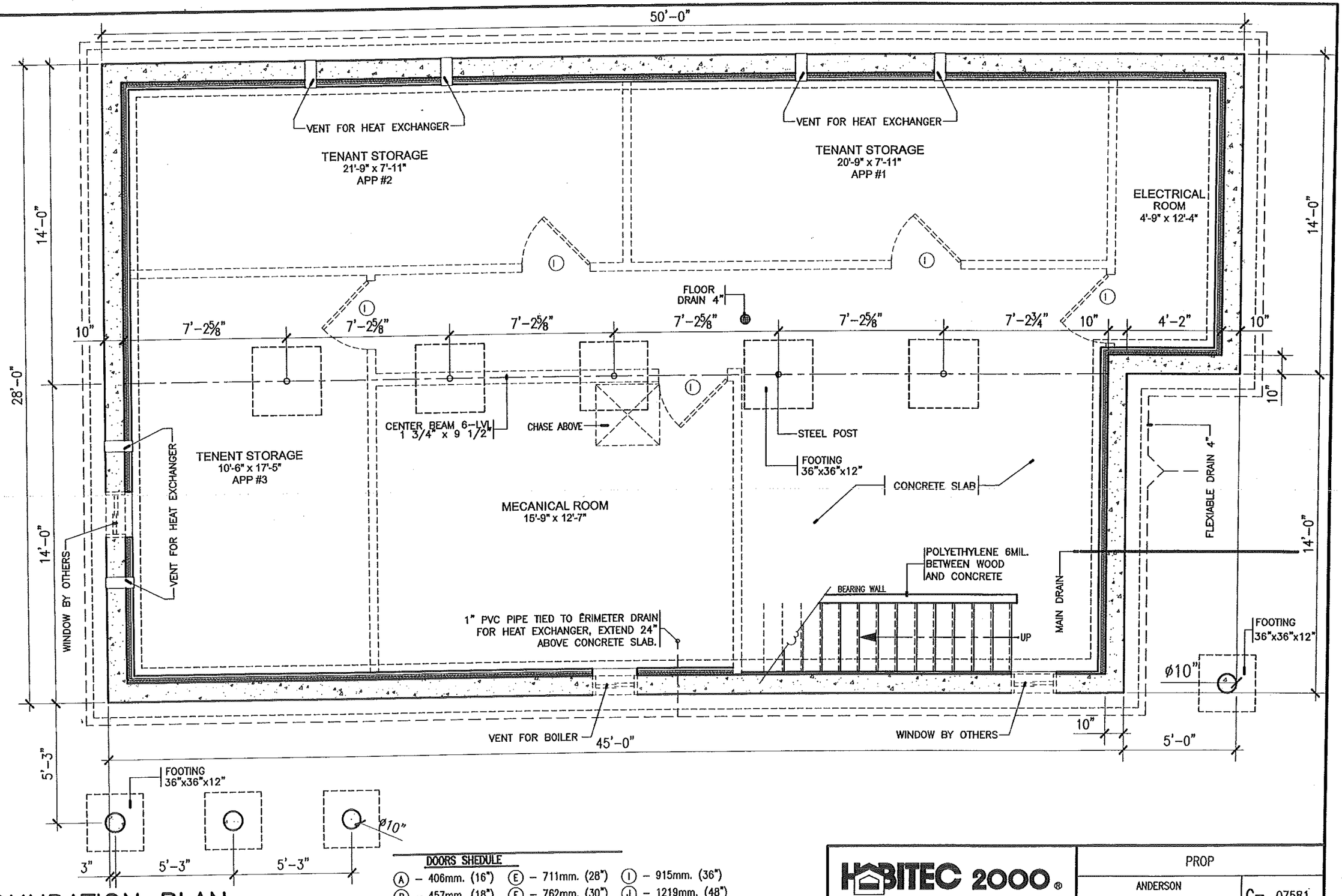
NO SCALE	S.B.		09/09/2002	COVER PAGE	1



FOUNDATION PLAN

DOORS SHEDULE

(A) – 406mm. (16")	(E) – 711mm. (28")	(I) – 915mm. (36")
(B) – 457mm. (18")	(F) – 762mm. (30")	(J) – 1219mm. (48")
(C) – 508mm. (20")	(G) – 813mm. (32")	(K) – 1524mm. (60")
(D) – 610mm. (24")	(H) – 864mm. (34")	(L) – 1829mm. (72")

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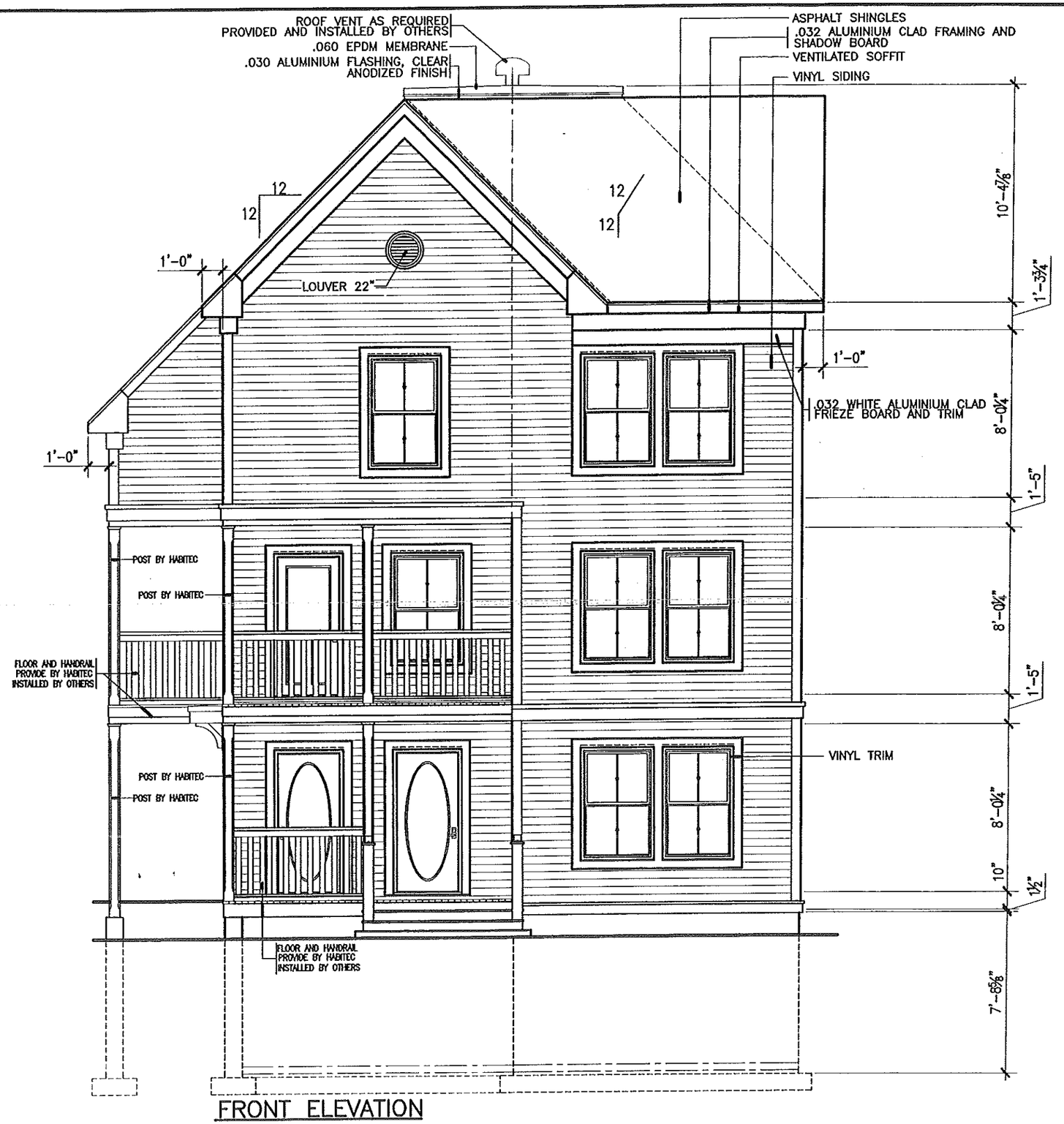
Scale:	Dr. by:	App. by:	Date:	Plan:	Page
1/4"=1'-0"	S.B.		08/27/2002	PLAN FOUNDATION	2

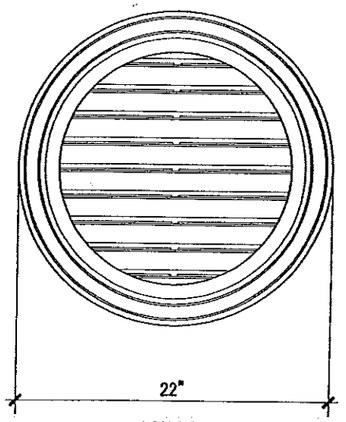


FRONT ELEVATION

NOTICE

THE CONTRACTOR SHOULD CARREFULLY VERIFY THE MEASUREMENTS AND DETAILS OF THE PLAN AND ADVISE "LES HABITATIONS TECHNIQUES LTEE" OF ALL ERRORS AND LACK OF DETAILS BEFORE THE PRODUCTION OF HOUSES.

ONLY "LES HABITATIONS TECHNIQUES LTEE." HAS THE RIGHTS TO USE THESE PLANS FOR THE CONSTRUCTION OF HOUSES.



