

City of Portland, Maine - Bu	ilding or Use Permi	it	Permit No:	Date Applied For:	CBL:
389 Congress Street, 04101 Tel:	6 04-1785	12/02/2004	008 A004001		
Location of Construction:	Owner Name:		Owner Address:		Phone:
358 Eastern Promenade	City Of Portland 389 Congress St				
Business Name:	Contractor Name: Contractor Address:				Phone
	Ledgewood Inc.	PO Box 8107 Portl	PO <b>Box</b> 8107 Portland (207)		
Lessee/Buyer's Name	Phone: Permit Type:				
			Foundation Only/C	Commercial	
Proposed Use:		Proposo	d Project Description:		
Foundation only for the 70,000 sq ft	East End School	-	ation only for the 70	000 so ft East End	School
Touhuation only for the 70,000 sq.in	a East End School	Toulia	ation only for the 70	,000 sq.m. East End	i School
Dente 7. Statem	A		M 01 11		-4
	Approved	Reviewer:	Marge Schmucka	Approval D	
Note:					Okto Issue: 🗹
1) Approved on permit #041016					
Dente D'11' Ctatan		D	M.1. Nugant	A	-4 10/02/2004
	Approved with Condition	ns <b>Reviewer:</b>	Mike Nugent	Approval D	
Note:					Ok to Issue:
<ol> <li>Special inspections pursuant to t Special Inspections.</li> </ol>	he Engineers notes on Pa	ge S.1R1 must of	ccur as well as thos	e specified in the St	atement of
This statement is being emended	hy Dinthom and Cusan a	nd automitted for	norration and ammuorra	1	
This statement is being amended	by Pinknam and Greer a	nd submitted for	reveiw and approva	ll	
Dept: Fire Status:	Approved	<b>Reviewer:</b>	Lt. MacDougal	Approval Da	ate:
Note:			0		Ok to Issue: 🗹
1) Fire Dept. Approved on permit (	11016				
	1010				
All original conditions apply					
<b>Dept:</b> Engineering <b>Status:</b>	Open	Reviewer:	Tony	Approval Da	
	-		Tony		
Note: PUBLIC WORKS ENGINE	ERING REVIEW	04			Ok to Issue:
I have reviewed the applicat	ion and plans and offer th	e following com	ments:		
1. Public Works will offer co	omment upon receipt of a	more detailed ar	nd complete submitt	al.	
D 11' W. 1. D. '. 4/1/	1/04			RIMIS	800788 877 1 - 28
Public Works Review4/14	ŀ/U4			LEIAAN F. MOR	
It appears that the majority of	f improvements necessar	ry aand concerns	of Public Works ha	verbeen addressed.	
in the application materials of	lated March 30.2004. Th	he one item that r	emains outstanding	is the review of the	4
drainage design. Those initi					
Goodwin and Katherine Earl	ey of Public Works. The	erefore, comment		orth Water TesighTLA	
will be provided by those inc	lividuals, under separate	cover.		Roman and the second	Acture 2
Dente D'a Gi i i	A	D		A 1 P	
<b>Dept:</b> Fire <b>Status:</b> A	Approved	keviewer:	Lt. MacDougal	Approval Da	
Note:					Ok to Issue: 🗹
	~		<u> </u>		
-	Approved with Conditions	s <b>Reviewer:</b>	Sebago Technic	Approval Da	
Note:					Ok to Issue:
ls norman i i i	ng from the Planning Boa	ard raviou			

[Location of Construction:	Owner Name:	Owner Address:	Phone:
358 Eastern Promenade	City Of Portland	389 Congress St	
Business Name:	Contractor Name:	Contractor Address:	Phone
	Ledgewood Inc.	PO Box 8107 Portland	(207) 767-1866
Lessee/Buyer's Name	Phone:	Permit Type:	
		Foundation Only/Commercial	

