6-D-1

2000-0027

349 Park Ave.
High Bay Cooler Expansion
H.P. Hood

on Spreadsheet

CITY OF PORTLAND, MAINE
DEVELOPMENT REVIEW APPLICATION
PLANNING DEPARTMENT PROCESSING FORM

20000027
I. D. Number

H.P. Hood Inc.
Applicant

3/8/00
Application Date

349 Park Ave, Portland, ME 04101
Applicant's Mailing Address

High Bay Cooler Expansion
Project Name/Description

Phillip Doughty Associates
Consultant/Agent

349 Park Ave, Portland, Maine 04101
Address of Proposed Site

781-5346 781-2908
Applicant or Agent Daytime Telephone, Fax

006-D-001
Assessor's Reference: Chart-Block-Lot

Proposed Development (check all that apply): ☐ New Building ☒ Building Addition ☐ Change Of Use ☐ Residential
☐ Office ☐ Retail ☐ Manufacturing ☒ Warehouse/Distribution ☐ Parking Lot ☐ Other (specify)

7,110 sf
Proposed Building square Feet or # of Units

1.7393
Acreage of Site

Zoning

Check Review Required:

☒ Site Plan (major/minor)
☐ Subdivision # of lots
☐ PAD Review
☐ 14-403 Streets Review
☐ Flood Hazard
☐ Shoreland
☐ HistoricPreservation
☐ DEP Local Certification
☐ Zoning Conditional Use (ZBA/PB)
☐ Zoning Variance
☐ Other

Fees Paid: Site Plan $400.00 Subdivisio Engineer Review Date 3/8/00

Planning Approval Status:

Reviewer sarah

☒ Approved ☐ Approved w/Conditions See Attached ☐ Denied

Approval Date 5/3/00 Approval Expiration 5/3/01 Extension to ☐ Additional Sheets Attached

☒ OK to Issue Building Permi sarah (signature) 5/4/00 (date)

Performance Guarantee ☐ Required* ☒ Not Required

* No building permit may be issued until a performance guarantee has been submitted as indicated below

☐ Performance Guarantee Accepted — date — amount — expiration date

☒ Inspection Fee Paid — date — $300.00 amount

☐ Building Permit Issue — date

☐ Performance Guarantee Reduced — date — remaining balance — signature

☐ Temporary Certificate of Occupancy — date ☐ Conditions (See Attached)

☐ Final Inspection — date — signature

Certificate Of Occupancy — date

rformance Guarantee Released — date — signature

ct Guarantee Submitted — submitted date — amount — expiration date

Guarantee Released — date — signature

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☐ Certificate Of Occupancy	date		
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☐ Defect Guarantee Submitted	submitted date	amount	expiration date

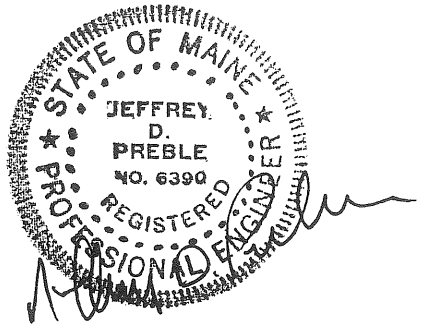
STORMWATER MANAGEMENT REPORT

for the

H.P. HOOD, INC.
BUILDING EXPANSION PROJECT

Completed By
Dufresne-Henry, Inc.

Completed For
Phillip J. Doughty Associates - Architects



March 2000

Dufresne-Henry, Inc.
Consulting Engineers

BACKGROUND SUMMARY

PROJECT DESCRIPTION

H.P. Hood, Inc. is proposing an expansion to its principal facility located at 340 Park Avenue in Portland, Maine. The expansion includes the construction of a high bay cooler building located adjacent to the existing main building and will encompass a total area of approximately 7,100 square feet.

The existing H.P. Hood, Inc. site is approximately 2 acres and contains the main building facility, loading docks, trailer storage area, and parking lot. The entire area is paved and includes an existing stormwater drainage system consisting of catchbasins that convey stormwater runoff to the City combined sewer system on Park Avenue.

This drainage analysis was prepared to determine the stormwater runoff effects of the H.P. Hood, Inc. expansion project. Drainage calculations for pre-development conditions and post-development conditions have been developed to determine the difference between existing drainage conditions and those resulting from the proposed building expansion.

DRAINAGE ANALYSIS SUMMARY

INTRODUCTION

There is one distinct drainage area at the project site in the pre and post-development conditions and includes the existing H.P. Hood, Inc. facility and bituminous pavement parking lot to the property boundary. Stormwater management on the site will not be significantly affected by the proposed expansion. Proposed regrading will include slight modifications to existing grades in the vicinity of the proposed building to promote drainage to existing and proposed catchbasins. The proposed project will be designed such that no negative impacts to adjacent properties, or the City sewer system are anticipated.

METHODOLOGY

In order to compare the stormwater characteristics of today with the future stormwater characteristics of the site, computer modeling using HydroCad software was employed which incorporates the methodology outlined in the U.S. Natural Resources Conservation Service's (NRCS) Technical Release Number 20 (TR-20). Runoff was calculated for the 2, 10 and 25-year, 24 hour storm events. The one day precipitation values for the Portland, ME and the project site are as follows:

2-Year, 24 Hour Precipitation - 3.0 inches
10-Year, 24 Hour Precipitation - 4.7 inches
25-Year, 24 Hour Precipitation - 5.5 inches

Since the H.P. Hood, Inc. facility is located in southeast Maine, a Type III distribution was utilized throughout this study. Also, the project site has been designated as impervious in the pre and post-development conditions for this study. All curve number and time of concentration calculations may be found along with computer generated documentation following this narrative.

STORMWATER RUNOFF ANALYSIS

Subcatchments 1, Pre-Development and Post-Development Conditions

The drainage area encompassing the expansion project to the H.P. Hood facility is approximately 2 acres. A majority of the stormwater runoff generated adjacent to the project site is conveyed to the City sewer system through drainage structures not considered in this report, and therefore was not included in the stormwater runoff calculations. This area has been analyzed as one subcatchment for the entire drainage area for the pre-development conditions and for the post-development conditions (See Dwg. A, Pre-Development Conditions and Dwg B, Post-Development Conditions). Subcatchment 1 represents the area which drains to DMH 1 prior to being discharged through a 20-inch vitrified clay pipe from the project site. Generally, stormwater runoff generated on the project site sheet flows in a southwesterly direction to an existing catchbasin that conveys the collected stormwater runoff to DMH 1 through an existing 12-inch RCP pipe.

In the post-development condition, Subcatchment 1 is essentially unchanged. The building expansion results in less bituminous pavement on the site but the amount of impervious area at the site is unchanged. Also, as part of the proposed expansion, stormwater runoff is conveyed through 12-inch RCP pipes around the proposed expansion due to existing CB 2 being removed and existing CB 1 being removed and replaced with a new catchbasin to the east of the new building. Relocated CB 1 will tie-into two proposed structures installed along the north side of the building (DMH-2 and CB-3). As in the pre-development condition, a majority of the stormwater runoff generated on the site is conveyed to DMH 1 prior to being discharged from the project site.

The estimated peak rates of runoff in cubic feet per second (CFS) in the pre-development and post-development conditions for Subcatchments 1, with the projected differences in parenthesis are as follows:

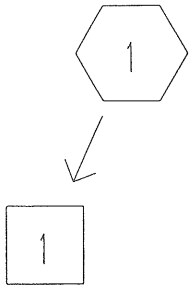
Pre-Development, 2 year storm		Post-Development, 2 year storm	
Subcatchment 1	6.07 CFS	Subcatchment 1	6.07 CFS (0 CFS)
Pre-Development, 10 year storm		Post-Development, 10 year storm	
Subcatchment 1	9.58 CFS	Subcatchment 1	9.58 CFS (0 CFS)
Pre-Development, 25 year storm		Post-Development, 25 year storm	
Subcatchment 1	11.23 CFS	Subcatchment 1	11.23 CFS (0 CFS)

CONCLUSIONS

The existing 12-inch RCP pipe conveying stormwater runoff to DMH 1 was analyzed in the post-development conditions using the Field's Hydraulic Calculator based on Manning's Equation. The results of this analysis demonstrated that sufficient capacity exists in the 12-inch RCP pipe to convey stormwater runoff for the 10-year storm event. During the 25-year storm event, the 12-inch RCP pipe is flowing at capacity producing 1.5 feet of headwater in the existing catchbasin. Lastly, the existing 18-inch RCP pipe draining DMH 1 has sufficient capacity to convey stormwater runoff for the 25-year storm event in the post-development conditions.

Therefore, the proposed development will have no adverse impact on Subcatchment 1. Projected post-development peak runoff quantities and the quality of stormwater runoff will not change from pre-development conditions.

WATERSHED ROUTING ==========



SUBCATCHMENT 1 = DRAINAGE AREA TO CATCH BASIN (RIM-11.43') -> REACH 1

REACH 1 = ->

Data for **HP HOOD COOLER EXPANSION**
TYPE III 24-HOUR RAINFALL= 3.00 IN
Prepared by DUFRESNE-HENRY, INC. 11 Feb 00
HydroCAD 5.11 001123 (c) 1986-1999 Applied Microcomputer Systems

SUBCATCHMENT 1 **DRAINAGE AREA TO CATCH BASIN (RIM-11.43')**

PEAK= 6.07 CFS @ 11.99 HRS, VOLUME= .39 AF

ACRES	CN	
2.07	98	IMPERVIOUS

SCS TR-20 METHOD
TYPE III 24-HOUR
RAINFALL= 3.00 IN
SPAN= 10-20 HRS, dt=.1 HRS

Method	Comment	Tc (min)
TR-55 SHEET FLOW	**Segment ID:**	1.6
Smooth surfaces n=.011 L=205'	P2=5.5 in s=.022 '/'	
CIRCULAR CHANNEL	**Segment ID:**	.3
12" Diameter a=.79 sq-ft Pw=3.1'	r=.25'	
s=.0066 '/' n=.011 V=4.36 fps	L=85' Capacity=3.4 cfs	
	Total Length= 290 ft Total Tc=	1.9

REACH 1

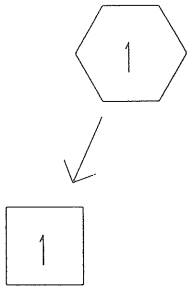
Qin = 6.07 CFS @ 11.99 HRS, VOLUME= .39 AF
Qout= 6.00 CFS @ 11.99 HRS, VOLUME= .39 AF, ATTEN= 1%, LAG= 0.0 MIN

DEPTH (FT)	END AREA (SQ-FT)	DISCH (CFS)
0.00	0.00	0.00
.15	.09	.26
.30	.25	1.09
.45	.45	2.43
1.05	1.32	10.39
1.20	1.52	12.13
1.35	1.68	13.23
1.41	1.72	13.35
1.46	1.75	13.23
1.50	1.77	12.41

18" PIPE
n= .011
LENGTH= 55 FT
SLOPE= .01 FT/FT

STOR-IND+TRANS METHOD
PEAK DEPTH= .72 FT
PEAK VELOCITY= 7.2 FPS
TRAVEL TIME = .1 MIN
SPAN= 10-20 HRS, dt=.1 HRS
2 x FINER ROUTING