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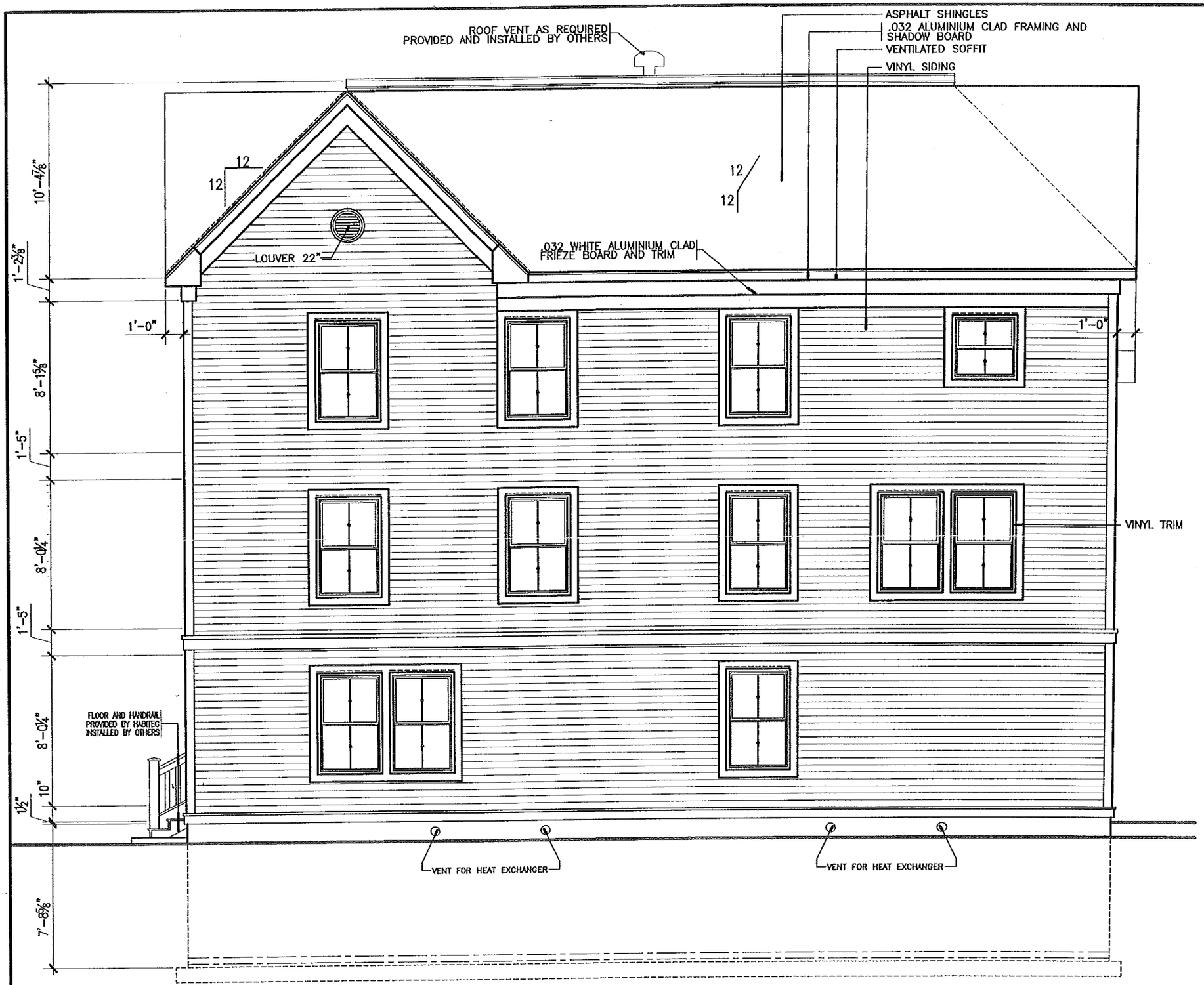

RIGHT ELEVATION

NOTICE

THE CONTRACTOR SHOULD CARREFULLY VERIFY THE MEASUREMENTS AND DETAILS OF THE PLAN AND ADVISE "LES HABITATIONS TECHNIQUES LTEE". OF ALL ERRORS AND LACK OF DETAILS BEFORE THE PRODUCTION OF HOUSES.

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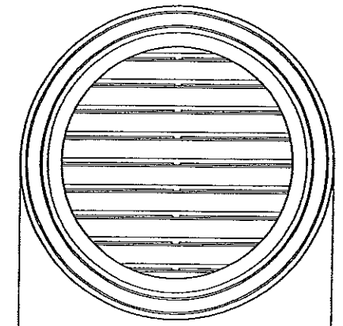

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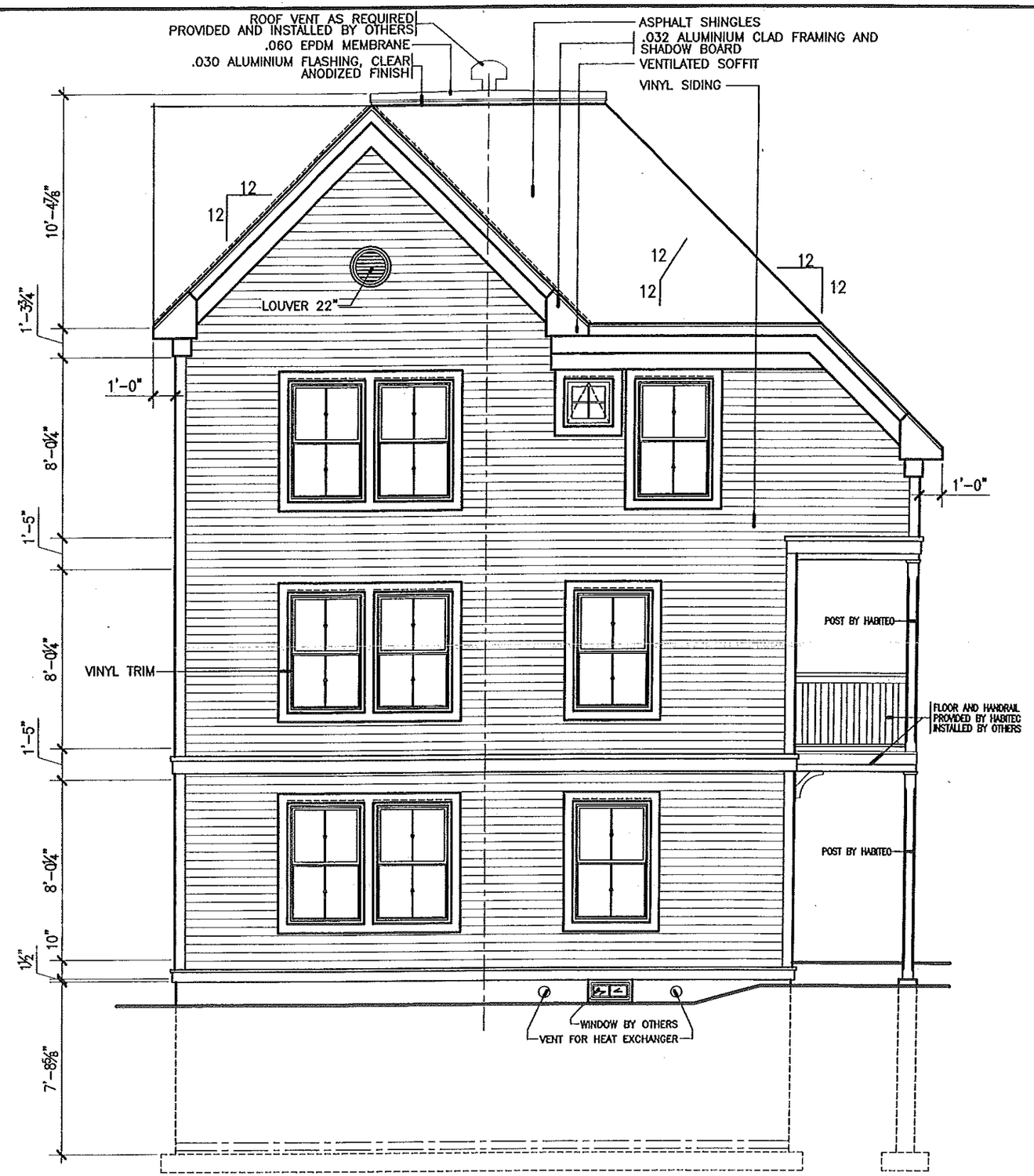
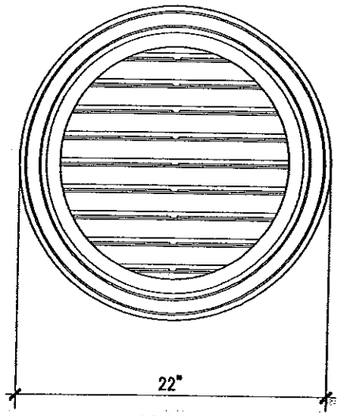


REAR ELEVATION

NOTICE



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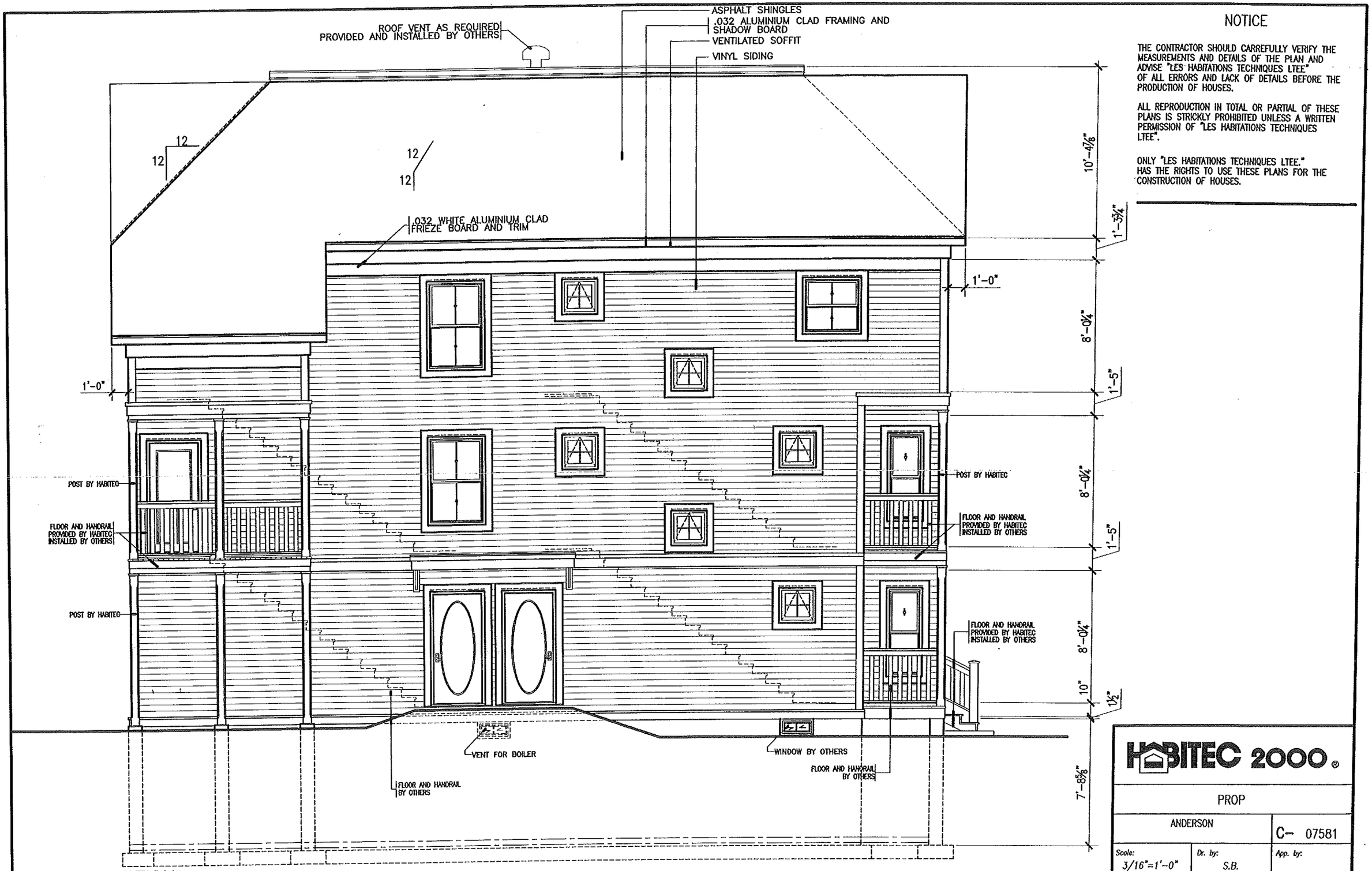