PERMITISSUED
DEC 0 2 2004
CITY OF PORTLAND

City of Portland, Maine	- Building or Use	Permit Applicatio	on Per	rmit <sup>No:</sup>	Issue Date:	CBL:	
389 Congress Street, 04101	Tel: (207) 874-8703	, Fax: (207) 874-87	/16	04-1785	12/02/		A004001
Jocation of Construction: Owner Name:		Owne	r Address:	ু চন্দ্রইবায়	Phone:		
358 Eastern Promenade	City Of Portla	City Of Portland		Congress St		14	
Business Name:	Contractor Name	-	Contr	actor Address:	lij Diro	2 700 Phone	
	Ledgewood In	с.			£.	20776	71866
Lessee/Buyer's Name	Phone:			t Type: ndation Only/	OTY OF P	TELAND (	Zone:
Past Use:	Proposed Use:			it Fee:	Cost of Work:	CEO Distric	 t:
Jack Jr. High	_	ly for the 70,000				I	
sq.ft. East End S		School	FIRE		Approved Us	e Group:	NDATIO
Proposed Project Description:				/014	,	IĐ.	2 [04]
Foundation only for the 70,00	0 sq.ft. East End School	l	Signature S		Sig	gnature: Clip 4	
		Action: Approved Approved w/Conditions Denied			Denied		
			Signa	ture:		Date:	
Permit Taken By: mjn	Date Applied For: 12/02/2004		•	Zoning	Approval		
	pes not preclude the	Special Zone or Reviews		Zonir	ng Appeal	Historic	Preservation
<ol> <li>This permit application does not preclude the Applicant(s) from meeting applicable State and Federal Rules.</li> </ol>		Shoreland		Variance	9	Not in D	istrict or Landmark
2. Building permits do not include plumbing, septic or electrical work.		Wetland		Miscellaneous		Does No	t Require Review
<ol> <li>Building permits are void if work is not started within six (6) months of the date of issuance. False information may invalidate a building permit and stop all work</li> </ol>		Flood Zone		Conditional Use		Requires	Review
		Subdivision		Interpret	ation	Approve	d
				Approve	d	Approve	d w/Conditions
		Maj 🗆 Minor 🕁 yīl	$\dot{\mathbf{M}}$	Denied		Denied	
		late:		Date:		late:	

### CERTIFICATION

I hereby certify that I am the owner of record of the named property, or that the proposed work is authorized by the owner of record and that I have been authorized by the owner to make this application as his authorized agent and I agree to conform to all applicable laws of this jurisdiction. In addition, if a permit for work described in the application is issued, I certify that the code official's authorized representative shall have the authority to enter all areas covered by such permit at any reasonable hour to enforce the provision of the code(s) applicable to such permit.

READANAIRI E REPARIN RUAN RALAR OF AFTWORK THE		5.175	BUGNE
SIGNATURE OF APPLICANT	ADDRESS	DATE	PHONE

# BUILDING PERMIT INSPECTION PROCEDURES Please call 874-8703 or 874-8693 to schedule your inspections as agreed upon

Permits expire in 6 months, if the project is not started or ceases for 6 months.

The Owner or their designee is required to notify the inspections office for the following inspections and provide adequate notice. Notice must be called in **48-72** hours in advance in order to schedule an inspection:

By initializing at each inspection time, you are agreeing that you understand the inspection procedure and additional fees from a "Stop Work Order" and "Stop Work Order Release" will be incurred if the procedure is not followed as stated below.

A Pre-construction Meeting will take place upon receipt of your building permit.

Footing/Building Loc	ation Inspection;	Prior to pouring concrete	
_X_ Re-Bar Schedule Insp	pection:	Prior to pouring concrete	
Foundation Inspectio	n:	Prior to placing ANY backfill	
/ Framing/Rough Plum	bing/Electrical:	Prior to any insulating or drywalling	
Final/Certificate of O	use. N	to any occupancy of the structure or NOTE: There is a \$75.00 fee per ction at this point.	

Certificate of Occupancy is not required for certain projects. Your inspector can advise you if your project requires a Certificate of Occupancy. All projects DO require a final inspection

**If any** of the inspections do not occur, the project cannot go on to the next phase, REGARDLESS OF THE NOTICE OR CIRCUMSTANCES.

\_\_\_\_\_ CERIFICATE OF OCCUPANICES MUST BE ISSUED AND PAID FOR, BEFORE THE SPACE MAY BE OCCUPIED

- PA-	12/2/04
Signature of Applicant/Designee	Date
Donna portin Hamin HSt.	12204
signature of Inspections'Official	Date
CBL: & A CO 4 Building Permit #:	241785



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#### **GEOTECHNICAL RECOMMENDATIONS REPORT**

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### PROPOSED EAST END SCHOOL FORMER JACK ELEMENTARY SCHOOL PORTLAND, MAINE

**Prepared for:** 

DeLuca-Hoffman Associates, Inc. 778 Main Street, Suite 8 South Portland, Maine 04106

**Prepared by:** 

Summit Geoengineering Services Project 7760 January 16,2004



January 16,2004 Summit #7760

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DeLuca Hoffman Associates, Inc. Attn: Mr. William G. Hoffman, P.E. 778 Main Street, Suite 8 South Portland, Maine **041**06

Reference: Geotechnica! Report, East End Elementary School, Portland, Maine

Dear Mr. Hoffman:

In accordance with our proposal dated September 2,2003 and **as** authorized by letter dated November 11,2003, we have prepared geotechnical recommendations in connection with the construction of the proposed East End School.