WATERSHED ROUTING ==========



SUBCATCHMENT 1 = DRAINAGE AREA TO CATCH BASIN (RIM-11.43') -> REACH 1

REACH 1 = ->

Data for **HP HOOD COOLER EXPANSION POST-DEVELOPMENT**
TYPE III 24-HOUR RAINFALL= 3.00 IN
Prepared by DUFRESNE-HENRY, INC. 11 Feb 00
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SUBCATCHMENT 1 **DRAINAGE AREA TO CATCH BASIN (RIM-11.43')**

PEAK= 6.07 CFS @ 11.99 HRS, VOLUME= .39 AF

ACRES	CN	
2.07	98	IMPERVIOUS

SCS TR-20 METHOD
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RAINFALL= 3.00 IN
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Method	Comment	Tc (min)
TR-55 SHEET FLOW	**Segment ID:**	1.6
Smooth surfaces n=.011 L=205'	P2=5.5 in s=.022 '/'	
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12" Diameter a=.79 sq-ft Pw=3.1' r=.25'		
s=.0066 '/' n=.011 V=4.36 fps	L=85' Capacity=3.4 cfs	
	Total Length= 290 ft Total Tc=	1.9

REACH 1

Qin = 6.07 CFS @ 11.99 HRS, VOLUME= .39 AF
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DEPTH (FT)	END AREA (SQ-FT)	DISCH (CFS)
0.00	0.00	0.00
.15	.09	.26
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n= .011
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PEAK VELOCITY= 7.2 FPS
TRAVEL TIME = .1 MIN
SPAN= 10-20 HRS, dt=.1 HRS
2 x FINER ROUTING

SUBCATCHMENT 1 **DRAINAGE AREA TO CATCH BASIN (RIM-11.43')**

PEAK= 9.58 CFS @ 11.99 HRS, VOLUME= .62 AF

ACRES	CN	
2.07	98	IMPERVIOUS

SCS TR-20 METHOD
TYPE III 24-HOUR
RAINFALL= 4.70 IN
SPAN= 10-20 HRS, dt=.1 HRS

Method	Comment	Tc (min)
TR-55 SHEET FLOW	**Segment ID:**	1.6
Smooth surfaces n=.011 L=205'	P2=5.5 in s=.022 '/'	
CIRCULAR CHANNEL	**Segment ID:**	.3
12" Diameter a=.79 sq-ft Pw=3.1'	r=.25'	
s=.0066 '/' n=.011 V=4.36 fps	L=85' Capacity=3.4 cfs	
	Total Length= 290 ft Total Tc=	1.9

Data for **HP HOOD COOLER EXPANSION**
TYPE III 24-HOUR RAINFALL= 4.70 IN
Prepared by DUFRESNE-HENRY, INC. 11 Feb 00
HydroCAD 5.11 001123 (c) 1986-1999 Applied Microcomputer Systems

REACH 1

Qin = 9.58 CFS @ 11.99 HRS, VOLUME= .62 AF
Qout= 9.49 CFS @ 11.99 HRS, VOLUME= .62 AF, ATTEN= 1%, LAG= 0.0 MIN

DEPTH (FT)	END AREA (SQ-FT)	DISCH (CFS)
0.00	0.00	0.00
.15	.09	.26
.30	.25	1.09
.45	.45	2.43
1.05	1.32	10.39
1.20	1.52	12.13
1.35	1.68	13.23
1.41	1.72	13.35
1.46	1.75	13.23
1.50	1.77	12.41

18" PIPE
n= .011
LENGTH= 55 FT
SLOPE= .01 FT/FT

STOR-IND+TRANS METHOD
PEAK DEPTH= .98 FT
PEAK VELOCITY= 7.8 FPS
TRAVEL TIME = .1 MIN
SPAN= 10-20 HRS, dt=.1 HRS
2 x FINER ROUTING

Data for **HP HOOD COOLER EXPANSION POST-DEVELOPMENT**
TYPE III 24-HOUR RAINFALL= 4.70 IN
Prepared by DUFRESNE-HENRY, INC. 11 Feb 00
HydroCAD 5.11 001123 (c) 1986-1999 Applied Microcomputer Systems

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PEAK= 9.58 CFS @ 11.99 HRS, VOLUME= .62 AF

ACRES	CN	
2.07	98	IMPERVIOUS

SCS TR-20 METHOD
TYPE III 24-HOUR
RAINFALL= 4.70 IN
SPAN= 10-20 HRS, dt=.1 HRS

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TR-55 SHEET FLOW	**Segment ID:**	1.6
Smooth surfaces n=.011 L=205'	P2=5.5 in s=.022 '/'	
CIRCULAR CHANNEL	**Segment ID:**	.3
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s=.0066 '/' n=.011 V=4.36 fps	L=85' Capacity=3.4 cfs	
	Total Length= 290 ft Total Tc=	1.9

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Qout= 9.49 CFS @ 11.99 HRS, VOLUME= .62 AF, ATTEN= 1%, LAG= 0.0 MIN

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n= .011
LENGTH= 55 FT
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PEAK VELOCITY= 7.8 FPS
TRAVEL TIME = .1 MIN
SPAN= 10-20 HRS, dt=.1 HRS
2 x FINER ROUTING

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TYPE III 24-HOUR RAINFALL= 5.50 IN
Prepared by DUFRESNE-HENRY, INC. 9 Feb 00
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SUBCATCHMENT 1 **DRAINAGE AREA TO CATCH BASIN (RIM-11.43')**

PEAK= 11.23 CFS @ 11.99 HRS, VOLUME= .73 AF

ACRES	CN	
2.07	98	IMPERVIOUS

SCS TR-20 METHOD
TYPE III 24-HOUR
RAINFALL= 5.50 IN
SPAN= 10-20 HRS, dt=.1 HRS

Method	Comment	Tc (min)
TR-55 SHEET FLOW	**Segment ID:**	1.6
Smooth surfaces n=.011 L=205'	P2=5.5 in s=.022 '/'	
CIRCULAR CHANNEL	**Segment ID:**	.3
12" Diameter a=.79 sq-ft Pw=3.1' r=.25'		
s=.0066 '/' n=.011 V=4.36 fps	L=85' Capacity=3.4 cfs	
	Total Length= 290 ft Total Tc=	1.9

REACH 1

Qin = 11.23 CFS @ 11.99 HRS, VOLUME= .73 AF
Qout= 11.12 CFS @ 11.99 HRS, VOLUME= .72 AF, ATTEN= 1%, LAG= 0.0 MIN

DEPTH (FT)	END AREA (SQ-FT)	DISCH (CFS)
0.00	0.00	0.00
.15	.09	.26
.30	.25	1.09
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n= .011
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SLOPE= .01 FT/FT

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PEAK VELOCITY= 7.9 FPS
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DRC Approval Status:

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☐ **Approved w/Conditions** see attache
☐ **Denied**

Approval Date 5/3/00 Approval Expiration 5/3/01 Extension to ☐ Additional Sheets Attached

☒ Condition Compliance sarah (signature) 5/4/00 (date)

Performance Guarantee ☐ **Required*** ☒ **Not Required**

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☐ Performance Guarantee Released	date	signature	
☐ Defect Guarantee Submitted	submitted date	amount	expiration date
☐ Defect Guarantee Released	date	signature	

RENTAL AGREEMENT

TELAHC PROPERTIES, L.P. with a principal place of business at Chalet Drive, Wilton, NH 03086 (hereinafter referred to as TELAHC) agrees to rent to H. P. HOOD, INC. certain parking spaces on TELAHC's property located at 340 Park Avenue, Portland, Maine, known as the Susse Chalet.

Similarly, H. P. HOOD, INC. with a principal place of business at 349 Park Avenue, Portland, ME 04102 (hereinafter referred to as HOOD) agrees to rent such parking spaces from TELAHC.

TERMS:

1. HOOD agrees that it will:

   (a) Pay TWENTY DOLLARS ($20.00) per space per month for thirty (30) parking spaces or SIX HUNDRED DOLLARS ($600.00) per month. Rental payments for SIX HUNDRED DOLLARS ($600.00) per month are to be made in advance by the 1st day of each month and without regard as to number of spaces used, commencing on the 1st day of July, 1999 and running for a period of two (2) years ending June 30, 2001, and to be re-negotiated at that time.
   (b) Parking spaces will only be used between the hours of 7:30 a.m. and 5:00 p.m. HOOD will hold TELAHC harmless for any damage done to cars parked on the property.

2. TELAHC agrees that it will:

   (a) Properly stripe and identify the appropriate parking spaces.
   (b) Clear the parking lot of snow and ice.
   (c) Make available such parking spaces between the hours of 7:30 a.m. and 5:00 p.m.

Each party recognizes that this Rental Agreement may be terminated with a ninety (90) days written notice of either party due to business reasons or other changes.

Each party also recognizes that this Rental Agreement shall terminate due to any change of ordinances or requirements of the City of Portland.

TELAHC PROPERTIES, L.P.
By: _____
Dated: 10/22/99 Title: Alan G. Retter, VP of CSI, Property Mgr.
Duly Authorized

H. P. HOOD, INC.
By: _____
Dated: _____ Title: Scott Blake, Operations Manager
Duly Authorized

U/lvh-Leases Intown
Telahc & H.P.Hood

Paramount
HOTEL GROUP

April 3, 2000

Via Facsimile (207) 773-2913
HP Hood, Inc.
Northern Region
Attention: Janet Dow

Dear Ms Dow:

Please be advised that Paramount Management Associates, LLC is presently managing agent for Oly Realty One, LLC and Paramount Hotel Group, LLC, joint owners of the Portland/Intown property.

We have assumed the existing contract between your organization and Telahc Properties, L.P. All correspondence and rental payments should be forwarded to my attention at the following address:

Paramount Management Associates, LLC
710 Route 46 East, Suite 102
Fairfield, New Jersey 07004
Attention: Peter Marino

This letter shall also confirm that HP Hood's rental payment is $600.00. Should you have any questions, please do not hesitate to contact me.

Sincerely,

Peter Marino
Senior Vice President/Operations

PM:mm

PDA Phillip Doughty Associates — Architects

364 U.S. Route One, Falmouth, ME 04105 e-mail: pjd1108@aol.com
Telephone: 207•781•5346 Fax: 207•781•2908

ADDENDUM #1

PROJECT: H.P. Hood High Bay Cooler Expansion and Renovation
PDA #990-028
DATE: April 12, 2000

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	Dane Ramsay	H.P. Hood
	Bob Bryant	H.P. Hood
	Scott Blake	H.P. Hood
	Brian B.	H.P. Hood
	Phil Doughty	Phillip Doughty Associates
CC:	File	

1. Sign-in Attendance List passed around the group.

2. Division 1 and Site Issues discussion:

   a. Site is extremely tight. Most trailers, though, which are seen on this day around the north and east sides of the yard, will not be on site during construction.
   b. H.P. Hood will provide two locations for trailer boxes in the northwest corner of the yard. These will be for use by mechanical and electrical contractors.
   c. General contractor's office space will be provided in the adjacent Carvel building. This will include bathroom facilities power and heat. Telephone will be the responsibility of the general contractor. Disregard Section 01500, 1.02.C.1 & .2; D.1, .2, .3, and further subsequent written requirements regarding these specific issues in that section.
   d. H.P. Hood will designate five people to facilitate and coordinate H.P. Hood's and the general contractor's use of the site.
   e. Parking for the general contractor's employees will be curbside along St. James street.
   f. Yard activity: the site will be the least busy between 6:30 and 7:00 a.m. Wednesday is the day of least activity over the course of a day.
   g. The City of Portland has required that the area catch basins, existing and new, be kept clear of all sediment and debris during the construction time period. A final cleaning is required at project closeout.
   h. The existing concrete hard stands at grade will need to be broken up and removed from the site in the area north of the existing structure.
   i. H.P. Hood is requiring the general contractor to comply fully with OSHA regulations as they relate to all construction on the site.
   j. Lay down area: once the site is excavated along the new westerly cooler and foundations are in, lay down will be along this wall. There will also be space along the rear property line.