LEFT ELEVATION

NOTICE

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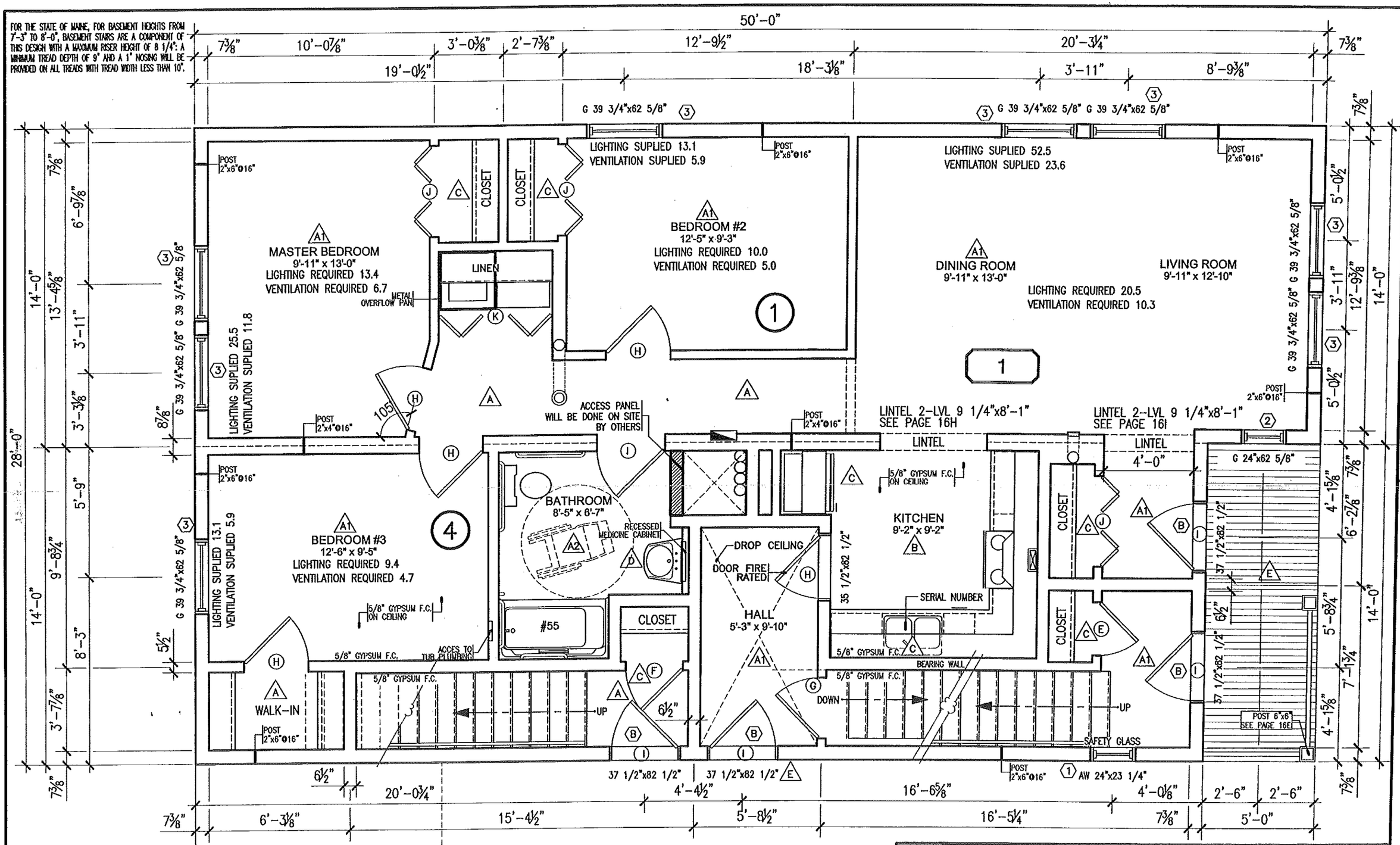

SCHEDULE FIXTURES
A - ARTCRAFT AC2128 PB
A1 - ARTCRAFT AC2129 PB
A2 - ARTCRAFT AC 2129 WH
B - PROGRESS P737530 WHITE
C - PROGRESS P7007-30 24"
D - PROGRESS P7172-30 24"
E - ARTCRAFT AC5661 BLACK

DOORS SHEDULE

(A) - 406mm. (16")	(E) - 711mm. (28")	(I) - 915mm. (36")
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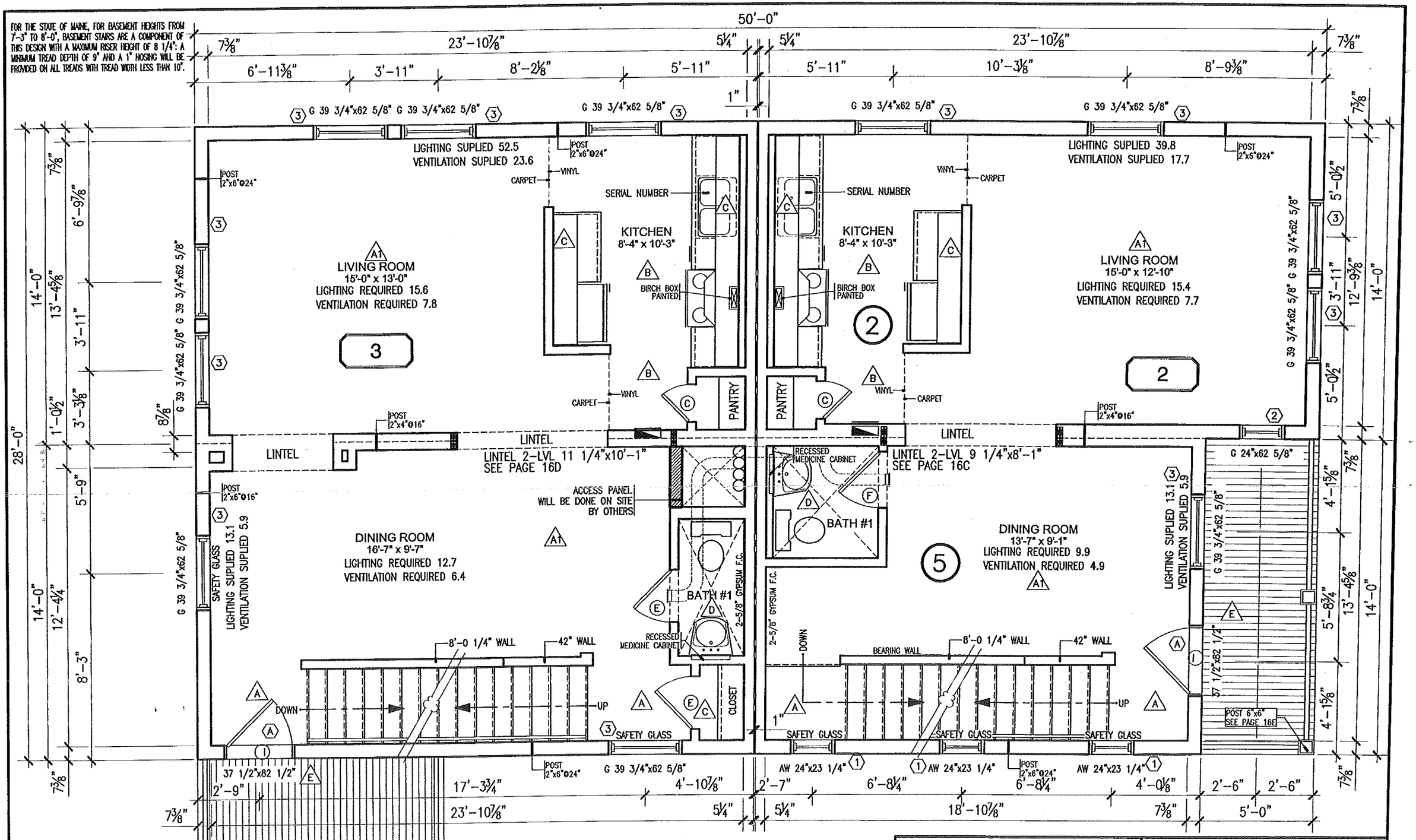
Scale:	Dr. by:	App. by:	Date:	Plan:	Page
1/4"=1'-0"	S.B.		08/27/2002	FIRST FLOOR	7



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Scale:	Dr. by:	App. by:	Date:	Plan:	Page
1/4"=1'-0"	S.B.		08/27/2002	SECOND FLOOR	8

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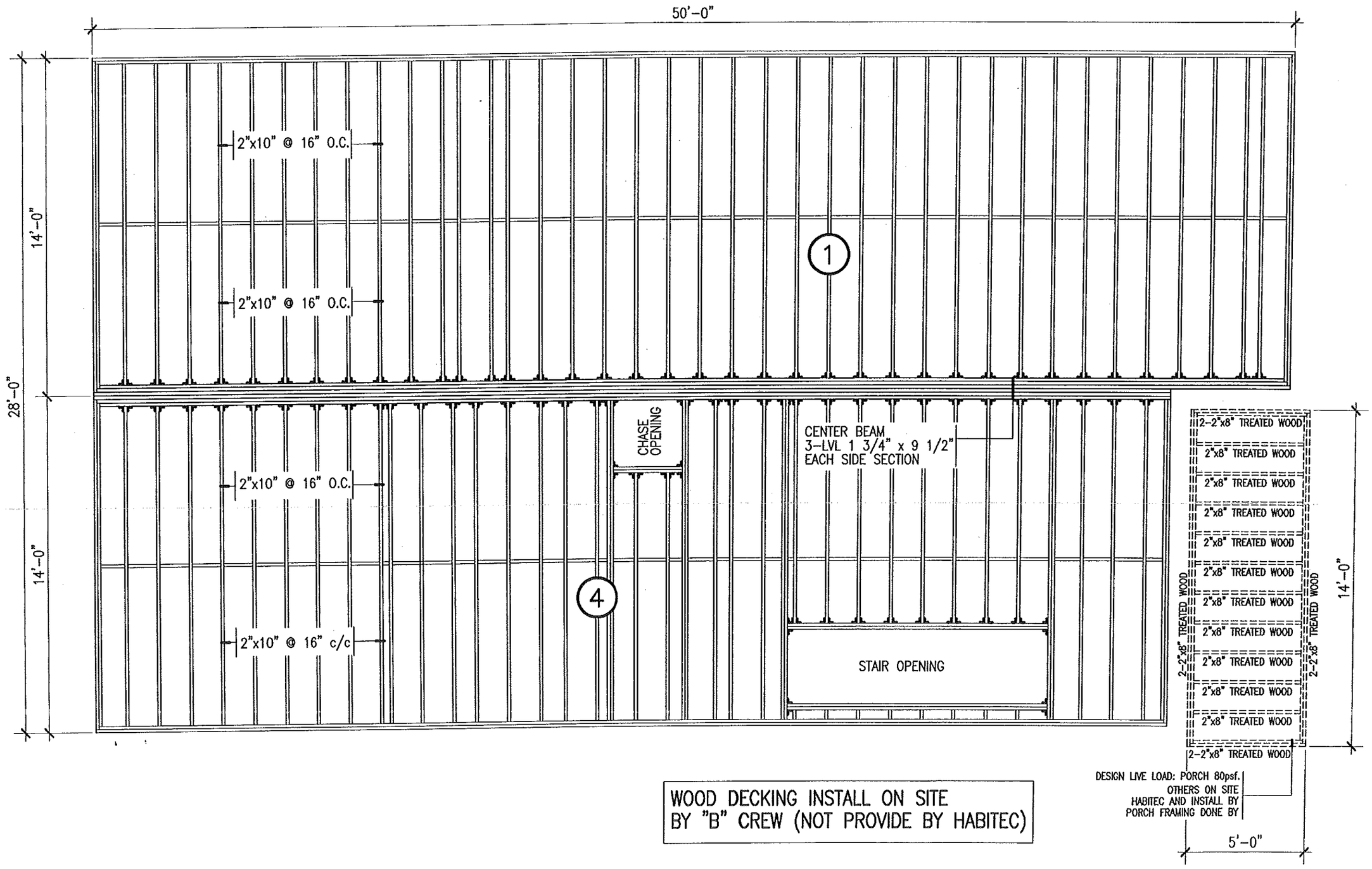