The **soil** encountered consists of topsoil and/or fill overlying native glacial till with cobbles and boulders. Groundwater was observed to within **two** feet of ground surface. Bedrock was not encountered in the explorations. The soil at this site **is** suitable to **support** the proposed building on conventional spread footing foundation and slabs-on-grade. Exterior perimeter foundation drains are recommended.

Our report also includes recommendations **for** the design and construction of the foundations, slabs-on-grades, pavement sections, the multipurpose field, and the stormwater management area.

We have appreciated providing geotechnical engineering services for this phase of the project. If there are any questions **or we** may be **of** further assistance, please do not hesitate to call.

Very Truly Yours, Summit Geoengineering Services OF Aca Very Truly Yours, ERIK J. Erik J. Wiberg, P.B. Senior Geotechnical Engine WIBERG 9178

William M. Peterlein, P.E. Principal Geotechnical Engineer

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# SECTION 1 INTRODUCTION

#### 1.1 Introduction

**The** Portland School Department is planning to construct a new school building at the site of the former Jack Elementary School on North Street in Portland, Maine. As part of this planning, Summit Geoengineenng Services (Summit) **was** retained by DeLuca-Hoffman & Associates, Inc. to perform a geotechnical investigation for the proposed development and to prepare this report with earthwork, pavement and foundation design, and construction recommendations. This work was completed in accordance with our proposal dated September 2, 2003.

#### 1.2 <u>Scope of Services</u>

Our scope of field services for the geotechnical investigation consisted of drilling 7 soil borings, installing two groundwater observation wells, and obtaining 6 topsoil samples from the existing ballfield to evaluate loam quality. The primary intent of our investigation was to develop subsurface information for design and construction of the building foundations, pavement sections, stormwater management areas, and earthwork. This work was performed in its entirety as proposed.

#### 1.3 <u>Hazardous Waste Disclaimer</u>

The scope of our work on this project does not include **an** environmental assessment or investigation into the presence or absence of contaminated soil or groundwater. Summit did not identify evidence of a release of hazardous substances and/or petroleum products at the exploration locations. **Any** comments regarding the nature and composition of the subsurface materials discovered are presented for informational purposes **only**.

north end of the parking are to about 2 feet above grade at the south end. Proposed grades in the ball field area are generally about 2 feet below current site grades.

Pinkham & Greer Consulting Engineers, Inc., the project structural engineering consultant, provided structural loading information to Summit. The exterior column loads will range from 20 kips to 83 kips and interior column loads will range from 20 kips to 103 kips in the classroom wing; approximately 43 percent of the **mexcimum** column loads is attributed to dead loads. For the gymnasium portion of the school, the interior column loads will range from 40 kips to 90 kips and exterior column loads will range from 20 kips to 60 kips with dead loads constituting approximately 21 percent of the maximum column ioad. Exterior school wall loads will be 100 percent dead loads ranging from 625 to 900 pounds per linear foot. Exterior wall spread footings associated with the column loads will be constructed integral with the exterior wall footings. Column spacings in the school will range from 10 to 36 feet.

The area of the seasonally flooded skating pond on the south side of North Street will be the site of the project stonnwater detention *area* Two options are under consideration for the stonnwater detention: above-grade detention and subsurface detention chambers. If above-grade detention is used, the total and ponded depth relative to the existing skating pond will be about three feet lower. The pond would be designed **as** a *dxy* pond. If subsurface detention were used, the structures would be located about 6 feet below existing grade.

The existing school recreation fields **on** the south side of the Jack Elementary School will be reconfigured **as** part of the project. The new multipurpose field will be used for adult softball for approximately 900 hours per **year** and the field will also be used for school recess activities. The multipurpose field turf system will need to be upgraded to accommodate the more intense and Frequent uses relative to the current facility.

**As** part of site development, the existing Jack Elementary School building will be demolished. We understand that the School Department would like to process demolished brick, concrete, and masonry and reuse this material onsite **as** fill.

at B7, which was drilled through the skating area **berm**. A summary of the exploration methods and logs of the explorations are attached. The stratification lines on the logs represent approximate boundaries between soil types and the transitions may be more gradual than implied.

Groundwater observation well OW1 was **installed** in borehole B4 and observation well OW2 was installed in the middle of the skating area. The observation well sand pack zone of OW1 extends from **4** to 19 feet below ground surfaces. The sand pack of OW2 is located from 7 to 14 feet below ground surface. Well construction details are provided on the logs in Appendix B-

Hand auguring was used to collect topsoil samples and measure the total topsoil thickness at six locations within the existing multipurpose field. The topsoil samples were evaluated in laboratories for nutrient content and soil gradation.