3. **Drawing PH-I** – Phasing High bay cooler area:
   a. Phase One is to be constructed first. The area of low roof represented on this drawing as Phase 3, currently a loading dock area, must remain in operation until the westerly loading bays shown in the high bay area are operational and refrigerated. The Phase One Penthouse cannot be closed in until all refrigeration equipment is in place.

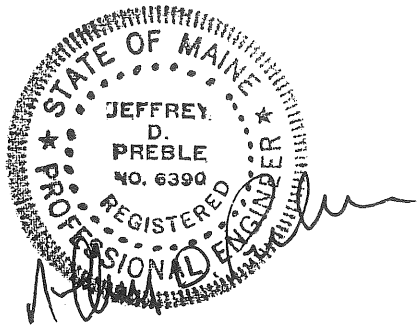
STORMWATER MANAGEMENT REPORT

for the

H.P. HOOD, INC.
BUILDING EXPANSION PROJECT

Completed By
Dufresne-Henry, Inc.

Completed For
Phillip J. Doughty Associates - Architects



March 2000

Dufresne-Henry, Inc.
Consulting Engineers

BACKGROUND SUMMARY

PROJECT DESCRIPTION

H.P. Hood, Inc. is proposing an expansion to its principal facility located at 340 Park Avenue in Portland, Maine. The expansion includes the construction of a high bay cooler building located adjacent to the existing main building and will encompass a total area of approximately 7,100 square feet.

The existing H.P. Hood, Inc. site is approximately 2 acres and contains the main building facility, loading docks, trailer storage area, and parking lot. The entire area is paved and includes an existing stormwater drainage system consisting of catchbasins that convey stormwater runoff to the City combined sewer system on Park Avenue.

This drainage analysis was prepared to determine the stormwater runoff effects of the H.P. Hood, Inc. expansion project. Drainage calculations for pre-development conditions and post-development conditions have been developed to determine the difference between existing drainage conditions and those resulting from the proposed building expansion.

DRAINAGE ANALYSIS SUMMARY

INTRODUCTION

There is one distinct drainage area at the project site in the pre and post-development conditions and includes the existing H.P. Hood, Inc. facility and bituminous pavement parking lot to the property boundary. Stormwater management on the site will not be significantly affected by the proposed expansion. Proposed regrading will include slight modifications to existing grades in the vicinity of the proposed building to promote drainage to existing and proposed catchbasins. The proposed project will be designed such that no negative impacts to adjacent properties, or the City sewer system are anticipated.

METHODOLOGY

In order to compare the stormwater characteristics of today with the future stormwater characteristics of the site, computer modeling using HydroCad software was employed which incorporates the methodology outlined in the U.S. Natural Resources Conservation Service's (NRCS) Technical Release Number 20 (TR-20). Runoff was calculated for the 2, 10 and 25-year, 24 hour storm events. The one day precipitation values for the Portland, ME and the project site are as follows:

2-Year, 24 Hour Precipitation - 3.0 inches
10-Year, 24 Hour Precipitation - 4.7 inches
25-Year, 24 Hour Precipitation - 5.5 inches

Since the H.P. Hood, Inc. facility is located in southeast Maine, a Type III distribution was utilized throughout this study. Also, the project site has been designated as impervious in the pre and post-development conditions for this study. All curve number and time of concentration calculations may be found along with computer generated documentation following this narrative.

STORMWATER RUNOFF ANALYSIS

Subcatchments 1, Pre-Development and Post-Development Conditions

The drainage area encompassing the expansion project to the H.P. Hood facility is approximately 2 acres. A majority of the stormwater runoff generated adjacent to the project site is conveyed to the City sewer system through drainage structures not considered in this report, and therefore was not included in the stormwater runoff calculations. This area has been analyzed as one subcatchment for the entire drainage area for the pre-development conditions and for the post-development conditions (See Dwg. A, Pre-Development Conditions and Dwg B, Post-Development Conditions). Subcatchment 1 represents the area which drains to DMH 1 prior to being discharged through a 20-inch vitrified clay pipe from the project site. Generally, stormwater runoff generated on the project site sheet flows in a southwesterly direction to an existing catchbasin that conveys the collected stormwater runoff to DMH 1 through an existing 12-inch RCP pipe.

In the post-development condition, Subcatchment 1 is essentially unchanged. The building expansion results in less bituminous pavement on the site but the amount of impervious area at the site is unchanged. Also, as part of the proposed expansion, stormwater runoff is conveyed through 12-inch RCP pipes around the proposed expansion due to existing CB 2 being removed and existing CB 1 being removed and replaced with a new catchbasin to the east of the new building. Relocated CB 1 will tie-into two proposed structures installed along the north side of the building (DMH-2 and CB-3). As in the pre-development condition, a majority of the stormwater runoff generated on the site is conveyed to DMH 1 prior to being discharged from the project site.

The estimated peak rates of runoff in cubic feet per second (CFS) in the pre-development and post-development conditions for Subcatchments 1, with the projected differences in parenthesis are as follows:

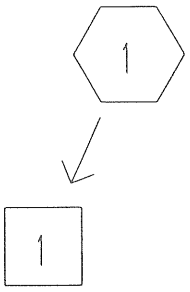
Pre-Development, 2 year storm		Post-Development, 2 year storm	
Subcatchment 1	6.07 CFS	Subcatchment 1	6.07 CFS (0 CFS)
Pre-Development, 10 year storm		Post-Development, 10 year storm	
Subcatchment 1	9.58 CFS	Subcatchment 1	9.58 CFS (0 CFS)
Pre-Development, 25 year storm		Post-Development, 25 year storm	
Subcatchment 1	11.23 CFS	Subcatchment 1	11.23 CFS (0 CFS)

CONCLUSIONS

The existing 12-inch RCP pipe conveying stormwater runoff to DMH 1 was analyzed in the post-development conditions using the Field's Hydraulic Calculator based on Manning's Equation. The results of this analysis demonstrated that sufficient capacity exists in the 12-inch RCP pipe to convey stormwater runoff for the 10-year storm event. During the 25-year storm event, the 12-inch RCP pipe is flowing at capacity producing 1.5 feet of headwater in the existing catchbasin. Lastly, the existing 18-inch RCP pipe draining DMH 1 has sufficient capacity to convey stormwater runoff for the 25-year storm event in the post-development conditions.

Therefore, the proposed development will have no adverse impact on Subcatchment 1. Projected post-development peak runoff quantities and the quality of stormwater runoff will not change from pre-development conditions.

WATERSHED ROUTING ==========



SUBCATCHMENT 1 = DRAINAGE AREA TO CATCH BASIN (RIM-11.43') -> REACH 1

REACH 1 = ->

Data for **HP HOOD COOLER EXPANSION**
TYPE III 24-HOUR RAINFALL= 3.00 IN
Prepared by DUFRESNE-HENRY, INC. 11 Feb 00
HydroCAD 5.11 001123 (c) 1986-1999 Applied Microcomputer Systems

SUBCATCHMENT 1 **DRAINAGE AREA TO CATCH BASIN (RIM-11.43')**

PEAK= 6.07 CFS @ 11.99 HRS, VOLUME= .39 AF

ACRES	CN	
2.07	98	IMPERVIOUS

SCS TR-20 METHOD
TYPE III 24-HOUR
RAINFALL= 3.00 IN
SPAN= 10-20 HRS, dt=.1 HRS

Method	Comment	Tc (min)
TR-55 SHEET FLOW	**Segment ID:**	1.6
Smooth surfaces n=.011 L=205'	P2=5.5 in s=.022 '/'	
CIRCULAR CHANNEL	**Segment ID:**	.3
12" Diameter a=.79 sq-ft Pw=3.1'	r=.25'	
s=.0066 '/' n=.011 V=4.36 fps	L=85' Capacity=3.4 cfs	
	Total Length= 290 ft Total Tc=	1.9

REACH 1

Qin = 6.07 CFS @ 11.99 HRS, VOLUME= .39 AF
Qout= 6.00 CFS @ 11.99 HRS, VOLUME= .39 AF, ATTEN= 1%, LAG= 0.0 MIN

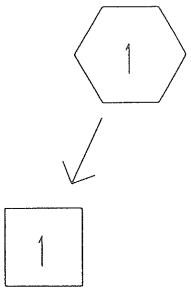
DEPTH (FT)	END AREA (SQ-FT)	DISCH (CFS)
0.00	0.00	0.00
.15	.09	.26
.30	.25	1.09
.45	.45	2.43
1.05	1.32	10.39
1.20	1.52	12.13
1.35	1.68	13.23
1.41	1.72	13.35
1.46	1.75	13.23
1.50	1.77	12.41

18" PIPE

n= .011
LENGTH= 55 FT
SLOPE= .01 FT/FT

STOR-IND+TRANS METHOD
PEAK DEPTH= .72 FT
PEAK VELOCITY= 7.2 FPS
TRAVEL TIME = .1 MIN
SPAN= 10-20 HRS, dt=.1 HRS
2 x FINER ROUTING

WATERSHED ROUTING ==========



SUBCATCHMENT 1 = DRAINAGE AREA TO CATCH BASIN (RIM-11.43') -> REACH 1

REACH 1 = ->

Data for **HP HOOD COOLER EXPANSION POST-DEVELOPMENT**
TYPE III 24-HOUR RAINFALL= 3.00 IN
Prepared by DUFRESNE-HENRY, INC. 11 Feb 00
HydroCAD 5.11 001123 (c) 1986-1999 Applied Microcomputer Systems

SUBCATCHMENT 1 **DRAINAGE AREA TO CATCH BASIN (RIM-11.43')**

PEAK= 6.07 CFS @ 11.99 HRS, VOLUME= .39 AF

ACRES	CN	
2.07	98	IMPERVIOUS

SCS TR-20 METHOD
TYPE III 24-HOUR
RAINFALL= 3.00 IN
SPAN= 10-20 HRS, dt=.1 HRS

Method	Comment	Tc (min)
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Smooth surfaces n=.011 L=205'	P2=5.5 in s=.022 '/'	
CIRCULAR CHANNEL	**Segment ID:**	.3
12" Diameter a=.79 sq-ft Pw=3.1'	r=.25'	
s=.0066 '/' n=.011 V=4.36 fps	L=85' Capacity=3.4 cfs	
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DEPTH (FT)	END AREA (SQ-FT)	DISCH (CFS)
0.00	0.00	0.00
.15	.09	.26
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18" PIPE
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LENGTH= 55 FT
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PEAK DEPTH= .72 FT
PEAK VELOCITY= 7.2 FPS
TRAVEL TIME = .1 MIN
SPAN= 10-20 HRS, dt=.1 HRS
2 x FINER ROUTING