WOOD DECKING INSTALL ON SITE
BY "B" CREW (NOT PROVIDE BY HABITEC)

FLOOR FRAMING PLAN
FIRST FLOOR

JH JOIST HANGER

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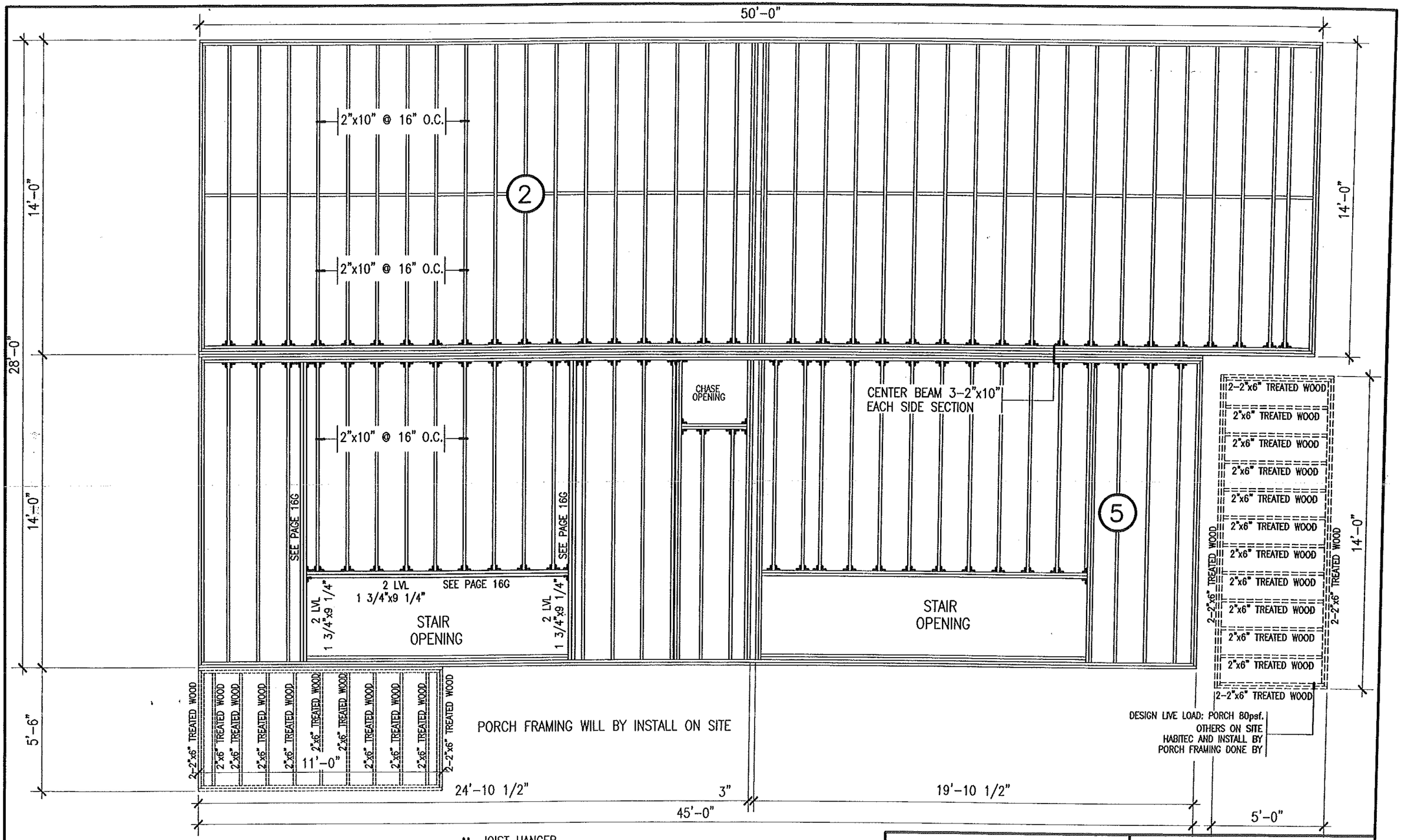
1/4"=1'-0"	S.B.		09/06/2002	FRAMING FIRST FL.	10



JL JOIST HANGER

WOOD DECKING INSTALL ON SITE
BY "B" CREW (NOT PROVIDE BY HABITEC)

FLOOR FRAMING PLAN
SECOND FLOOR

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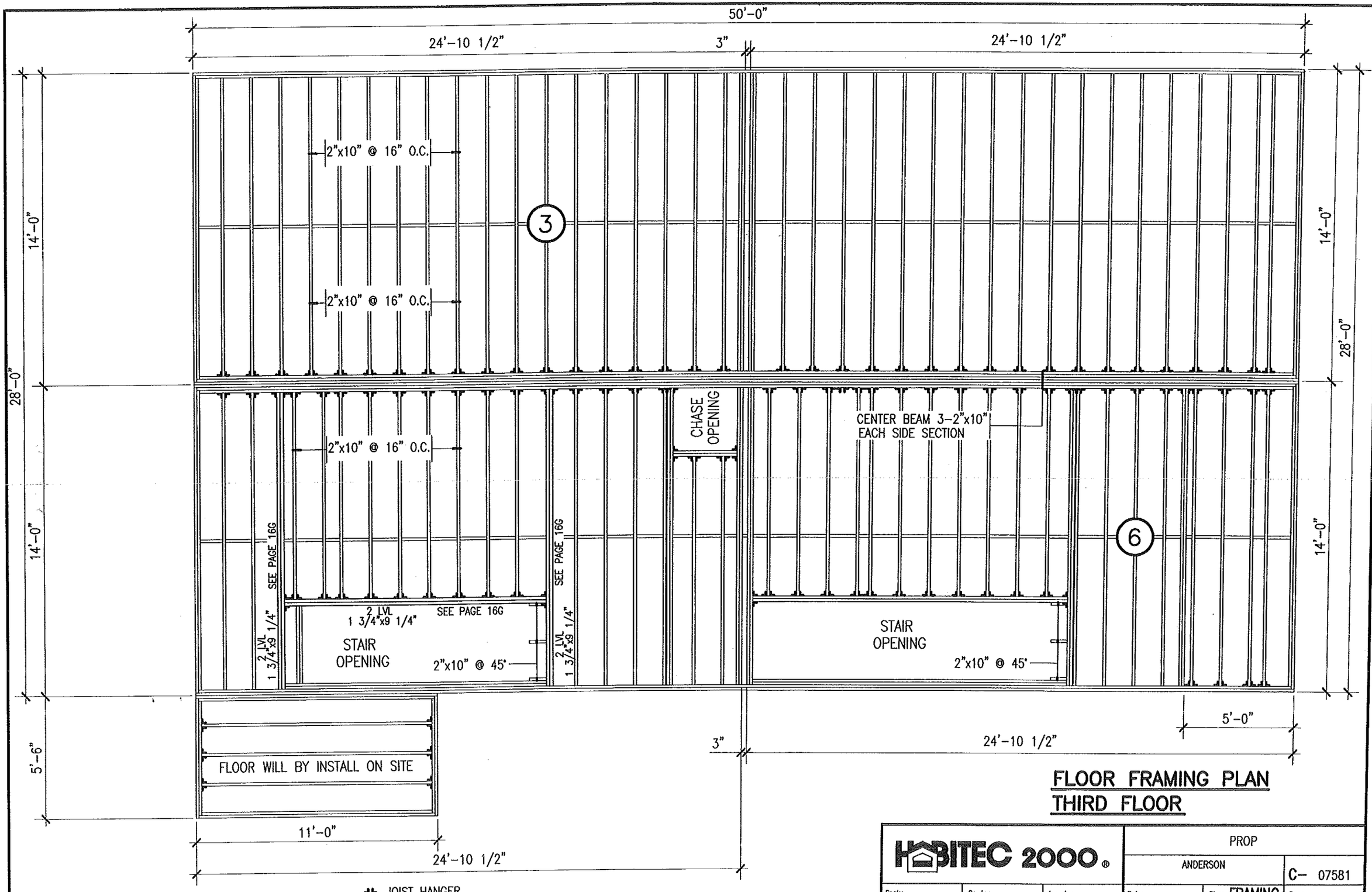
1/4"=1'-0"	S.B.		09/06/2002	FRAMING SECOND FL.	11



FLOOR FRAMING PLAN
THIRD FLOOR

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WINDOWS SHEDULE

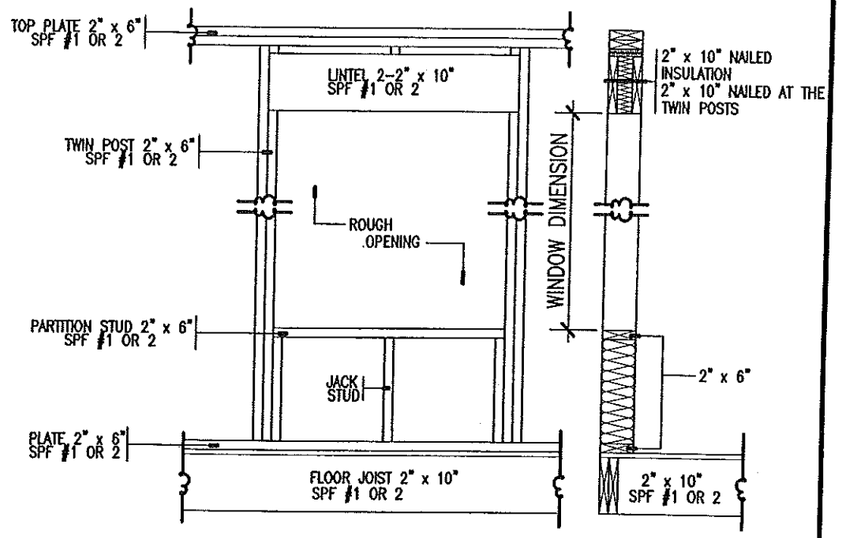
	OUTSIDE FRAME MEASUREMENTS WIDTH X HT	GLASS SIZE WIDTH X HT	SCREEN SIZE WIDTH X HT	OPENING (TO GO OUT) WIDTH X HT	PRODUCT ABRITEK
1	24" x 23 1/4"	2.63 SQ. FT.	2.88 SQ. FT.	2.25 SQ. FT.	AWING
2	24" x 62 5/8"	5.00 SQ. FT.	5.50 SQ. FT.	2.44 SQ. FT.	DOUBLE HUNG
3	39 3/4" x 62 5/8"	12.75 SQ. FT.	13.13 SQ. FT.	5.90 SQ. FT. *	DOUBLE HUNG
4	39 3/4" x 39"	7.38 SQ. FT.	7.63 SQ. FT.	3.15 SQ. FT.	DOUBLE HUNG
5					
6					
7					
8					
9					
10					
11					
12					
13					