Owen Haskell, Inc. horizontally and vertically surveyed ground surface at the exploration locations, except for **boring** B4. The location of B4 and the six topsoil hand auger locations were established by Summit by measurement from onsite features.

# SECTION 4 LABORATORY TESTING

Laboratory tests were performed on the topsoil hand auger samples and on selected split spoon samples. Topsoil samples were analyzed for soil pH and soil nutrients by Maine Soil Testing Service at the University of Maine in Orono. Subsamples of the topsoil and split-spoon soil samples were tested by Summit for soil gradation. The results of the laboratory tests are summarized on Table C-1 in Appendix *C*. Sample **data** sheets are also provided in Appendix C. Data from the split-spoon samples is provided on the soil boring logs. gravelly sand with silt. Approximately 12 inches **d** gravelly sand pavement base material was encountered beneath the bituminous pavement at boring B4.

Near surface soils encountered in the **borings** drilled near the school building were similar in texture to the underlying native glacial till but generally less dense **and** could have been placed as fill during original site development. This **scil** was not readily distinguishable in the split-spoon samples from the underlying glacial till. The SPTN-Value in the near surface soils ranged from 6 to **18** blows per foot (bpf), which corresponds to compact soil conditions.

*Glacial* till was encountered at all boring locations and typically consisted of olive-brown to olive-gray, silty sand to sandy silt with little gravel. Cobbles and/or boulders were also present in the glacial till. SPT N-Values in the glacial till below a depth of 4 feet ranged from 22 to 88 blows per foot and indicated compact to very dense soil conditions and/or the presence of cobbles and boulders. Gradation tests were conducted on two split spoon samples: sample S2 from 4.5 to 6.5 feet at B4 and sample S3 from 9.5 to 11 feet at B2. The test results are summarized adjacent to the sample locations on the boring logs and gradation curves are presented in Appendix C.

### 5.2 <u>Bedrock</u>

and a second

Bedrock was not encountered in the borings.

#### 5.3 <u>Groundwater</u>.

Groundwater levels were measured in **the** two observation wells installed at the site approximately 48 to 72 hours after installation and about approximately 2-1/2 weeks after installation. Groundwater levels were measured in borehole **B1** approximately 6 **and** 24-hours after drilling. The groundwater levels were not determined at other boring locations due to due to slow groundwater recovery rates in the dense, silty soils. The water levels are summarized below:

# SECTION 7 FOUNDATION AND EARTHWORK RECOMMENDATIONS

#### 7.1 Building Foundation Design Recommendations

A. Foundation Type. Based on proposed finish floor elevation between elevation 115 and 1**16** feet, the footings and slabs will be constructed on compacted Gravel Borrow fill (Section 7.1.F) or recompacted in-situ glacial till. These materials are suitable to support the proposed school building foundations using conventional spread footing foundations and the slabs-on-grade.

**B.** <u>Allowable Bearing Pressures</u>. We recommend that the footings constructed for the proposed building be proportioned using an allowable bearing pressure of 3,000 psf at all locations. The computed *maximum* settlement associated with this allowable bearing pressure is approximately <sup>3</sup>/<sub>4</sub>-inch. Computed differential settlements are within tolerable limits for the proposed brick veneer exterior fnish and interior column spacings of 10 feet or more. The computed factor of safety **against** bearing capacity failure is greater **than** 3.

The allowable bearing pressure and the associated settlements are based on the following assumptions:

- All components of existing concrete slabs and foundation elements of existing buildings and associated construction debris are removed from within the proposed building footprint. Exposed soil is proofrolled and the excavations are backfilled with approved granular backfill.
- For areas outside the existing Jack Elementary School footprint, exposed soil is proofrolled and the excavations are backfilled with approved granular backfill.
- All topsoil and organic material is removed from within the building areas prior to proofrolling or the placement of fill.
- The native soil and fill soil exposed at foundation subgrade are compacted prior to constructing footings.

placed in 9 to 12-inch thick lifts. Where hand compaction equipment is used, the lift thickness should be reduced to 6 to 8 inches.

**The** glacial till soils encountered in the explorations are not suitable for use **as** Foundation Backfill due to the excessive amount of silt and clay size particles (the portion passing the No. 200 sieve size). It is our professional opinion that the glacial till soil cannot be economically processed onsite **to** meet the above gradation criteria.