Data for **HP HOOD COOLER EXPANSION**
TYPE III 24-HOUR RAINFALL= 4.70 IN
Prepared by DUFRESNE-HENRY, INC. 11 Feb 00
HydroCAD 5.11 001123 (c) 1986-1999 Applied Microcomputer Systems

SUBCATCHMENT 1 **DRAINAGE AREA TO CATCH BASIN (RIM-11.43')**

PEAK= 9.58 CFS @ 11.99 HRS, VOLUME= .62 AF

ACRES	CN	
2.07	98	IMPERVIOUS

SCS TR-20 METHOD
TYPE III 24-HOUR
RAINFALL= 4.70 IN
SPAN= 10-20 HRS, dt=.1 HRS

Method	Comment	Tc (min)
TR-55 SHEET FLOW	**Segment ID:**	1.6
Smooth surfaces n=.011 L=205'	P2=5.5 in s=.022 '/'	
CIRCULAR CHANNEL	**Segment ID:**	.3
12" Diameter a=.79 sq-ft Pw=3.1'	r=.25'	
s=.0066 '/' n=.011 V=4.36 fps	L=85' Capacity=3.4 cfs	
	Total Length= 290 ft Total Tc=	1.9

REACH 1

Qin = 9.58 CFS @ 11.99 HRS, VOLUME= .62 AF
Qout= 9.49 CFS @ 11.99 HRS, VOLUME= .62 AF, ATTEN= 1%, LAG= 0.0 MIN

DEPTH (FT)	END AREA (SQ-FT)	DISCH (CFS)
0.00	0.00	0.00
.15	.09	.26
.30	.25	1.09
.45	.45	2.43
1.05	1.32	10.39
1.20	1.52	12.13
1.35	1.68	13.23
1.41	1.72	13.35
1.46	1.75	13.23
1.50	1.77	12.41

18" PIPE

n= .011
LENGTH= 55 FT
SLOPE= .01 FT/FT

STOR-IND+TRANS METHOD
PEAK DEPTH= .98 FT
PEAK VELOCITY= 7.8 FPS
TRAVEL TIME = .1 MIN
SPAN= 10-20 HRS, dt=.1 HRS
2 x FINER ROUTING

Data for **HP HOOD COOLER EXPANSION POST-DEVELOPMENT**
TYPE III 24-HOUR RAINFALL= 4.70 IN
Prepared by DUFRESNE-HENRY, INC. 11 Feb 00
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SUBCATCHMENT 1 **DRAINAGE AREA TO CATCH BASIN (RIM-11.43')**

PEAK= 9.58 CFS @ 11.99 HRS, VOLUME= .62 AF

ACRES	CN	
2.07	98	IMPERVIOUS

SCS TR-20 METHOD
TYPE III 24-HOUR
RAINFALL= 4.70 IN
SPAN= 10-20 HRS, dt=.1 HRS

Method	Comment	Tc (min)
TR-55 SHEET FLOW	**Segment ID:**	1.6
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REACH 1

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Qout= 9.49 CFS @ 11.99 HRS, VOLUME= .62 AF, ATTEN= 1%, LAG= 0.0 MIN

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0.00	0.00	0.00
.15	.09	.26
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1.20	1.52	12.13
1.35	1.68	13.23
1.41	1.72	13.35
1.46	1.75	13.23
1.50	1.77	12.41

18" PIPE
n= .011
LENGTH= 55 FT
SLOPE= .01 FT/FT

STOR-IND+TRANS METHOD
PEAK DEPTH= .98 FT
PEAK VELOCITY= 7.8 FPS
TRAVEL TIME = .1 MIN
SPAN= 10-20 HRS, dt=.1 HRS
2 x FINER ROUTING

Data for **HP HOOD COOLER EXPANSION**
TYPE III 24-HOUR RAINFALL= 5.50 IN
Prepared by DUFRESNE-HENRY, INC. 9 Feb 00
HydroCAD 5.11 001123 (c) 1986-1999 Applied Microcomputer Systems

SUBCATCHMENT 1 **DRAINAGE AREA TO CATCH BASIN (RIM-11.43')**

PEAK= 11.23 CFS @ 11.99 HRS, VOLUME= .73 AF

ACRES	CN	
2.07	98	IMPERVIOUS

SCS TR-20 METHOD
TYPE III 24-HOUR
RAINFALL= 5.50 IN
SPAN= 10-20 HRS, dt=.1 HRS

Method	Comment	Tc (min)
TR-55 SHEET FLOW	**Segment ID:**	1.6
Smooth surfaces n=.011 L=205'	P2=5.5 in s=.022 '/'	
CIRCULAR CHANNEL	**Segment ID:**	.3
12" Diameter a=.79 sq-ft Pw=3.1'	r=.25'	
s=.0066 '/' n=.011 V=4.36 fps	L=85' Capacity=3.4 cfs	
	Total Length= 290 ft Total Tc=	1.9

REACH 1

Qin = 11.23 CFS @ 11.99 HRS, VOLUME= .73 AF
Qout= 11.12 CFS @ 11.99 HRS, VOLUME= .72 AF, ATTEN= 1%, LAG= 0.0 MIN

DEPTH (FT)	END AREA (SQ-FT)	DISCH (CFS)
0.00	0.00	0.00
.15	.09	.26
.30	.25	1.09
.45	.45	2.43
1.05	1.32	10.39
1.20	1.52	12.13
1.35	1.68	13.23
1.41	1.72	13.35
1.46	1.75	13.23
1.50	1.77	12.41

18" PIPE

n= .011
LENGTH= 55 FT
SLOPE= .01 FT/FT

STOR-IND+TRANS METHOD
PEAK DEPTH= 1.12 FT
PEAK VELOCITY= 7.9 FPS
TRAVEL TIME = .1 MIN
SPAN= 10-20 HRS, dt=.1 HRS
2 x FINER ROUTING

Data for **HP HOOD COOLER EXPANSION POST-DEVELOPMENT**
TYPE III 24-HOUR RAINFALL= 5.50 IN
Prepared by DUFRESNE-HENRY, INC. 9 Feb 00
HydroCAD 5.11 001123 (c) 1986-1999 Applied Microcomputer Systems

SUBCATCHMENT 1 **DRAINAGE AREA TO CATCH BASIN (RIM-11.43')**

PEAK= 11.23 CFS @ 11.99 HRS, VOLUME= .73 AF

ACRES	CN	
2.07	98	IMPERVIOUS

SCS TR-20 METHOD
TYPE III 24-HOUR
RAINFALL= 5.50 IN
SPAN= 10-20 HRS, dt=.1 HRS

Method	Comment	Tc (min)
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18" PIPE
n= .011
LENGTH= 55 FT
SLOPE= .01 FT/FT

STOR-IND+TRANS METHOD
PEAK DEPTH= 1.12 FT
PEAK VELOCITY= 7.9 FPS
TRAVEL TIME = .1 MIN
SPAN= 10-20 HRS, dt=.1 HRS
2 x FINER ROUTING

CITY OF PORTLAND, MAINE
DEVELOPMENT REVIEW APPLICATION
PLANNING DEPARTMENT PROCESSING FORM

I. D. Number

H.P. Hood
Applicant

9/16/97
Application Date

500 Rutherford Ave, Boston, MA 02129
Applicant's Mailing Address

Park Ave 349
Project Name/Description

Center Line Const/Dick Miller
Consultant/Agent

349 Park Ave, Great Diamond Island
Address of Proposed Site

846-0042
Applicant or Agent Daytime Telephone, Fax

066-D-001
Assessor's Reference: Chart-Block-Lot

Proposed Development (check all that apply): ☐ New Building ☐ Building Addition ☐ Change Of Use ☐ Residential
☐ Office ☐ Retail ☐ Manufacturing ☐ Warehouse/Distribution ☐ Parking Lot ☒ Other (specify) **2 Milk Storage Tanks**

Proposed Building square Feet or # of Units

66,890 Sq Ft
Acreage of Site

Zoning

Check Review Required:

☒ Site Plan (major/minor)
☐ Subdivision # of lots ____
☐ PAD Review
☐ 14-403 Streets Review
☐ Flood Hazard
☐ Shoreland
☐ HistoricPreservation
☐ DEP Local Certification
☐ Zoning Conditional Use (ZBA/PB)
☐ Zoning Variance
☐ Other ____

Fees Paid: Site Plan **$300.00** Subdivision ____ Engineer Review ____ Date: **9/16/97**

Planning Approval Status:

Reviewer Sarah H

☑ **Approved**
☐ **Approved w/Conditions** See Attached
☐ **Denied**

Approval Date 9/18/97 Approval Expiration ____ Extension to ____

☑ OK to Issue Building Permit Sarah ____ ____
signature date

☐ Additional Sheets Attached

Performance Guarantee ☐ **Required*** ☑ **Not Required**

* No building permit may be issued until a performance guarantee has been submitted as indicated below

☐ Performance Guarantee Accepted	date	amount	expiration date
☐ Inspection Fee Paid	date	amount	
☐ Building Permit Issued	date		
☐ Performance Guarantee Reduced	date	remaining balance	signature
☐ Temporary Certificate of Occupancy	date	☐ Conditions (See Attached)	
☐ Final Inspection	date	signature	
☐ Certificate Of Occupancy	date		
☐ Performance Guarantee Released	date	signature	
☐ Defect Guarantee Submitted	submitted date	amount	expiration date
☐ Defect Guarantee Released	date	signature	

CITY OF PORTLAND, MAINE
DEVELOPMENT REVIEW APPLICATION
PLANNING DEPARTMENT PROCESSING FORM

I. D. Number

H.P. Hood
Applicant

9/16/97
Application Date

500 Rutherford Ave, Boston, MA 02129
Applicant's Mailing Address

Park Ave 349
Project Name/Description

Center Line Const/Dick Miller
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349 Park Ave, Great Diamond Island
Address of Proposed Site

846-0042
Applicant or Agent Daytime Telephone, Fax

066-D-001
Assessor's Reference: Chart-Block-Lot

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☐ Office ☐ Retail ☐ Manufacturing ☐ Warehouse/Distribution ☐ Parking Lot ☒ Other (specify) **2 Milk Storage Tanks**

Proposed Building square Feet or # of Units

66,890 Sq Ft
Acreage of Site

Zoning

Check Review Required:

☒ Site Plan (major/minor)
☐ Subdivision # of lots ____
☐ PAD Review
☐ 14-403 Streets Review
☐ Flood Hazard
☐ Shoreland
☐ HistoricPreservation
☐ DEP Local Certification
☐ Zoning Conditional Use (ZBA/PB)
☐ Zoning Variance
☐ Other ____

Fees Paid: Site Plan **$300.00** Subdivision ____ Engineer Review ____ Date: **9/16/97**

DRC Approval Status:

Reviewer Sarah Hyde

☐ **Approved**
☐ **Approved w/Conditions** see attached
☐ **Denied**

Approval Date 9/18/97 Approval Expiration ____ Extension to ____ ☐ Additional Sheets Attached