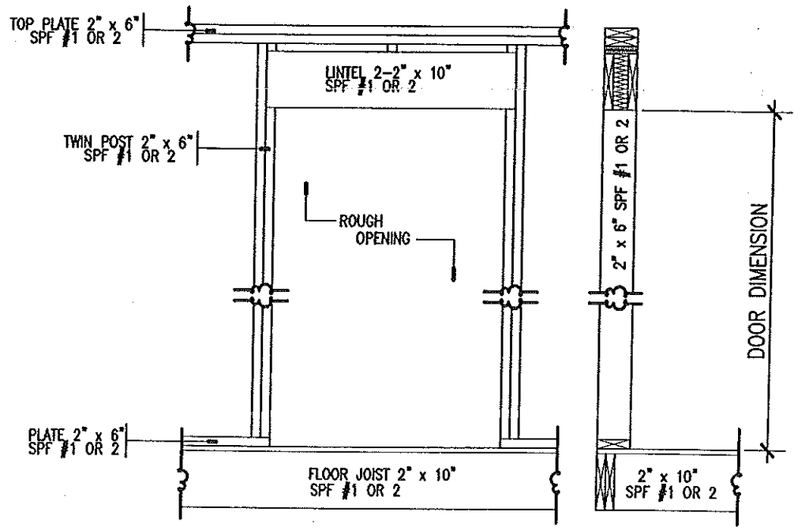
* EGRESS WINDOW

DOORS SHEDULE

	OUTSIDE FRAME MEASUREMENTS WIDTH X HT	GLASS SIZE WIDTH X HT	SCREEN SIZE WIDTH X HT	OPENING (TO GO OUT) WIDTH X HT	PRODUCT ABRITEK
A	37 1/2" x 82 1/2"	23" x 37"	----------	36" x 81"	STEEL DOOR
B	37 1/2" x 82 1/2"	23" x 65"	----------	36" x 81"	STEEL DOOR
C					
D					
E					

LINTEL DETAILS





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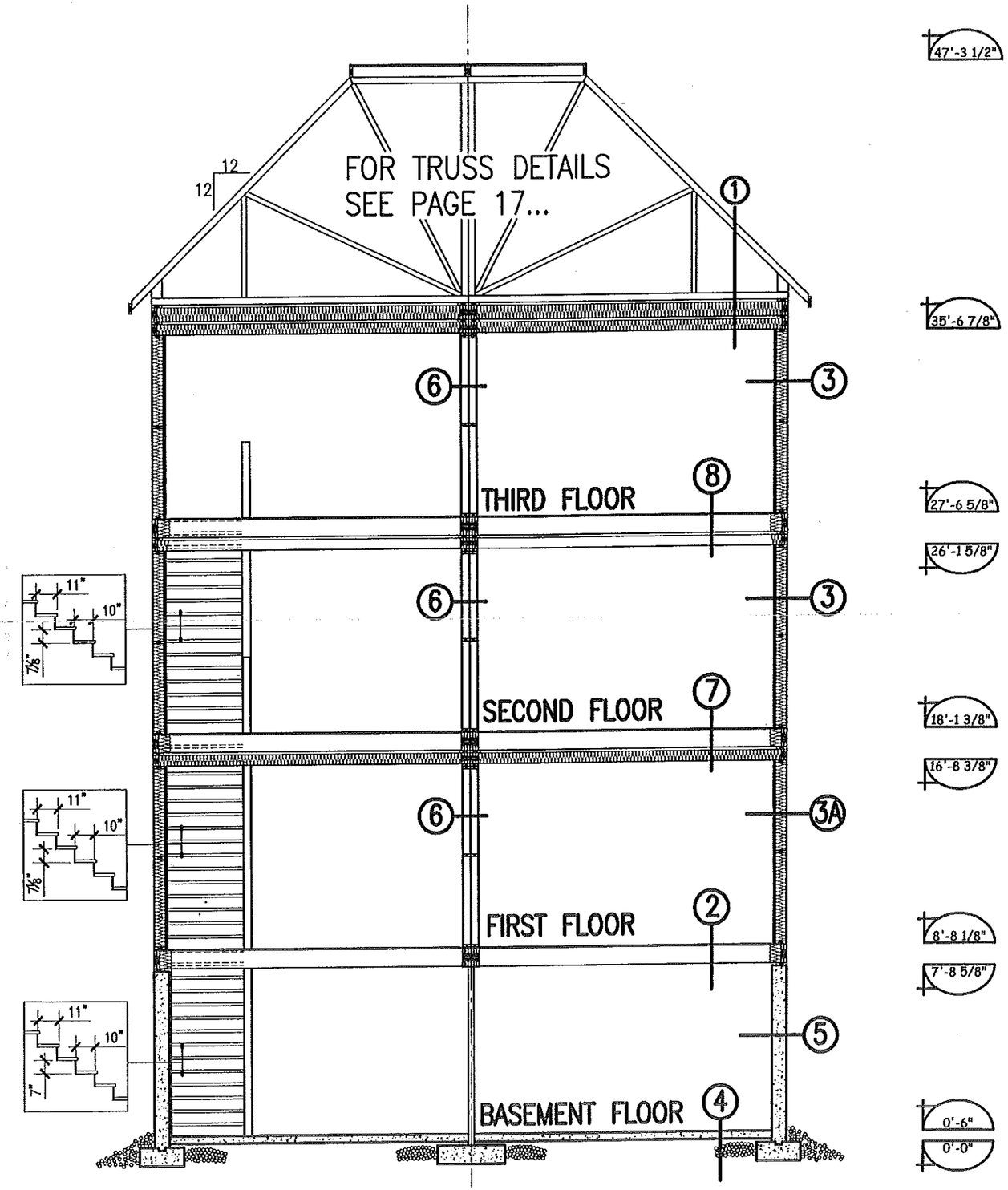

CROSS SECTION

1
-SHINGLES ON LOW SLOPE ROOFING
-BUIDING PAPER
-2 LAYERS OF ICE AND WATER SHIELD AT START OF ROOF (TRIPLE IN VALLEY)
-5/8" PLYWOOD SHEATING
-MINERAL WOOL (SEE MECCHECK)
-VAPOUR BARRIER #1
-1"x3" @ 16" O.C. S.P.F. #2
-5/8" GYPSUM WALL-BOARD
----R-37 --(ALL MATERIALS)

2
-FLOORING
-3/4" T/G SHEATING
-2"x10" JOITS @ 16" O.C. S.P.F. #2
-2 LAYERS GYPSUM 5/8" (BY OTHERS)

3
-VINYL SIDING
-VAPOUR BARRIER TYPE TYVEK
-1/2" PLYWOOD SHEATING
-2"x6" @ 24" O.C. S.P.F. #2
-MINERAL WOOL (SEE MECCHECK)
-VAPOUR BARRIER #1
-1"x2" @ 16" O.C. S.P.F. #2
-5/8" GYPSUM WALL-BOARD F.C.
----R-25 --(ALL MATERIALS)

3A
-VINYL SIDING
-VAPOUR BARRIER TYPE TYVEK
-1/2" PLYWOOD SHEATING
-2"x6" @ 16" O.C. S.P.F. #2
-MINERAL WOOL (SEE MECCHECK)
-VAPOUR BARRIER #1
-1"x2" @ 16" O.C. S.P.F. #2
-5/8" GYPSUM WALL-BOARD F.C.
----R-25 --(ALL MATERIALS)

4
-4" CONCRETE SLAB
-POLYTHENE .006"
-6" GRAVEL

5
-1/2" GYPSUM WALL-BOARD
-1"x3" FURRING
-TYPE 1 VAPOUR BARRIER
-2" RIGID INSULATION
-CONCRETE WALL

6
-5/8" GYPSUM WALL-BOARD F.C.
-2"x3" (OR 2"x4") @ 16" O.C. S.P.F. #2
-1/4" WAFERBOARD

7
-3/4" PLYWOOD SHEATING
-2"x10" JOITS @ 16" O.C. S.P.F. #2
-INSULBOARD
-2"x6" @ 24" O.C. S.P.F. #2
-MINERAL WOOL (SEE MECCHECK)
-1"x3" @ 16" O.C. S.P.F. #2
-5/8" GYPSUM WALL-BOARD F.C.

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3/16"=1'-0"	S.B.	
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HOUSE 2000	Scale: 1:12.5	Dr. by: S.B.	Appr. by:
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PREFABRICATED ROOF TRUSS APPROVED ACCORDING TO PLAN @ 24" O.C. OR LESS

FLOOR OF TRUSS

2'-0"

2'-7"

ROOF (R-42) ALL MATERIALS
NO. 210 GLUED ASPHALT BUILDING PAPER
5/8" PLYWOOD SHEATING
MINERAL WOOL R-40
VAPOUR BARRIER #1
1"x3" @ 16" O.C. S.P.F. #2
5/8" GYPSUM WALL-BOARD

ROOF SPACE VENTILATION 6"

2.2 KG/M2 CANTILEVER PROTECTION PAPER ASCENDING PITCH 3'-0" MIN. OR 1'-0" MIN. IN THE INNER SIDE OF THE OUTER WALL ICE SHIELD PROTECTION

FLOOR
FLOORING
3/4" PLYWOOD SHEATING
2"x10" JOISTS @ 16" O.C. S.P.F. #2
2"x6" @ 24" O.C. S.P.F. #2
INSULBOARD
MINERAL WOOL (SEE MECCHECK)
1"x3" @ 16" O.C. S.P.F. #2
5/8" GYPSUM WALL-BOARD

FLOOR
FLOORING
3/4" PLYWOOD SHEATING
2"x10" JOISTS @ 16" O.C. S.P.F. #2
2 LAYERS OF 5/8" GYPSUM F.C. (BY OTHERS)

WALL
INSULATION (R-10) BY OTHERS
CONCRETE WALL

4" DIAM. DRAIN

2"x6" SILL PLATE BY OTHERS
1/2" DIAM. ANCHOR BOLT @ 6'-0" O.C.

3 1/2" NAILING 16" O.C.

R-28 MINERAL WOOL
+ TYPE 1 VAPOR BARRIER BY BUILDER ON SITE

1-(2"x6") TOP PLATE OR 2-(2"x6") IF TRUSS SPACING AT 16" O.C.

2(2"x8") LINTEL OPENING +72"

2(2"x10") LINTEL OPENING -72"

1-(2"x6") TOP PLATE OR 2-(2"x6") IF TRUSS SPACING AT 16" O.C.

