Truss Worthy
217 Lincoln Road
Hodgdon, ME 04730
ph (207) 532-3200

Truss: T05-ASMART
JobName: TW09185
Date: 09/18/17 16:21:20
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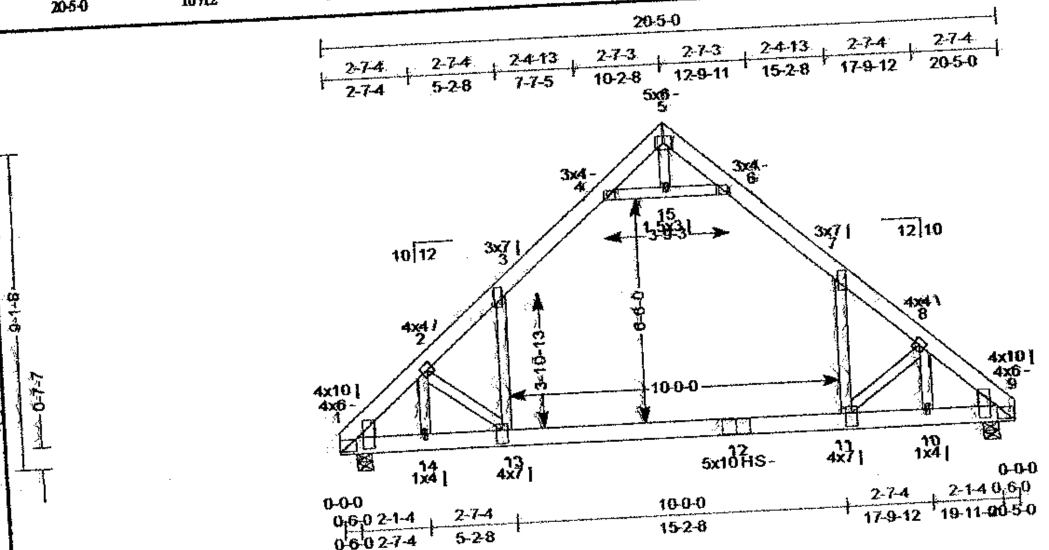
Notes: All connector plates to be Eagle 20 gauge unless otherwise noted

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
20-5-0	10 /12	1	0-0-0	0-0-0	0-6-0	0-6-0	1	24 in	114 lbs



All plates shown to be Eagle 20 unless otherwise noted

Loading (psf)
GSL : 60
TCDL : 7
BCLL : 0
BCDL : 10

General
Bldg Code : IBC 2009/ TPI 1-2007
Rep Mbr Increase : Yes
Lumber D.O.L. : 115 %

CSI
TC : 0.90 (5-6)
BC : 0.79 (10-11)
Web : 0.30 (4-15)

Deflection		L/	(loc)	Allowed
Vert TL:	0.75 in	L / 295	(12-13)	L / 240
Vert LL:	0.43 in	L / 517	(12-13)	L / 360
Horz TL:	0.03 in		9	

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	5.5 in	2.27 in	1,444 lbs		-14 lbs		-14 lbs	114 lbs
9	1	5.5 in	2.27 in	1,444 lbs		-14 lbs		-14 lbs	

Material
TC: SPF 1650f-1.5 2 x 6
BC: SPF 1650f-1.5 2 x 6
Web: SPF #2 2 x 4

Bracing
TC: Sheathed or Purlins at 4-8-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads
1) This truss has been designed for the effects of balanced (31.7 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 -05 with the following user defined input: 60 psf GSL, Terrain C, Exposure (Ce = 1.0), Building Category II (I = 1.00), Thermal (Ct = 1.00), DOL = 1.15. Unobstructed slippery surface. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.

2) This truss has been designed for the effects of wind loads in accordance with ASCE7 -05 with the following user defined input: 90 mph, Exposure C, Enclosed, Gable/Hip, Building Category II (I = 1.00), h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
3) This truss has been designed for the effects of a 14 psf live load computed in accordance with IBC 2009 assuming slope = 10 /12 and area supported = 40.83 ft^2, DOL = 115 %.
4) Minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

Table indicates Member ID, max CSI, max axial force (in compression force if different from max axial force). Only forces greater than 300 lbs are shown in this table.

	Member	CSI	Force
TC	1-2	0.151	-1,951 lbs
	2-3	0.736	-2,102 lbs
	3-4	0.733	-1,225 lbs
	4-5	0.896	560 lbs
	5-6	0.896	560 lbs
	6-7	0.733	-1,225 lbs
	7-8	0.736	-2,102 lbs
	8-9	0.151	-1,951 lbs
BC	9-10	0.284	1,544 lbs
	10-11	0.788	1,544 lbs
	11-13	0.788	1,262 lbs
	13-14	0.788	1,544 lbs
	14-1	0.284	1,544 lbs
Web	2-14	0.082	-580 lbs
	2-13	0.089	-486 lbs
	3-13	0.290	1,180 lbs
	4-15	0.298	-1,958 lbs
	6-15	0.298	-1,958 lbs
	7-11	0.290	1,180 lbs
	8-11	0.089	-486 lbs
	8-10	0.082	-580 lbs

Notes
1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
2) Attic floor area has been designed for storage with a 40 psf floor live and a 5 psf floor dead load
3) When this truss has been chosen for quality assurance inspection, the Double Polygon Method per TPI 1-2007/Chapter 3 shall be used.
4) The fabrication tolerance for this roof truss is 10 % (Cq = 0.90).
5) Brace bottom chord with approved sheathing or purlins per Bracing Summary
6) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 " of each web panel point.
7) Listed wind uplift reactions based on MWFRS & C&C loading.

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