☐ Condition Compliance Sarah Hyde ____
signature date

Performance Guarantee ☐ **Required*** ☑ **Not Required**

* No building permit may be issued until a performance guarantee has been submitted as indicated below

☐ Performance Guarantee Accepted	date	amount	expiration date
☐ Inspection Fee Paid	date	amount	
☐ Building Permit	date		
☐ Performance Guarantee Reduced	date	remaining balance	signature
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☐ Final Inspection	date	signature	
☐ Certificate Of Occupancy	date		
☐ Performance Guarantee Released	date	signature	
☐ Defect Guarantee Submitted	submitted date	amount	expiration date
☐ Defect Guarantee Released	date	signature	

CENTER LINE CONSTRUCTION, INC.
P.O. Box 1264
PORTLAND, ME 04104
(207) 767-5609 8460042

Sept. 15, 1997

Site Plan Review Statement

Project: Installation of two 25,000 Gal. Milk Tanks

Owner: H.P. Hood & Sons, Inc.
349 Park Avenue
Portland, Me. 04101

Mr. John Kneb, Pres./C.E.O.
Catamount Dairy Holdings
90 Everett Ave., Suite 200
Chelsea, MA 02150

Cost of Project:
Foundation Work $47,800.
New Tanks 125,000. (by owner)

1. Proposed Use: Install two new 25,000 gal milk tanks next to existing milk tanks.
2. Area of Site: 64,910 sq.ft. New tanks will set on a 15' x 30' concrete foundation/slab.
3. Easements: No new easements.
4. Solid Waste: No solid waste to be generated.
5. Off-site facilities; No change
6. Surface Drainage: The existing site is almost completely paved and served by a system of existing storm drains. The new tanks should have little or no impact on this system.
7. Construction plan: Install new foundation, set new tanks, & build "hall" to tie in system.
8. Regulatory Approvals - After new tanks are completely finished the FDA will inspect for final approval.
9. Evidence of Financial & Technical Capacity:
The project is being funded from Hoods Corp. assets.
Hood has been a going concern for many years.
10. Evidence of Title: Cumberland Co. Registry of Deeds 12272/84
11. Unusual Characteristics of Site: None

Center Line Construction, Inc.

Richard L. Miller, President

Planning & Urban Development

Joseph E. Gray Jr.
Director

CITY OF PORTLAND

June 16, 2000

Phillip Doughty
364 US Route 1
Falmouth, ME 04105

re: HP Hood at 349 Park Ave.

Dear Mr. Doughty:

On May 3, 2000, the Portland Planning Authority approved the site plan application for the high bay cooler expansion at the HP Hood facility at 349 Park Avenue.

The proposed site plan was found to be in conformance with the Site Plan Ordinance of the Land Use Code.

Please note the following provisions and requirements for all site plan approvals:

1. A performance guarantee covering the site improvements as well as an inspection fee payment of 1.7% of the guarantee amount must be submitted to and approved by the Planning Division and Public works prior to the recording of the subdivision plat. The subdivision approval is valid for three (3) years.

2. The site plan approval will be deemed to have expired unless work in the development has commenced within one (1) year of the approval or within a time period agreed upon in writing by the City and the applicant. Requests to extend approvals must be received before the expiration date.

3. A defect guarantee, consisting of 10% of the performance guarantee, must be posted before the performance guarantee will be released.

4. Prior to construction, a preconstruction meeting shall be held at the project site with the contractor, development review coordinator, Public Work's representative and owner to review the construction schedule and critical aspects of the site work. At that time, the site/building contractor shall provide three (3) copies of a detailed construction schedule to the attending City representatives. It shall be the contractor's responsibility to arrange a mutually agreeable time for the preconstruction meeting.

5. If work will occur within the public right-of-way such as utilities, curb, sidewalk and driveway construction, a street opening permit(s) is required for your site. Please contact Carol Merritt at 874-8300, ext. 8828. (Only excavators licensed by the City of Portland are eligible.)

The Development Review Coordinator (who is located at DeLuca Hoffman at 775-1121) must be notified five (5) working days prior to date required for final site inspection. Please make allowances for completion of site plan requirements determined to be incomplete or defective during the inspection. This is essential as all site plan requirements must be completed and approved by the Development Review Coordinator prior to issuance of a Certificate of Occupancy. Please schedule any property closing with these requirements in mind.

If there are any questions, please contact the Planning Staff.

Sincerely,

Joseph E. Gray, Jr.
Director of Planning and Urban Development

cc: Alexander Jaegerman, Chief Planner
Sarah Hopkins, Senior Planner
P. Samuel Hoffses, Building Inspector
Marge Schmuckal, Zoning Administrator
Tony Lombardo, Project Engineer
Development Review Coordinator
William Bray, Director of Public Works
Nancy Knauber, Associate Engineer
Jeff Tarling, City Arborist
Penny Littell, Associate Corporation Counsel
Lt. Gaylen McDougall, Fire Prevention
Inspection Department
Lee Urban, Director of Economic Development
Don Hall, Appraiser, Assessor's Office
Susan Doughty, Assessor's Office
Approval Letter File

City of Portland
Planning Department

389 Congress Street, 4th Floor
Portland, ME 04101
207-874-8721 or 207-874-8719
Fax: 207-756-8258

FAX TRANSMISSION COVER SHEET

Date:

To: Phil Doughty

Company:

Fax #: 781-2908

From: Sarah Hopkins

RE:

Phil -

I spoke too soon.

Here are comments from Public Works.

Note #1 can be addressed with a note on the plan.

Note #2 can be addressed w/ a detail.

Sarah

YOU SHOULD RECEIVE 2 PAGE(S),
INLUDING THIS COVER SHEET.
IF YOU DO NOT RECEIVE ALL THE PAGES,
PLEASE CALL 207-874-8721 OR 207-874-8719.

From: Anthony Lombardo
To: Sarah Hopkins
Date: Thu, Mar 23, 2000 9:50 AM
Subject: HP Hood Addition.....3/23/00

Sarah,
I have reviewed the submittal package and offer the following comments:
1. Public Works did a recent inspection of the storm drain system into which HP Hood currently discharges. The structures and piping have a large amount of sediment collected. As a result, Public Works is requesting the applicant thoroughly clean the structures and pipes, both existing and proposed, at an appropriate time near the completion of this development.
2. Public Works is also requesting that all proposed and existing catch basin structures be fitted with "Casco Traps" or an approved equivalent.

66822

QUITCLAIM DEED WITH COVENANT

H.P. HOOD INC., a Massachusetts corporation, having a place of business at 500 Rutherford Avenue, Boston, Massachusetts 02129, for full value and consideration paid, hereby grants to H.P. HOOD & SONS, INC., a Massachusetts corporation, having a place of business and a mailing address at 500 Rutherford Avenue, Boston, Massachusetts 02129, with QUITCLAIM COVENANT, those parcels of land located at or near 349 Park Avenue in the City of Portland, County of Cumberland and State of Maine, as more particularly described in Exhibit A attached hereto and made a part hereof, together with all buildings, structures and improvements located thereon, and together with all rights and appurtenances pertaining to such land.

Being the same premises conveyed to H.P. Hood Inc. by deed of Agri-Mark, Inc., dated May 9, 1991, and recorded in the Cumberland County Registry of Deeds in Book 9555, page 137.

The premises are conveyed subject to and with the benefit of all easements, restrictions, rights of way and other matters of record, if any, insofar as the same are now in force and applicable.

IN WITNESS WHEREOF, H.P. Hood Inc. has caused this deed to be executed and sealed by Robert L. Keller, its duly authorized President, this 15th day of December, 1995.

WITNESS:

H.P. HOOD INC.

By: Robert L. Keller
Robert L. Keller
President

COMMONWEALTH OF MASSACHUSETTS

County of Suffolk, ss. December 15, 1995

Personally appeared the above-named Robert L. Keller, President of H.P. Hood Inc., and acknowledged the foregoing instrument to be the free act and deed of H.P. Hood Inc.

Before me,

SEAL

Notary Public/Attorney-at-Law
Print Name: MARIE S. MAYNIHAN
My Commission Expires: 7/27/96

F:\MXN\JKNB74\DEED.POR

EXHIBIT A

A certain lot or parcel of land situated in the City of Portland, in the County of Cumberland and State of Maine, bounded and described as follows:

Beginning at a point on the northerly side of Park Avenue, said point being twenty (20) feet northwesterly measured along said Park Avenue from the point of intersection of a line extended southwesterly one hundred eighteen (118) feet from and measured normal to the center or base line of location of said Portland Terminal Company as established by the Federal Valuation Plans dated June 30, 1916;

Thence northwesterly along said Park Avenue fifty-four (54) feet, more or less, to the southeasterly corner of land now or formerly of H.P. Hood & Sons, Inc.; Thence N 19° 27' 30" W along the northeasterly line of said Hood land and continuing on a straight prolongation of said northeasterly line a distance of five hundred forty and nine-tenths (540.9) feet, more or less, to a point which is one hundred eighteen (118) feet southwesterly of and measured normal to said base line;

Thence southeasterly on a straight line a distance of five hundred sixty-one and three tenths (561.3) feet, more or less, to the point of beginning. Containing 13,760 square feet, more or less.

A certain lot or parcel of land situated in Portland, in the County of Cumberland, and State of Maine, and bounded and described as follows:

Beginning at a point in the northerly side line of Park Avenue, formerly called Portland Street in said City of Portland, which point is seventy-four feet (74') westerly along said northerly side line of said Park Avenue, from the southwesterly corner of a strip of land conveyed to the Maine Central Railroad Company by trustees of the J.B. Brown Estate; thence from said point northerly by the westerly side line of the triangular lot of land sold by the J.B. Brown & Sons to the Maine Central Railroad Company three hundred eighteen and fifty-five hundredths (318.55) feet to a stake; thence westerly on a line parallel to said Park Avenue two hundred (200) feet to a stake, thence southerly, parallel to the westerly side line of the triangular lot of land above referred to, three hundred eighteen and fifty-five hundredths (318.55) feet to a stake in the northerly side line of said Park Avenue, thence easterly along side line of said Park Avenue, to the point of beginning containing sixty thousand (60,000) square feet of land northerly of the present line of Park Avenue.

Excepting so much of the premises as was conveyed by H.P. Hood & Sons, Inc. to the State of Maine by deed dated July 28, 1971 and recorded with the Cumberland County Registry of Deeds in Book 3186, Page 109.

Together with all of Grantor's right, title and interest in the land abutting the above-described premises formerly known as St. James Street which was the subject of the Order of **Discontinuance adopted by the Portland City Council on July 3, 1989 recorded in the Cumberland County Registry of Deeds in Book 8882, Page 11.**

Haley & Aldrich, Inc.

10/18/89

REPORT ON

SUBSURFACE AND FOUNDATION INVESTIGATION
PROPOSED SILO AND ADDITION
PORTLAND, MAINE

Consulting

Geotechnical Engineers,

Geologists and

Hydrogeologists

10/18/89

REPORT ON

SUBSURFACE AND FOUNDATION INVESTIGATION PROPOSED SILO AND ADDITION PORTLAND, MAINE

by

Haley & Aldrich, Inc.
Portland, Maine

for

HP Hood, Inc.
Portland, Maine

File No. 80154-00

October 1989

Haley & Aldrich, Inc.