2(2"x8") LINTEL OPENING +72"

2"x6" 2"x8"

ALUMINIUM CLAD 2"x22"

ALUMINIUM CLAD 2"x22"

3/8" PLYWOOD

VENTILATED SOFFIT

2"x6" ALUMINIUM FASCIA

FLASHING

APPROVED WINDOW

CAULKING SPACE

5" VINYL TRIM

WALL (R-25) ALL MATERIALS
VINYL FACING
WINDSHIELD PAPER (TYVEK TYPE)
1/2" PLYWOOD SHEATING
2"x6" @ 24" O.C. S.P.F. #2
MINERAL WOOL R-20
VAPOUR BARRIER #1
1"x3" @ 16" O.C.
5/8" GYPSUM WALL-BOARD

2"x6"x1/2" METAL PLATE @ 4'-0" O.C. S.P.F. #2

2(2"x10") S.P.F. #2

5" VINYL TRIM

FLASHING

APPROVED WINDOW

CAULKING SPACE

WALL (R-25) ALL MATERIALS
VINYL FACING
WINDSHIELD PAPER (TYVEK TYPE)
1/2" PLYWOOD SHEATING
2"x6" @ 16" O.C. S.P.F. #2
MINERAL WOOL R-20
VAPOUR BARRIER #1
1"x3" @ 16" O.C.
5/8" GYPSUM WALL-BOARD

2"x6"x1/2" METAL PLATE @ 4'-0" O.C. S.P.F. #2
2(2"x10") S.P.F. #2
FLASHING

2 2"x6" S.P.F. #2

GROUND LEVEL

FOUNDATION PITCH

4" DIAM. DRAIN

FASTENING ASSEMBLIES AS HANGER MANUFACTURER RECOMANDATIONS

2"x6"x1/2" METAL PLATE @ 4'-0" O.C. S.P.F. #2

EXTERIOR FLOOR

EXTERIOR FLOOR

EXTERIOR WALL

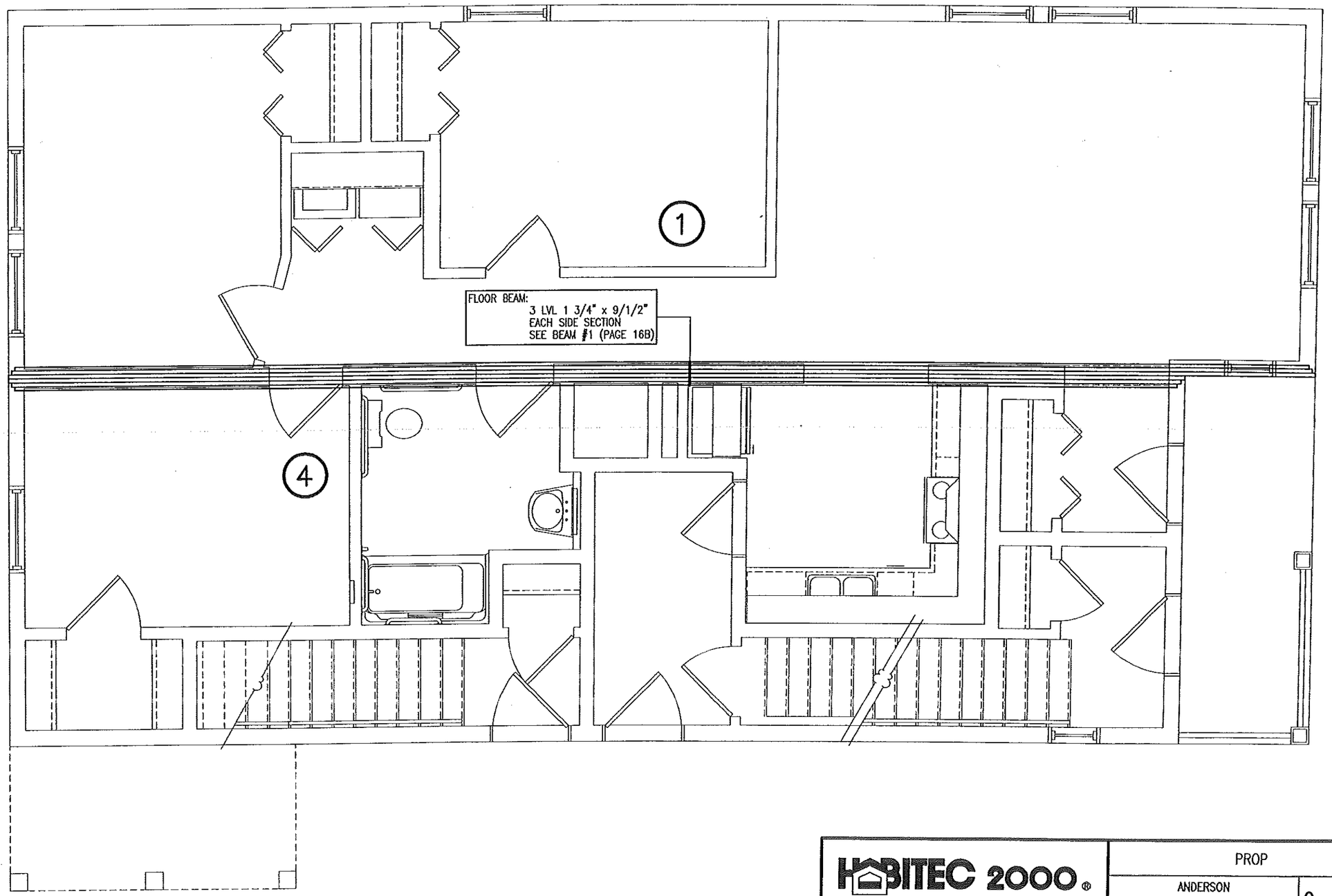
EXTERIOR WALL

4'-0"

TOP PLATE FASTENINGS

SIMPLE TOP PLATE

3" x 6" x 0.036" NAILED METAL PLATE



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ANDERSON

C-- 07581

Scale:	Dr. by:	App. by:	Date:	Plan:	Page
1/4"=1'-0"	S.B.		09/06/2002	BEAM FIRST FL.	16A

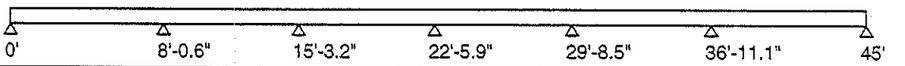
GOODLAM	COMPANY Goodlam Division Tel. 1-800-361-6503 Fax 1-450-635-3728 Apr. 16, 2002 16:56:06	PROJECT Beam1

Design Check Calculation Sheet

LOADS: (lbs, psf, or plf)

Load	Type	Distribution	Magnitude Start	Magnitude End	Location [ft] Start	Location [ft] End	Pattern Load?
1	Live	Full UDL	840				Yes
2	Dead	Full UDL	315				No
3	Live	Full UDL	368				Yes
4	Dead	Full UDL	119				No

MAXIMUM REACTIONS (lbs) and BEARING LENGTHS (in) :



Dead	1399		3827		2913		3243		2911		3835	1403
Live	4281		11357		10155		10553		10156		11378	4292
Total	5680		15184		13068		13796		13066		15212	5695
Bearing: Length	1.2		3.2		2.8		2.9		2.8		3.2	1.2

WELDWOOD LVL, 1 3/4" Wide, 2.0E, 1-3/4x9-1/2", 3-Plys

Load combinations: ASCE 7-95;

SECTION vs. DESIGN CODE NDS-1997: (lbs, lbs-ft, or in)

Criterion	Analysis Value	Design Value	Analysis/Design
Shear	V @d = 6749	Vr = 9476	V/Vr = 0.71
Bending(+)	M = 9878	Mr = 21012	M/Mr = 0.47
Bending(-)	M = 11502	Mr = 21012	M/Mr = 0.55
Live Defl'n	0.11 = L/882	0.20 = L/480	0.54
Total Defl'n	0.15 = L/633	0.40 = L/240	0.38

ADDITIONAL DATA:

FACTORS: F		CD	CM	Ct	CL	CF	CV	Cfu	Cr	LC#
Fb'+=	3100	1.00	1.00	1.00	1.000	1.03	1.000	1.00	1.00	44
Fb'-=	3100	1.00	1.00	1.00	1.000	1.03	1.000	1.00	1.00	55
Fv' =	285	1.00	1.00	1.00			(CH = 1.000)			54
Fcp'=	900		1.00	1.00						-
E' =	2.0	million	1.00	1.00						44

Bending(+): LC#44 = D+L (pattern: _L_L_L), M = 9878 lbs-ft
Bending(-): LC#55 = D+L (pattern: L_L_LL), M = 11502 lbs-ft
Shear : LC#54 = D+L (pattern: __L_LL), V = 8049, V@d = 6749 lbs
Deflection: LC#44 = D+L (pattern: _L_L_L) EI= 750.20e06 lb-in2/ply
Total Deflection = 1.50(Dead Load Deflection) + Live Load Deflection.
(D=dead L=live S=snow W=wind I=impact C=construction CLd=concentrated)
(All LC's are listed in the Analysis output)
(Load Pattern: s=S/2, X=L+S or L+C, _=no pattern load in this span)

DESIGN NOTES:

1. Please verify that the default deflection limits are appropriate for your application.
2. BEAMS require restraint against lateral displacement and rotation at points of bearing
3. SCL-BEAMS: Structural Composite Lumber design has assumed: - dry service conditions - full lateral support - no preservative or fire-retardant treatment
- no notches - single member use (no load sharing) - the specified dead load is no greater than 1/2 the specified live load
4. BUILT-UP SCL-BEAMS: contact manufacturer's LVL user guide for connection details

Scale: NONE | Dr. by: S.B. | App. by: | Date: 04/16/2002 | Phase: LVL CALCUL | ANDERSON | PROP | Page 16B | C- 07581

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May 8, 2002 15:29:22

PROJECT
C-07575

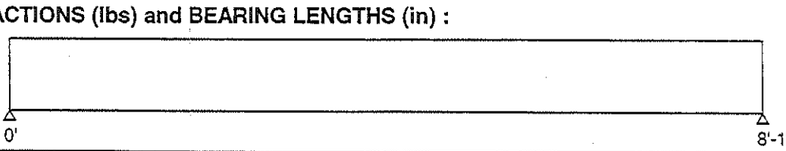
LIITEAU ET1-APP #2

Design Check Calculation Sheet

LOADS: (lbs, psf, or plf)

Load	Type	Distribution	Magnitude Start	Magnitude End	Location [ft] Start	Location [ft] End	Pattern Load?
1	Live	Full UDL	280				No
2	Dead	Full UDL	105				No
3	Dead	Partial UDL	60	60.00	4.00	8.08	No
4	Live	Partial UDL	368	368.00	4.00	8.08	No
5	Dead	Partial UDL	119	119.00	4.00	8.08	No
6	Live	Point	750		4.00		No
7	Dead	Point	243		4.00		No

MAXIMUM REACTIONS (lbs) and BEARING LENGTHS (in) :



Dead	732		1091
Live	1890		2626
Total	2622		3717
Bearing:			
Length	1.0		1.2

WELDWOOD LVL, 1 3/4" Wide, 2.0E, 1-3/4x9-1/4", 2-Plys

Load combinations: ASCE 7-95;