Evaluation of the base gravel beneath existing paved and building areas is beyond the scope of the current evaluation. Depending on the materials used in construction and their handling during demolition, it is possible that the base material beneath the existing paved areas and the building slab will meet the gradation requirements for Foundation Backfill (and possibly Structural Fill or Gravel Borrow, see Section **7.1.F** below). If reuse of these materials is proposed during construction, they should be tested for compatibility with their intended use.

D. <u>Foundation Drainage</u>. The groundwater levels measured in observation well OW1 and in open borehole B1 were above the projected bottom of foundations at these locations (approximately elevation 106 feet and 111.0, respectively) and we anticipate that the depth to groundwater is similar in other areas. If foundation *drains* are not provided, groundwater could rise above the bottom of the foundation footings and potentially above the slab in the southwest portion of the building where proposed finish floor level is about 5 feet below existing grade.

We recommend that foundation underdrains be installed at the base of the perimeter foundation footings on all sides of the school. The finish grade around the perimeter of the building should be sloped to promote drainage away **from** the foundations.

Perimeter foundation underdrains should consist of **4** inch rigid perforated PVC surrounded by a minimum of 6 inches of crushed stone wrapped in filter fabric to prevent clogging from the migration of the fine soil particles in the foundation backfill soils. The underdrain pipe should be outlet to a location where it will be free flowing. Where exposed at the ground surface, the

perimeter frost wall footings with the **bottom** of footings located **a minimum** of **4** feet below finish grade.

Fill, where required beneath the Structural Fill, should consist of Gravel Borrow (MDOT 703.20). The Gravel Borrow should extend a minimum of 5 feet horizontally outside the proposed building limits at finish grade with side slopes at 2H: 1V or flatter. The portion of Gravel Borrow soil passing the 3-inch sieve should meet the following specification.

GRAVEL BORROW					
Sieve Size Percent finer					
3 inch	100				
1/4 inch	0 to 70				
No. 200	0 to 10				

Reference: MDOT Specification 703.20, Gravel Borrow. The **maximum** particle size is 6 inches.

Static estécuio

-Section -

strated strated Gravel Borrow should be placed in a maximum of 12-inch lifts, and should be compacted to 95 percent, in accordance with ASTM D1557.

Slabs constructed on 12 inches of Structural Fill directly overlying proof-rolled glacial till soil or less than 2 feet of compacted Gravel Borrow can be designed using a subgrade modulus value of 200 pci. Slabs constructed on 12 inches of Structural Fill and over more than 2 feet of compacted Gravel Borrow can be designed using a subgrade modulus of 225 pci.

The glacial till soils encountered in the explorations are **not** suitable for use **as** Structural Fill or as Gravel Borrow due to the excessive amount of silt and clays size particles..

Depending on the materials used in construction and their handling during demolition, it is possible that the gravel base material and fill beneath the existing building slab will meet the gradation requirements for Structural Fill **and/or** Gravel Borrow. If reuse of these materials is proposed during construction, they should be tested for compatibility with their intended use.



The mean annual freezing index for the Portland area is approximately 900 degree-days with a corresponding mean arrual fiost penetration depth of about 36 inches for the glacial till soils at the site. The glacial till soils at the site are highly frost susceptible due to the significant percentage of silt **and** clay in the soil. With respect to fiost penetration, we recommend a minimum pavement section consist of 60% percent of the mean arrual frost penetration depth, or 22 inches.

Design pavement sections based on vehicle type **and** design traffic counts provided by DeLuca-Hoffman Associates, Inc. were determined **using** the American Association of State Highway Official (AASHOT) "Guide for Design of Pavement". Vehicles traveling at low speeds will use the onsite pavement areas. Pavement design section calculations are provided in Appendix D. We recommend that following design pavement sections:

	Minin	num Thickne	Material	
Material	Bus Loop	Service Drive	Parking Lot	Specification
Asphalt Surface	1.5	1	1	<b>MDOT</b> 703.09,
Coarse				9.5 mm
Asphalt Binder	2.5	2	2	MDOT 703.09
Coarse				19.5 mm
Base Soil	3	3	3	MDOT 703.06
				Type A
Subbase Soil	18	18	16	MDOT 703.06
		·······		Type D

The material specifications are referenced to the State of Maine Department of Transportation Standard Specifications, Revision of December 2002.

All public roadways or paved areas to be maintained **or** accepted by the City of Portland should be constructed in accordance with City of Portland design standards. City of Portland Design Standards (March 2000) for collector streets, which we understand North Street is considered, require a total section thickness of 25 inches **(4** inches of asphalt pavement and 21 inches of base and subbase soil).