Consulting
Geotechnical Engineers,
Geologists and
Hydrogeologists

622 Congress Street
P.O. Box 4076
Portland, ME 04101
207/772-7869
FAX 207/772-2698

16 October 1989
File No. 80154-00

HP Hood, Inc.
349 Park Avenue
Portland, ME 04101

Attention: Mr. Scott Sawyer

Subject: Subsurface and Foundation Investigation
Proposed Silo and Addition
Portland, Maine

Gentlemen:

This report presents the results of our evaluation of the subsurface conditions and foundation requirements for the proposed silo and building addition at your facility in Portland, Maine. This evaluation was performed in accordance with our proposal dated 19 September 1989.

In summary, it is our opinion that the silo and addition can be supported on spread footings bearing on undisturbed, naturally deposited clay or on compacted structural fill placed after removal of unsuitable soil. In addition, an earth supported slab-on-grade may be used for the ground floor. Specific recommendations for foundation design and construction considerations are presented below.

INTRODUCTION

The silo and addition will be constructed at your facility on Park Avenue. The silo will be built adjacent to an existing silo located along Park Avenue. The addition will be built on the north and east sides of the existing building. The site of the proposed silo and portions of the building addition are presently paved roadways and parking areas.

Offices
Cambridge, Massachusetts
Glastonbury, Connecticut
Bedford, New Hampshire

Affiliate
H&A of New York
Rochester, New York

Portions of the existing building consist of a two-story brick structure with a basement in the area near the proposed silo. The basement floor is approximately 8 ft. below ground surface. The remainder of the building consists of a one-story masonry block structure with loading docks and no basement.

PROPOSED CONSTRUCTION

It is understood that the silo will be approximately 60 ft. high and 12 ft. in diameter. It is anticipated that the silo will require a pad approximately 15 ft. square. Total load of the silo is on the order of 400 kips.

It is further understood that the addition will consist of a two-story building having a plan area of approximately 2,100 sq. ft. A portion of the addition will consist of a second story built over the masonry block building with loading docks. It is anticipated that the addition will be supported independently from the existing building. Construction will require relocation of some existing fuel pumps and buried tanks.

SUBSURFACE CONDITIONS

Subsurface Explorations

A total of four test pits, TP1 to TP4, were excavated at the site on 20 September 1989 by W.H. Lavigne Excavating at locations shown on Figure 2, Test Pit Plan. The test pits were excavated to depths varying from 7 ft. to 10 ft. below ground surface using a Ford 555A backhoe. All test pits terminated in naturally deposited soil and were backfilled with the excavated material.

Haley & Aldrich, Inc. (H&A) monitored the test pits and prepared the logs which are included in Appendix A. Test pit locations were determined by H&A by taping from existing buildings and site features.

During excavation of test pits, the working space was monitored for volatile organic vapors using a photoionization detector. No significantly elevated levels above background were noted, however, a strong fuel oil odor was noted in TP1 and TP2 and an oily sheen was noted on the water in TP2 during excavation. A sample of the material from 4.5 ft. to 7 ft. in TP2 was monitored and a level of 30 parts per million was recorded.

The test pit logs depict subsurface conditions and groundwater levels only at the specific locations and times indicated on the logs. Soil conditions at other locations may vary from conditions observed in the test pits, and the passage of time may result in differing groundwater conditions.

Subsurface Conditions

The test pits disclose the following subsurface conditions from ground surface:

Thickness (Ft.)	Description
1.4 to 5.4	Light brown, gravelly, coarse to fine SAND; to dark brown, silty CLAY, with various amounts of brick, ash and scrap metal - FILL -
1.5 to 6.4	Gray to brown, mottled, silty CLAY, trace fine sand
Greater than 6.4	Gray, silty, fine SAND; to brown, silty medium to fine SAND, little to trace gravel, coarse sand and cobbles and boulders.

Water was measured in TP1 and TP3 at depths of 8 ft. and 5 ft., respectively, below ground surface. No water was encountered in the other test pits. However, observations were over a short period of time and do not represent the actual groundwater conditions. Water levels at the site will fluctuate with precipitation, temperature,season and construction activity in the area. Thus, water levels during and following construction may vary from conditions observed in the test pits.

RECOMMENDATIONS FOR FOUNDATION DESIGN AND CONSTRUCTION

Recommended Foundation Type and Design Criteria

The fill is not considered suitable for support of the silo or building and floor slab. All fill should be removed from within the silo and building limits.

It is recommended that the silo and building be supported on spread footings bearing on undisturbed, naturally deposited clay or sand or on compacted structural fill placed after removal of unsuitable soil.

Footings may be proportioned for an allowable bearing pressure of 2,000 lbs per sq. ft. All footings should be at least 2 ft. wide.

Exterior footings should be founded at least 4.5 ft. below the lowest adjacent ground surface exposed to freezing. Interior footings should be founded a minimum of 1.5 ft. below the ground floor.

The proposed silo will be located adjacent to the existing silo and a portion of the existing building with a basement approximately 8 ft. below ground surface. Normally, the foundation for the silo should bear at approximately the same elevation as the basement so as not to add lateral load to the basement wall. However, the existing silo, which is adjacent to the basement wall, bears at approximately 4.5 ft. below ground surface and the new silo should bear at approximately the same depth in order not to undermine the existing foundation. Because the new silo will be adjacent to a relatively small portion of the basement wall, it is anticipated that the silo bearing at approximately 4.5 ft. below ground surface will not apply excessive lateral loads to the basement wall. However, this loading condition should be reviewed by the structural engineer.

Footings for the new addition, where they abut the existing building, should bear at approximately the same elevation.

Compacted structural fill supporting footings should extend laterally from the footings to at least the limits defined by 1 horizontal to 1 vertical lines sloped outward and downward from points located at least 2 ft. horizontally beyond the bottom edges of the footings.

The ground floor slab may be a slab-on-grade bearing on compacted structural fill. For consistency, it is recommended that the floor slab bear on a minimum of 12 in. of compacted structural fill.

Lateral Foundation Loads

It is recommended that lateral loads be resisted by bottom friction on footings. It is recommended that a coefficient of friction equal to 0.35 be used for footings.

Fill Materials

Structural fill below footings and floor slab and backfill should consist of sandy gravel to gravelly sand. It should be free of organic material, loam, trash, snow, ice, frozen soil and other objectionable material, and should conform to the following gradation:

Sieve Size	Percent Finer by Weight
6 in.	100
No. 4	30 to 90
No. 40	10 to 50
No. 200	0 to 8

In open areas, compacted structural fill should be placed in layers not exceeding eight in. in loose measure and compacted by self-propelled compaction equipment at approximately optimum moisture content to a dry density of at least 95 percent of the maximum dry density, as determined in accordance with ASTM Test Designation D1557. In confined areas, the maximum particle size should be reduced to three in. and the loose layer thickness to six in. and compaction performed by hand-guided equipment.

Construction Considerations

It is anticipated that all foundation excavation can be accomplished with open excavation. The subgrade soil is susceptible to disturbance due to construction traffic (personnel and equipment). Equipment and personnel should not be permitted to travel across footing bearing surfaces. Subgrades should be protected against freezing if exposed to freezing temperatures during construction. The final excavation to grade should be performed using equipment with smooth-edge buckets.

Groundwater may be encountered during construction. It is anticipated that groundwater can be controlled by trenching and open pumping from sumps. All groundwater and surface runoff should be controlled in such a manner as to prevent disturbance to the subgrade and to permit construction in-the-dry.

It is recommended that excavation for foundations be monitored using a photoionization detector to check for the presence of volatile organic compounds. If elevated readings are encountered, provisions should be made for evaluating the source of the vapors and potential corrective action.

It is recommended that all aspects of earthwork and foundation construction be monitored by personnel qualified by training and experience.

LIMITATIONS OF RECOMMENDATIONS

This report has been prepared for specific application to the subject project in accordance with generally accepted geotechnical engineering practices. No other warranty, expressed or implied, is made. In the event that any changes in the nature, design or location of the building are planned, the conclusions and recommendations contained in this report should not be considered valid, unless the changes are reviewed and the conclusions of this report modified or verified in writing.

The recommendations presented herein are based in part upon the data obtained from the referenced test pits. The nature and extent of variations between the explorations may not become evident until construction. If variations then appear evident, it will be necessary to reevaluate the recommendations of this report.

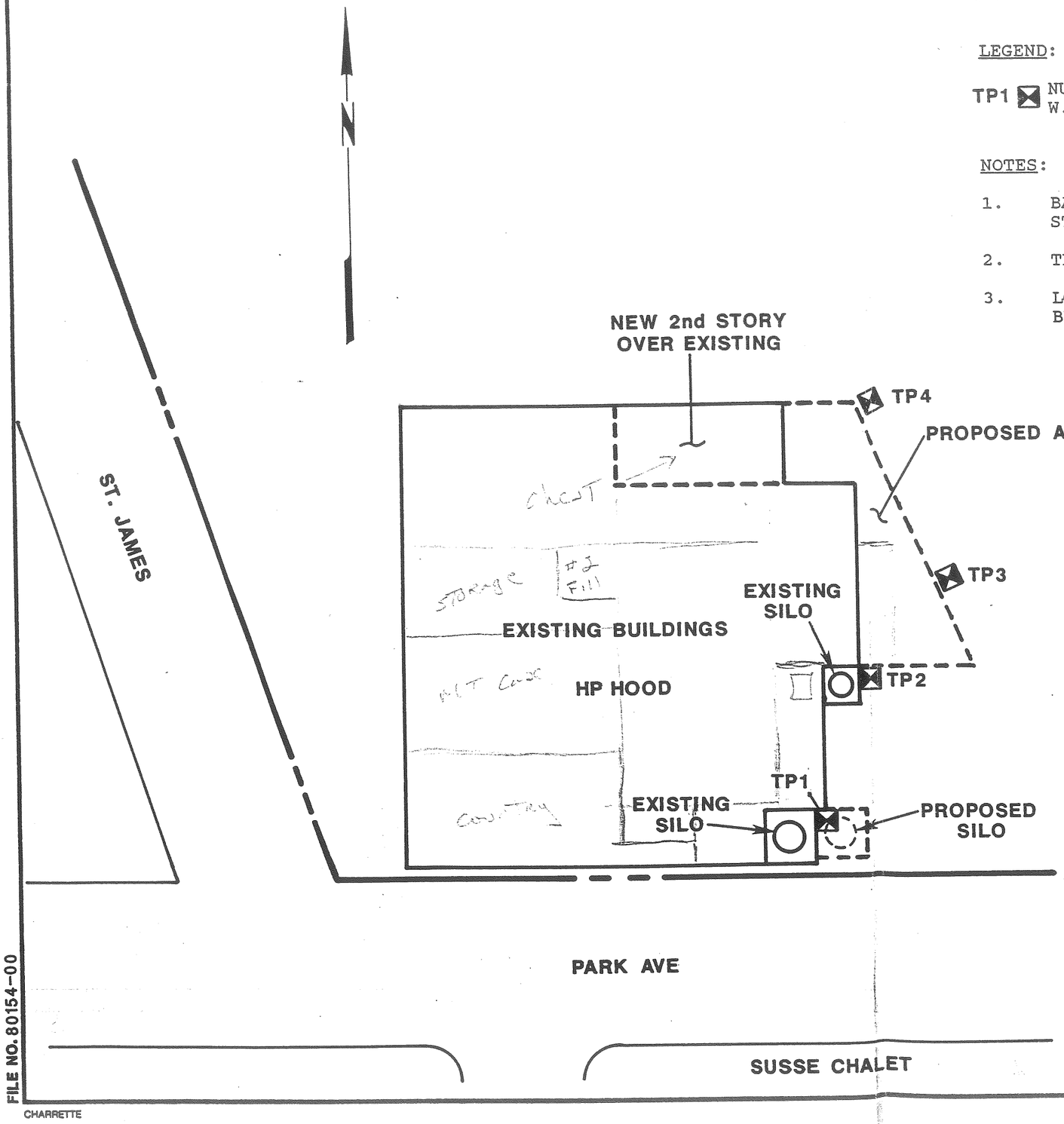
We request that we be provided the opportunity for a general review of final design and specifications in order to determine that our earthwork and foundation recommendations have been interpreted and implemented in the design and specifications as they were intended.