SECTION vs. DESIGN CODE NDS-1997: (lbs, lbs-ft, or in)

Criterion	Analysis Value	Design Value	Analysis/Design
Shear	V @d = 2998	Vr = 6151	V/Vr = 0.49
Bending(+)	M = 7411	Mr = 13281	M/Mr = 0.56
Live Defl'n	0.13 = L/757	0.20 = L/480	0.63
Total Defl'n	0.20 = L/474	0.40 = L/240	0.51

ADDITIONAL DATA:

FACTORS:	F	CD	CM	Ct	CL	CF	CV	Cfu	Cr	LC#
Fb'+=	3100	1.00	1.00	1.00	1.000	1.03	1.000	1.00	1.00	2
Fv' =	285	1.00	1.00	1.00			(CH = 1.000)			2
Fcp'=	900		1.00	1.00						-
E' =	2.0	million	1.00	1.00						2

Bending(+): LC# 2 = D+L, M = 7411 lbs-ft
Shear : LC# 2 = D+L, V = 3717, V@d = 2998 lbs
Deflection: LC# 2 = D+L EI= 461.68e06 lb-in2/ply
Total Deflection = 1.50(Dead Load Deflection) + Live Load Deflection.
(D=dead L=live S=snow W=wind I=impact C=construction CLd=concentrated)
(All LC's are listed in the Analysis output)

DESIGN NOTES:

1. Please verify that the default deflection limits are appropriate for your application.
2. BEAMS require restraint against lateral displacement and rotation at points of bearing
3. SCL-BEAMS: Structural Composite Lumber design has assumed: - dry service conditions - full lateral support - no preservative or fire-retardant treatment
- no notches - single member use (no load sharing) - the specified dead load is no greater than 1/2 the specified live load
4. BUILT-UP SCL-BEAMS: contact manufacturer's LVL user guide for connection details

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Scale: 1/4"=1'-0" | Dr. by: S.B. | App. by:

Date: 05/13/2002 | Plan: LVL CALCUL | ANDERSON | PROP

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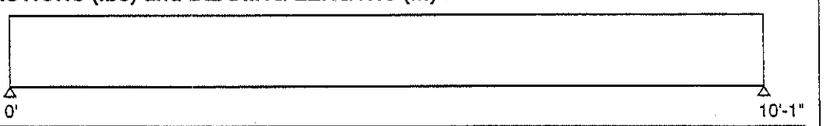
COMPANY	PROJECT
Goodlam Division Tel. 1-800-361-6503 Fax 1-450-635-3728	C-07575
May 8, 2002 15:10:02	LINTEAU ET1-APP #3

Design Check Calculation Sheet

LOADS: (lbs, psf, or plf)

Load	Type	Distribution	Magnitude Start	Magnitude End	Location [ft] Start	Location [ft] End	Pattern Load?
1	Live	Full UDL	280				No
2	Dead	Full UDL	105				No
3	Live	Partial UDL	368	368.00	3.42	10.08	No
4	Dead	Partial UDL	119	119.00	3.42	10.08	No
5	Live	Point	735		0.00		No
6	Dead	Point	238		0.00		No
7	Dead	Partial UDL	60	60.00	3.42	10.08	No

MAXIMUM REACTIONS (lbs) and BEARING LENGTHS (in) :



	0'	10'-1"
Dead	1162	1328
Live	2958	3054
Total	4120	4382
Bearing: Length	1.3	1.4

WELDWOOD LVL, 1 3/4" Wide, 2.0E, 1-3/4x11-1/4", 2-Plys

Load combinations: ASCE 7-95;

WARNING: point loads applied at support locations only affect maximum reactions and bearing lengths. The point loads have been added to the reactions without regard for load patterns.

SECTION vs. DESIGN CODE NDS-1997: (lbs, lbs-ft, or in)

Criterion	Analysis Value	Design Value	Analysis/Design
Shear	V @d = 3508	Vr = 7481	V/Vr = 0.47
Bending(+)	M = 10302	Mr = 19263	M/Mr = 0.53
Live Defl'n	0.16 = L/779	0.25 = L/480	0.62
Total Defl'n	0.26 = L/473	0.50 = L/240	0.51

ADDITIONAL DATA:

FACTORS: F	CD	CM	Ct	CL	CF	CV	Cfu	Cr	LC#
Fb'+= 3100	1.00	1.00	1.00	1.000	1.01	1.000	1.00	1.00	2
Fv' = 285	1.00	1.00	1.00			(CH = 1.000)			2
Fcp'= 900		1.00	1.00						-
E' = 2.0 million		1.00	1.00						2

```
Bending(+): LC# 2 = D+L, M = 10302 lbs-ft
Shear     : LC# 2 = D+L, V = 4382, V@d = 3508 lbs
Deflection: LC# 2 = D+L   EI= 830.57e06 lb-in2/ply
Total Deflection = 1.50(Dead Load Deflection) + Live Load Deflection.
  (D=dead  L=live  S=snow  W=wind  I=impact  C=construction  CLd=concentrated)
  (All LC's are listed in the Analysis output)
```

DESIGN NOTES:

1. Please verify that the default deflection limits are appropriate for your application.
2. BEAMS require restraint against lateral displacement and rotation at points of bearing
3. SCL-BEAMS: Structural Composite Lumber design has assumed: - dry service conditions - full lateral support - no preservative or fire-retardant treatment
- no notches - single member use (no load sharing) - the specified dead load is no greater than 1/2 the specified live load
4. BUILT-UP SCL-BEAMS: contact manufacturer's LVL user guide for connection details

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Date: 05/13/2002 | Plan: LVL CALCUL | ANDERSON | PROP

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May 8, 2002 09:58:47

PROJECT

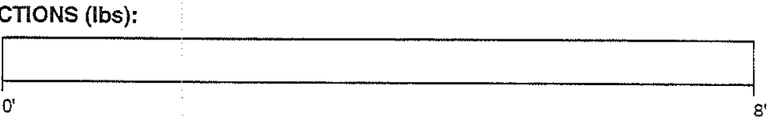
POTEAU DE GALERIE

Design Check Calculation Sheet

LOADS: (lbs, psf, or plf)

Load	Type	Distribution	Magnitude Start	Magnitude End	Location [ft] Start	Location [ft] End	Pattern Load?
1	Dead	Axial	1055	(Eccentricity = 0.00 in			
2	Live	Axial	1164	(Eccentricity = 0.00 in			

MAXIMUM REACTIONS (lbs):



Timber-soft, S. Pine, No.2 , 6x6"

Pinned base; Loadface = width(b); Ke x Lb: 1.00 x 8.00= 8.00 [ft]; Ke x Ld: 1.00 x 8.00= 8.00 [ft]; Load combinations: ASCE 7-95;

SECTION vs. DESIGN CODE NDS-1997: (stress=psi, and in)

Criterion	Analysis Value	Design Value	Analysis/Design
Axial	fc = 73	Fc' = 465	fc/Fc' = 0.16
Axial Bearing	fg = 73	Fg' = 1450	fg/Fg' = 0.05

ADDITIONAL DATA:

FACTORS: F	CD	CM	Ct	CL	CF	CV	Cfu	Cr	LC#
Fc' = 525	1.00	1.00	1.00		1.00	(Cp = 0.885)			2
E' = 1.2 million		1.00	1.00						0
Fg' = 1450	1.00		1.00						2

Axial : LC# 2 = D+L, P = 2219 lbs
(D=dead L=live S=snow W=wind I=impact C=construction CLd=concentrated)
(All LC's are listed in the Analysis output)

DESIGN NOTES:

1. Please verify that the default deflection limits are appropriate for your application.

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Scale: 1/4"=1'-0"
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May 11, 2002 12:51:15

PROJECT C-07575

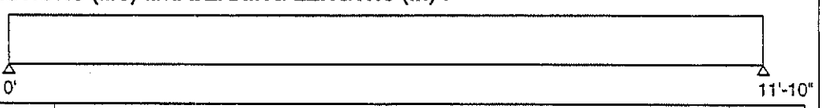
APP. # 3
POUTRE TROU ESC ET 1 - 2

Design Check Calculation Sheet

LOADS: (lbs, psf, or plf)

Load	Type	Distribution	Magnitude Start	Magnitude End	Location [ft] Start	Location [ft] End	Pattern Load?
1	Live	Full UDL	200				No
2	Dead	Full UDL	75				No
3	Dead	Partial UDL	60	60.00	0.00	8.83	No
4	Live	Full UDL	50				No
5	Dead	Full UDL	25				No

MAXIMUM REACTIONS (lbs) and BEARING LENGTHS (in) :



Dead	924		789
Live	1479		1479
Total	2403		2269
Bearing:			
Length	1.0		1.0

WELDWOOD LVL, 1 3/4" Wide, 2.0E, 1-3/4x9-1/4", 2-Plys

Load combinations: ASCE 7-95;

SECTION vs. DESIGN CODE NDS-1997: (lbs, lbs-ft, or in)

Criterion	Analysis Value	Design Value	Analysis/Design
Shear	V @d = 2087	Vr = 6151	V/Vr = 0.34
Bending(+)	M = 7042	Mr = 13281	M/Mr = 0.53
Live Defl'n	0.24 = L/594	0.30 = L/480	0.81
Total Defl'n	0.46 = L/311	0.59 = L/240	0.77

ADDITIONAL DATA:

FACTORS: F	CD	CM	Ct	CL	CF	CV	Cfu	Cr	LC#
Fb'+= 3100	1.00	1.00	1.00	1.000	1.03	1.000	1.00	1.00	2
Fv' = 285	1.00	1.00	1.00			(CH = 1.000)			2
Fcp'= 900		1.00	1.00						-
E' = 2.0 million		1.00	1.00						2

Bending(+): LC# 2 = D+L, M = 7042 lbs-ft
Shear : LC# 2 = D+L, V = 2403, V@d = 2087 lbs
Deflection: LC# 2 = D+L EI= 461.68e06 lb-in2/ply
Total Deflection = 1.50(Dead Load Deflection) + Live Load Deflection.
(D=dead L=live S=snow W=wind I=impact C=construction CLd=concentrated)
(All LC's are listed in the Analysis output)

DESIGN NOTES:

1. Please verify that the default deflection limits are appropriate for your application.
2. BEAMS require restraint against lateral displacement and rotation at points of bearing
3. SCL-BEAMS: Structural Composite Lumber design has assumed: - dry service conditions - full lateral support - no preservative or fire-retardant treatment
- no notches - single member use (no load sharing) - the specified dead load is no greater than 1/2 the specified live load
4. BUILT-UP SCL-BEAMS: contact manufacturer's LVL user guide for connection details

Scale: 1/4"=1'-0" | Dr. by: S.B. | App. by: | Date: 05/13/2002 | Plan: LVL CALCUL | Page 16F

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PROJECT C-07575

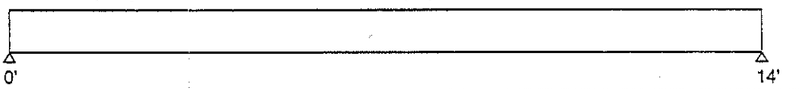
App. #3
CHEVÊTRE TROU ESC-ET 1~2

Design Check Calculation Sheet

LOADS: (lbs, psf, or plf)