It has been a pleasure to work with you on this project. Please do not hesitate to contact us if you have any questions or require additional information.

Sincerely yours,
HALEY & ALDRICH, INC.

Kenneth L. Recker
Vice President

KLR:ljw/1.05
Enclosures: Figure 1 - Project Locus
Figure 2 - Test Pit Plan
Appendix A - Logs of Test Pits



LEGEND:

TP1 ⊠ NUMBER AND APPROXIMATE LOCATION OF TEST PITS EXCAVATED BY W.H. LAVIGNE EXCAVATING ON 20 SEPTEMBER 1989.

NOTES:

1. BASE PLAN PREPARED FROM 40 SCALE PLAN PROVIDED BY STRUCTURAL DESIGN CONSULTANTS.
2. TEST PITS MONITORED BY HALEY & ALDRICH, INC.
3. LOCATIONS OF TEST PITS DETERMINED BY HALEY & ALDRICH, INC. BY TAPING FROM EXISTING BUILDINGS.

40 0 40 80
scale feet

Haley & Aldrich, Inc.
Consulting Geotechnical Engineers, Geologists and Hydrogeologists

PROPOSED SILO AND ADDITION
HP HOOD, INC.
PORTLAND, MAINE

TEST PIT PLAN

SCALE: AS SHOWN OCTOBER 1989

FILE NO. 80154-00

CHARRETTE

FIGURE 2

HALEY & ALDRICH, INC., PORTLAND, MAINE Consulting Geotechnical Engineers, Geologists and Hydrogeologists	TEST PIT REPORT	TEST PIT NO. TP1 FILE NO. 80154-00
PROJECT: PROPOSED SILO AND BUILDING ADDITION LOCATION: PORTLAND, MAINE CLIENT: H.P. HOOD, INC. CONTRACTOR: W.H. LAVIGNE EXCAVATING EQUIPMENT USED: FORD 555A BACKHOE		LOCATION: SEE PLAN ELEVATION: EXPLORATION DATE: 20 SEPT 89 H&A REP.: S. DIXON

SCALE IN FEET	SAMPLE NUMBER	SAMPLE DEPTH RANGE	STRATA CHANGE	DESCRIPTION OF MATERIALS	REMARKS
2 4 6 8 10 12			0.2 1.0 1.7 3.0 5.4	Bituminous concrete -PAVEMENT- Light brown, gravelly, coarse to fine SAND Dark brown, silty, medium to fine SAND, with brick fragments -FILL- Gray ASH Dark brown, silty CLAY and ASH, trace brick fragments 1.1 ft. 0.9 ft. -FILL- Gray, silty CLAY, interbedded with fine, sandy SILT Gray, fine SAND, layer of black fine sand from 9.0-9.5 ft. Bottom of Test Pit at 10.0 ft. No Refusal	Concrete footing supports a 15 ft. x 15 ft. pad upon which sits a storage tank. The top of the pad is 1.5 ft. above ground surface. Top of footing is 3.6 ft. below ground surface. Excavated with shovel beneath footing, noted that it is bearing upon brown clay fill to at least 0.5 ft. below bottom of footing. Excavated with shovel down along adjacent building wall. Did not encounter wall footing, but encountered related wooden forms near 8.0 ft. Noted that cellar floor within building is 7.8 ft. below ground. Standing water at 8.0 ft. Sand is saturated and very loose. Black sand has strong fuel oil odor.

WATER LEVEL			APPROXIMATE PIT DIMENSIONS AT SURFACE	SUMMARY
DATE	TIME*	DEPTH (FT)	LENGTH 8.0 feet WIDTH 4.0 feet	DEPTH: 10.0 FT.
9-20-89	0.25	8.0		JAR SAMPLES: --
--	--	--	BOULDERS	BAG SAMPLES: --
--	--	--	8" to 18" DIAMETER: No. -- = Vol. -- cu ft	WATER LEVEL: 8.0 FT.
* Hrs after completed			Over 18" DIAMETER: No. -- = Vol. -- cu ft	TEST PIT NO. TP1

HALEY & ALDRICH, INC., PORTLAND, MAINE Consulting Geotechnical Engineers, Geologists and Hydrogeologists	TEST PIT REPORT	TEST PIT NO. TP3 FILE NO. 80154-00
PROJECT: PROPOSED SILO AND BUILDING ADDITION LOCATION: PORTLAND, MAINE CLIENT: H.P. HOOD, INC. CONTRACTOR: W.H. LAVIGNE EXCAVATING EQUIPMENT USED: FORD 555A BACKHOE		LOCATION: SEE PLAN ELEVATION: EXPLORATION DATE: 20 SEPT 89 H&A REP.: S. DIXON

SCALE IN FEET	SAMPLE NUMBER	SAMPLE DEPTH RANGE	STRATA CHANGE	DESCRIPTION OF MATERIALS	REMARKS
2 4 6 8 10 12			0.4 0.8 2.8 9.2	Dark brown, coarse to fine SAND, little silt, trace gravel Brown, gravelly, coarse to fine SAND, trace silt Dark brown, coarse to fine SAND, to sandy GRAVEL, some fragments of brick and particles of ash, trace scrap metal pieces -FILL- Gray, mottled, silty CLAY, trace organics Brown, mottled, silty CLAY, trace fine sand -MARINE DEPOSITS- Light brown, silty, fine SAND, trace clay Bottom of Test Pit at 9.8 ft. No Refusal	Encountered former utility pole buried from 0.5 ft. to 5.0 ft. below ground surface. Gray clay deposit may represent the sediment that filled in a channel-like erosionary feature cut into the underlying brown clay. Slight seepage at depth of 5.0 ft. within the gray clay suggests preferred ground water flow within the channel feature. FIELD VANE TESTS Depth (ft.) / Undrained Shear Strength (tsf) (tests in brown clay) 3.0 .58 3.0 .56 (tests in gray clay) 3.0 .48 3.0 .64

WATER LEVEL			APPROXIMATE PIT DIMENSIONS AT SURFACE	SUMMARY
DATE	TIME*	DEPTH (FT)	LENGTH 8.0 feet WIDTH 5.0 feet	DEPTH: 9.8 FT.
				JAR SAMPLES: --
			BOULDERS	BAG SAMPLES: --
			8" to 18" DIAMETER: No. -- = Vol. -- cu ft	WATER LEVEL: 5.0 FT.
* Hrs after completed			Over 18" DIAMETER: No. -- = Vol. -- cu ft	TEST PIT NO. TP3

HALEY & ALDRICH, INC., PORTLAND, MAINE
Consulting Geotechnical Engineers,
Geologists and Hydrogeologists

TEST PIT REPORT

TEST PIT NO. TP3

FILE NO. 80154-00

PROJECT: PROPOSED SILO AND BUILDING ADDITION
LOCATION: PORTLAND, MAINE
CLIENT: H.P. HOOD, INC.
CONTRACTOR: W.H. LAVIGNE EXCAVATING
EQUIPMENT USED: FORD 555A BACKHOE

LOCATION: SEE PLAN

ELEVATION:
EXPLORATION DATE: 20 SEPT 89
H&A REP.: S. DIXON

SCALE IN FEET	SAMPLE NUMBER	SAMPLE DEPTH RANGE	STRATA CHANGE	DESCRIPTION OF MATERIALS
			0.4	Dark brown, coarse to fine SAND, little silt, trace gravel
			0.8	Brown, gravelly, coarse to fine SAND, trace silt
2				Dark brown, coarse to fine SAND, to sandy GRAVEL, some fragments of brick and particles of ash, trace scrap metal pieces -FILL-
			2.8	Gray, mottled, silty CLAY, trace organics
4				
				Brown, mottled, silty CLAY, trace fine sand
6				
				-MARINE DEPOSITS-
8				
			9.2	Light brown, silty, fine SAND, trace clay
10				Bottom of Test Pit at 9.8 ft. No Refusal
12				

REMARKS

Encountered former utility pole buried from 0.5 ft. to 5.0 ft. below ground surface.

Gray clay deposit may represent the sediment that filled in a channel--like erosionary feature cut into the underlying brown clay. Slight seepage at depth of 5.0 ft. within the gray clay suggests preferred ground water flow within the channel feature.

FIELD VANE TESTS

Depth (ft.)	Undrained Shear Strength (tsf)
(tests in brown clay)	
3.0	.58
3.0	.56
(tests in gray clay)	
3.0	.48
3.0	.64

WATER LEVEL		
DATE	TIME*	DEPTH (FT)

* Hrs after completed

APPROXIMATE PIT DIMENSIONS AT SURFACE

LENGTH 8.0 feet WIDTH 5.0 feet

BOULDERS

8" to 18" DIAMETER: No. -- = Vol. -- cu ft

Over 18" DIAMETER: No. -- = Vol. -- cu ft

SUMMARY

DEPTH: 9.8 FT.
JAR SAMPLES: --
BAG SAMPLES: --
WATER LEVEL: 5.0 FT.
TEST PIT NO. TP3

HALEY & ALDRICH, INC., PORTLAND, MAINE
Consulting Geotechnical Engineers,
Geologists and Hydrogeologists

TEST PIT REPORT

TEST PIT NO. TP4

FILE NO. 80154-00

PROJECT: PROPOSED SILO AND BUILDING ADDITION
LOCATION: PORTLAND, MAINE
CLIENT: H.P. HOOD, INC.
CONTRACTOR: W.H. LAVIGNE EXCAVATING
EQUIPMENT USED: FORD 555A BACKHOE

LOCATION: SEE PLAN

ELEVATION:
EXPLORATION DATE: 20 SEPT 89
H&A REP.: S. DIXON

SCALE IN FEET	SAMPLE NUMBER	SAMPLE DEPTH RANGE	STRATA CHANGE	DESCRIPTION OF MATERIALS
			0.3	Bituminous concrete -PAVEMENT-
			0.6	
			0.8	Light brown, gravelly, coarse to fine SAND, trace silt -FILL-
				Bituminous concrete -PAVEMENT-
			1.4	Light brown, gravelly, coarse to fine SAND, trace silt -FILL-
2				Olive-brown, mottled, silty CLAY, trace fine sand, trace organics, with pockets of dark gray, fine sandy SILT and partings of fine SAND
			2.9	Brown, mottled, silty fine SAND, interbedded with fine SAND, little silt
4				-MARINE DEPOSITS-
			4.7	Brown, silty medium to fine SAND, little gravel, coarse sand and clay, few cobbles and boulders
6				
				-GLACIAL TILL-
8				
			9.3	Gray, sitly, fine SAND, little coarse to medium sand, trace gravel and clay
10				Bottom of Test Pit at 9.8 ft. No Refusal
12				

REMARKS

FIELD VANE TESTS

Depth (ft.)	Undrained Shear Strength (tsf)
2.0	1.56
2.0	1.80
2.0	1.84

Organic Vapor Analyzer Readings:
(ppm above background)
3 ppm from granular fill.
1 ppm from clay beneath granular fill

WATER LEVEL		
DATE	TIME*	DEPTH (FT)

* Hrs after completed

APPROXIMATE PIT DIMENSIONS AT SURFACE

LENGTH 8.0 feet WIDTH 3.0 feet

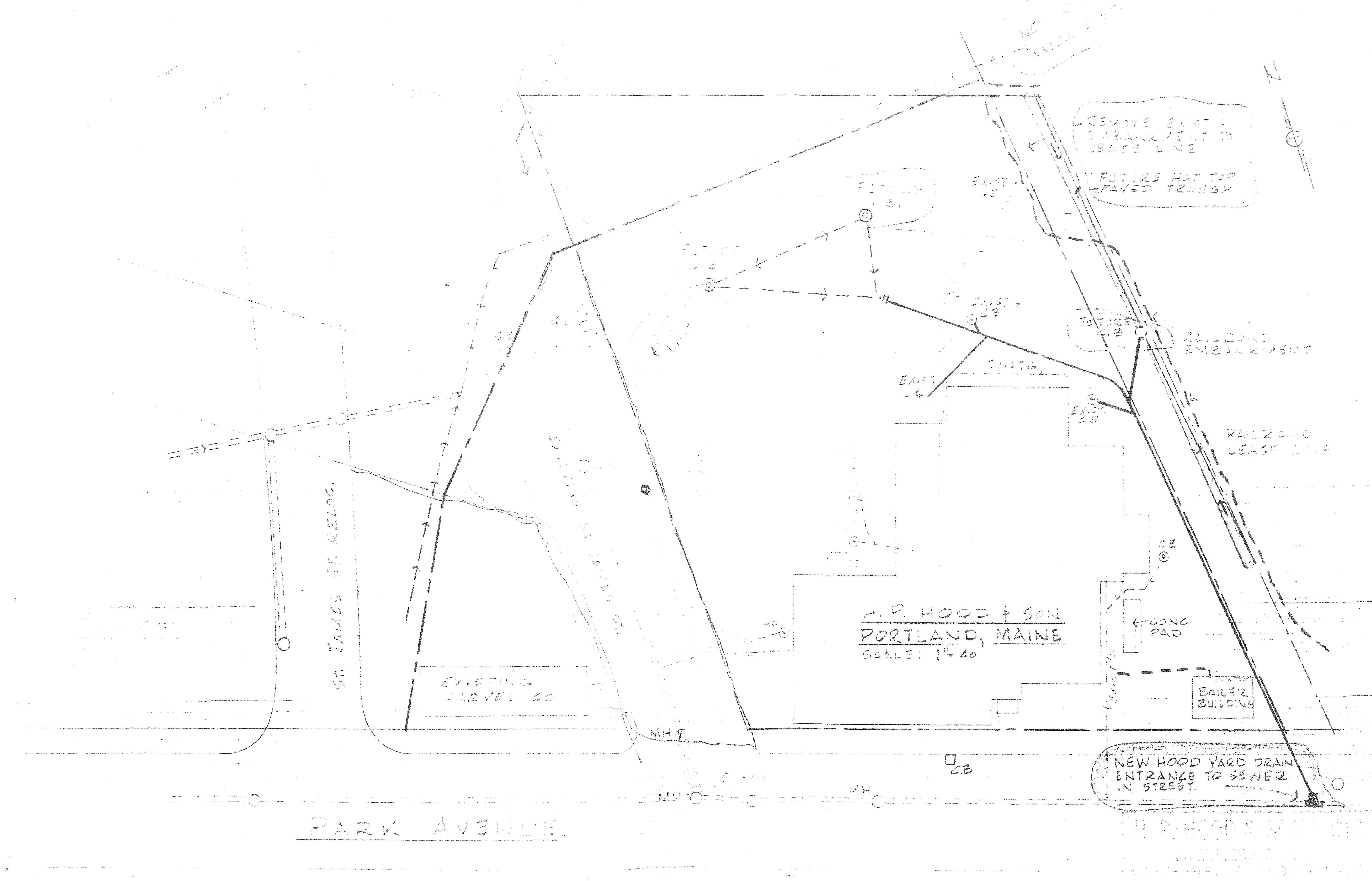
BOULDERS

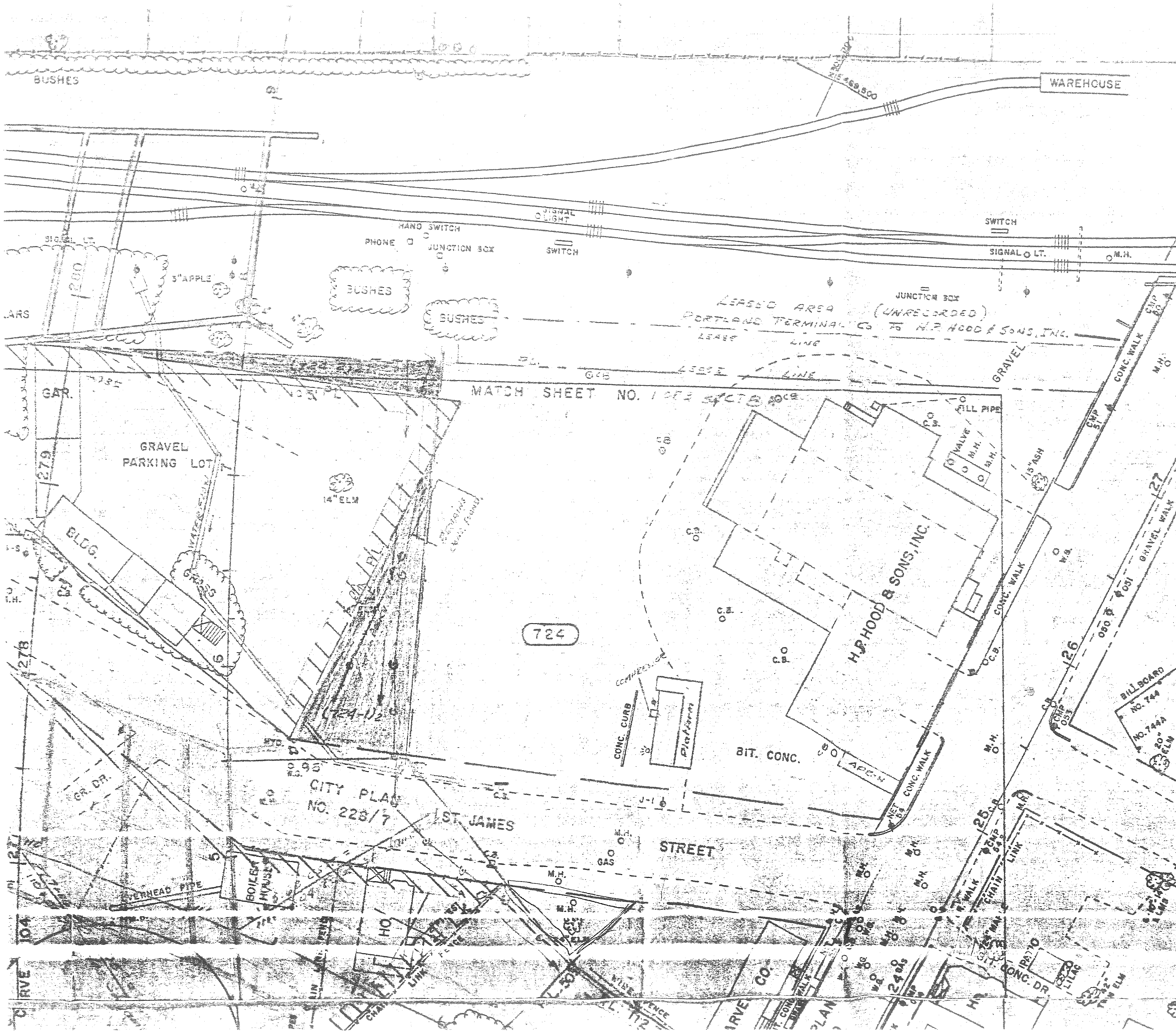
8" to 18" DIAMETER: No. 3 = Vol. 5 cu ft

Over 18" DIAMETER: No. -- = Vol. -- cu ft

SUMMARY

DEPTH: 9.8 FT.
JAR SAMPLES: --
BAG SAMPLES: --
WATER LEVEL: NE
TEST PIT NO. TP4


REMOVE EXIST'G EMBANKMENT TO LEASE LINE
FUTURE HOT TOP PAVED TROUGH
RAILROAD EMBANKMENT
RAILROAD LEASE LINE
EXIST. C.B.
EXISTING
St. JAMES St. BLDG.
H. P. HOOD & SON
PORTLAND, MAINE
SCALE: 1"=40'
CONC. PAD
BOILER BUILDING
C.B
NEW HOOD YARD DRAIN ENTRANCE TO SEWER .N STREET.
MH
PARK AVENUE



PARCEL NO. 715-2 3
NORTHERN ULTILITIES, INC.
LAND TAKEN = 3.69± AC.
(ENTIRE LOT)

PARCEL NO. 715-3 3
NORTHERN ULTILITIES, INC.
LAND TAKEN = 1.28± AC.
(ENTIRE LOT)

PARCEL NO. 721
MICHAEL J. THORNTON JR.
LAND TAKEN = 20,900± S.F.
(ENTIRE LOT)

PARCEL NO. 722
MARY E. ARSENAULT
LAND TAKEN = 14,000± S.F.
(ENTIRE LOT)

PARCEL NO. 723
J.B. BROWN & SONS
MARGARET E. BLACK
HELEN S. BEYER
HENRY ST. JOHN SMITH JR.
NANCY S. SALTONSTALL
J. HOPKINS SMITH JR.
BANK OF NEW YORK } TRUSTEES
LANGDON P. MARVIN } UNDER THE WILL OF ST. JOHN SMITH

LAND TAKEN =
TOTAL AREA =

PARCEL NO. 724-1 2
H.P. HOOD & SONS, INC.
LAND TAKEN = 4900± S.F.
TOTAL AREA = 73,750± S.F.

PARCEL NO. 724-2
LAND TAKEN = [illegible]

PARCEL NO. 725
E.& A. REALTY COMPANY
LAND TAKEN = 6,500± S.F.
(ENTIRE LOT)

PARCEL NO. 726
(OMMITTED)

PARCEL NO. 727
J.B. BROWN & SONS
MARGARET E. BLACK
HELEN S. BEYER
HENRY ST. JOHN SMITH JR.
NANCY S. SALTONSTALL
CITY OF PORTLAND
LAND TAKEN =
TOTAL AREA =

PARCEL NO. 728
CITY OF PORTLAND