Load	Type	Distribution	Magnitude Start	Magnitude End	Location [ft] Start	Location [ft] End	Pattern Load?
1	Live	Point	1480		4.00		No
2	Dead	Point	947		4.00		No

MAXIMUM REACTIONS (lbs) and BEARING LENGTHS (in) :



Dead	676		271
Live	1057		423
Total	1734		693
Bearing: Length	1.0		1.0

WELDWOOD LVL, 1 3/4" Wide, 2.0E, 1-3/4x9-1/4", 2-Plys

Load combinations: ASCE 7-95;

SECTION vs. DESIGN CODE NDS-1997: (lbs, lbs-ft, or in)

Criterion	Analysis Value	Design Value	Analysis/Design
Shear	V @d = 1734	Vr = 6151	V/Vr = 0.28
Bending(+)	M = 6934	Mr = 13281	M/Mr = 0.52
Live Defl'n	0.24 = L/694	0.35 = L/480	0.69
Total Defl'n	0.47 = L/354	0.70 = L/240	0.68

ADDITIONAL DATA:

FACTORS: F		CD	CM	Ct	CL	CF	CV	Cfu	Cr	LC#
Fb'+=	3100	1.00	1.00	1.00	1.000	1.03	1.000	1.00	1.00	2
Fv' =	285	1.00	1.00	1.00			(CH = 1.000)			2
Fcp'=	900		1.00	1.00						-
E' =	2.0	million	1.00	1.00						2

Bending(+): LC# 2 = D+L, M = 6934 lbs-ft
Shear : LC# 2 = D+L, V = 1734, V@d = 1734 lbs
Deflection: LC# 2 = D+L EI= 461.68e06 lb-in2/ply
Total Deflection = 1.50(Dead Load Deflection) + Live Load Deflection.
(D=dead L=live S=snow W=wind I=impact C=construction CLd=concentrated)
(All LC's are listed in the Analysis output)

DESIGN NOTES:

1. Please verify that the default deflection limits are appropriate for your application.
2. BEAMS require restraint against lateral displacement and rotation at points of bearing
3. SCL-BEAMS: Structural Composite Lumber design has assumed: - dry service conditions - full lateral support - no preservative or fire-retardant treatment
- no notches - single member use (no load sharing) - the specified dead load is no greater than 1/2 the specified live load
4. BUILT-UP SCL-BEAMS: contact manufacturer's LVL user guide for connection details

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Scale: 1/4"=1'-0"
Dr. by: S.B.
App. by:
Date: 05/13/2002
Plant: ANDERSON
LVL CALCUL
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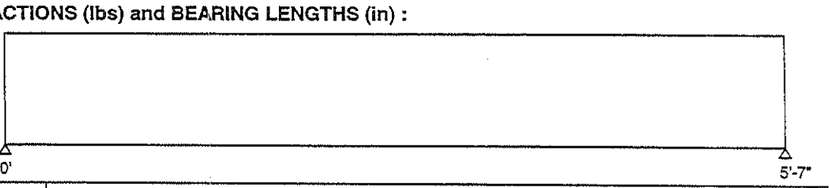
GOODLAM	**COMPANY** Goodlam Division Tel. 1-800-361-6503 Fax 1-450-635-3728 May 14, 2002 14:22:28	**PROJECT** LINTEAU R. DE C. APP 1

Design Check Calculation Sheet

LOADS: (lbs, psf, or plf)

Load	Type	Distribution	Magnitude Start	Magnitude End	Location [ft] Start	Location [ft] End	Pattern Load?
1	Live	Full UDL	368				No
2	Dead	Full UDL	119				No
3	Live	Full UDL	280				No
4	Dead	Full UDL	105				No
5	Live	Full UDL	280				No
6	Dead	Full UDL	105				No
7	Live	Full UDL	70				No
8	Dead	Full UDL	35				No
9	Live	Full UDL	70				No
10	Live	Full UDL	70				No
11	Dead	Full UDL	60				No
12	Dead	Full UDL	60				No

MAXIMUM REACTIONS (lbs) and BEARING LENGTHS (in) :



Dead	1351		1351
Live	3177		3177
Total	4528		4528
Bearing: Length	1.4		1.4

WELDWOOD LVL, 1 3/4" Wide, 2.0E, 1-3/4x9-1/2", 2-Plys

Load combinations: ASCE 7-95;

SECTION vs. DESIGN CODE NDS-1997: (lbs, lbs-ft, or in)

Criterion	Analysis Value	Design Value	Analysis/Design
Shear	V @d = 3244	Vr = 6317	V/Vr = 0.51
Bending(+)	M = 6320	Mr = 14008	M/Mr = 0.45
Live Defl'n	0.05 = <L/999	0.14 = L/480	0.36
Total Defl'n	0.08 = L/822	0.28 = L/240	0.29

ADDITIONAL DATA:

FACTORS: F	CD	CM	Ct	CL	CF	CV	Cfu	Cr	LC#
Fb'+= 3100	1.00	1.00	1.00	1.000	1.03	1.000	1.00	1.00	2
Fv' = 285	1.00	1.00	1.00			(CH = 1.000)			2
Fcp'= 900		1.00	1.00						-
E' = 2.0 million		1.00	1.00						2

```
Bending(+): LC# 2 = D+L, M =  6320 lbs-ft
Shear     : LC# 2 = D+L, V = 4528, V@d = 3244 lbs
Deflection: LC# 2 = D+L   EI= 500.14e06 lb-in2/ply
Total Deflection = 1.50(Dead Load Deflection) + Live Load Deflection.
  (D=dead  L=live  S=snow  W=wind  I=impact  C=construction  CLd=concentrated)
  (All LC's are listed in the Analysis output)
```

DESIGN NOTES:

1. Please verify that the default deflection limits are appropriate for your application.
2. BEAMS require restraint against lateral displacement and rotation at points of bearing
3. SCL-BEAMS: Structural Composite Lumber design has assumed: - dry service conditions - full lateral support - no preservative or fire-retardant treatment
- no notches - single member use (no load sharing) - the specified dead load is no greater than 1/2 the specified live load
4. BUILT-UP SCL-BEAMS: contact manufacturer's LVL user guide for connection details

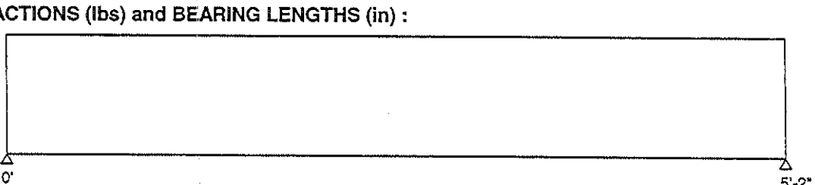
GOODLAM	COMPANY Goodlam Division Tel. 1-800-361-6503 Fax 1-450-635-3728 May 14, 2002 14:23:39	PROJECT LINTEAU R. DE C. APP 1

Design Check Calculation Sheet

LOADS: (lbs, psf, or plf)

Load	Type	Distribution	Magnitude Start	Magnitude End	Location [ft] Start	Location [ft] End	Pattern Load?
1	Live	Full UDL	368				No
2	Dead	Full UDL	119				No
3	Live	Full UDL	280				No
4	Dead	Full UDL	105				No
5	Live	Full UDL	280				No
6	Dead	Full UDL	105				No
7	Live	Full UDL	70				No
8	Dead	Full UDL	35				No
9	Live	Full UDL	70				No
10	Live	Full UDL	70				No
11	Dead	Full UDL	60				No
12	Dead	Full UDL	60				No

MAXIMUM REACTIONS (lbs) and BEARING LENGTHS (in) :



Dead	1250		1250
Live	2940		2940
Total	4190		4190
Bearing: Length	1.3		1.3

WELDWOOD LVL, 1 3/4" Wide, 2.0E, 1-3/4x9-1/2", 2-Plys

Load combinations: ASCE 7-95;

SECTION vs. DESIGN CODE NDS-1997: (lbs, lbs-ft, or in)

Criterion	Analysis Value	Design Value	Analysis/Design
Shear	V @d = 2906	Vr = 6317	V/Vr = 0.46
Bending(+)	M = 5412	Mr = 14008	M/Mr = 0.39
Live Defl'n	0.04 = <L/999	0.13 = L/480	0.28
Total Defl'n	0.06 = <L/999	0.26 = L/240	0.23

ADDITIONAL DATA:

FACTORS: F	CD	CM	Ct	CL	CF	CV	Cfu	Cr	LC#
Fb'+= 3100	1.00	1.00	1.00	1.000	1.03	1.000	1.00	1.00	2
Fv' = 285	1.00	1.00	1.00			(CH = 1.000)			2
Fcp'= 900		1.00	1.00						-
E' = 2.0 million		1.00	1.00						2

```
Bending(+): LC# 2 = D+L, M =  5412 lbs-ft
Shear     : LC# 2 = D+L, V = 4190, V@d = 2906 lbs
Deflection: LC# 2 = D+L   EI= 500.14e06 lb-in2/ply
Total Deflection = 1.50(Dead Load Deflection) + Live Load Deflection.
  (D=dead  L=live  S=snow  W=wind  I=impact  C=construction  CLd=concentrated)
  (All LC's are listed in the Analysis output)
```

DESIGN NOTES:

1. Please verify that the default deflection limits are appropriate for your application.
2. BEAMS require restraint against lateral displacement and rotation at points of bearing
3. SCL-BEAMS: Structural Composite Lumber design has assumed: - dry service conditions - full lateral support - no preservative or fire-retardant treatment
- no notches - single member use (no load sharing) - the specified dead load is no greater than 1/2 the specified live load
4. BUILT-UP SCL-BEAMS: contact manufacturer's LVL user guide for connection details

HBITEC 2000®	Scale: NONE	Dr. by: S.B.	App. by:	Date: 14/05/2002
PROP	ANDERSON			Plan: TRUSS DETAILS
				C- 07581

Ancrages pour le vent requis dans ce projet voir pages suivantes:

APPUIS CONTINUS JUSQU'AU FONDATIONS. ATTENTION LES POTEAUX AU DESSUS DU SOL DOIVENT SUPPORTER LA CHARGE PROVENANT DU TOIT AINSI QUE LA "CHARGE DE PLANCHER". LE CALCUL DE CETTE CHARGE RELEVE DE LA RESPONSABILITE DU CONSTRUCTEUR.

Structures St-Joseph Ltée

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St-Joseph, Bce, Québec
G0S 2V0

Telephone: (418) 397-5712
Fax: (418) 397-6952

Habitec 2000

Destination: Westbrook, ME, USA
Neige au sol: 60 lbs/pi2
Charges mortes: 7 + 10 lbs/pi2
Client: Prop

Résidentiel, Fermes @ 24"c/c

Projet:

02-0895 #C-07575

Scale: 1 : 96

Date: 2002-05-02

Dessinateur: C. Gagnon

200, rue du Parc
St-Joseph Beauce, Qc G0S 2V0
Tél.: (418) 397-5712 Fax: (418) 397-6952